

HYDRAULIC / HYDROLOGIC CALCULATIONS

**Site Plan
Main Street
Upton, Massachusetts**

Lobisser Construction

Prepared By:

**D&L Design Group, Inc.
115 Water Street
Milford Ma 01757**

**September 5, 2023
Revised 1/8/24**

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DRAINAGE SUMMARY

D&L Design Group Inc. is pleased to provide the following Hydraulic / Hydrologic analysis for the proposed site plan for Lobisser Construction. The proposed project is located at 0 Main Street, between 51 and 45 Main Street Upton Massachusetts. The existing site consists of 6.75 acres of undisturbed wooded area. The hydrologic conditions were analyzed using TR-55 and HydroCAD® for the 2, 10, 25 and 100 year storm events utilizing Technical Paper 40, 24 hour Rainfall events.

Project site consists of the constructing 4 story apartment building, driveway to access Main Street and parking areas around the proposed building. The project will also consist of the construction of three parking garages as shown on the site plan. The installation of proposed sewer main and pump station and an 8" water main loop to service the apartment building. The project will consist of proposed grading the site so all runoff generated from the proposed development can be collected and treated and mitigated before discharging the runoff toward the existing wetlands located on the site or before it leaves the parcel. The project as designed conforms to the Massachusetts DEP Stormwater Management Policy.

EXISTING CONDITIONS:

The project is located on Main Street between 51 and 45 Main Street in Upton Ma. The parcel is 6.75 acres in size. The site wooded and has not been disturbed. There is an existing wetland that is located in the middle of the site as well as in the east portion of the site closest to School Street. The parcel slopes from the north to the east and the south.

For the purpose of the analysis of the effect on the parcel toward the existing wetlands to the south and the property line to the east toward School Street. The site was analyzed as two watersheds. In the Pre-Development Condition, Subcatchment 1 represents the tributary area of the property that flows to the south property line toward Main Street and the wetlands located there. Subcatchment 2 represents the tributary area of the property that flows to the east property line toward School Street and the wetlands located there.

According to the online USGS soil survey, the analyzed area consists of soils with "C" hydrologic ratings. Per the soil testing performed on the site for the proposed project the soil classification and characteristics of the soil has been confirmed. All soil testing was witnessed by the Upton Conservation Agent.

PROPOSED CONDITIONS:

The proposed condition of the site will consist of a grass area around the driveway and parking areas as well as the proposed apartment building and parking garages as shown on the plans. The area left undisturbed will be woodland area adjacent to the wetlands and the middle of the site. The proposed drainage system will consist of catch basins and drain manholes located within the project to collect and direct the runoff into a forebays and infiltration basins. The infiltration basins will then discharge into a small settling basin with a level spreaders that will create a sheet flow and discharge into the 30 foot no disturb area. The site will also use proposed recharge at the propose building location that will collect and direct the runoff into underground basin.

In order to analyze the surface water flows, the site was divided into multiple Subcatchments, Pond and a Reaches

In summary, the peak rates of runoff were compared under pre-development and post-development conditions for analysis of the 2 year, 10 year, 25 year and 100 year storm events. The following is a **Peak Discharge Summary Table**:

Design Point Analysis:

Watershed		Design Event			
		2 Year	10 Year	25 Year	100 Year
Pre-Development	IP#1E	4.43/0.321	10.19/0.710	15.89/1.11	20.54/1.435
	IP#2E	4.30/0.341	10.14/0.766	14.21/1.069	20.77/1.567
	IP#3E	0.49/0.035	1.18/0.080	1.67/0.113	2.46/0.166
Post Development	IP#1P	2.89/0.250	8.08/0.671	12.20/0.976	19.08/1.428
	IP#2P	1.83/0.280	4.76/0.696	6.76/1.069	14.33/1.532
	IP#3P	0.45/0.031	0.78/0.055	0.99/0.070	1.31/0.094

DEP Stormwater Management Standards:

Standard #1: The proposed changes will not cause erosion in adjacent water of the Commonwealth, as BMP measures are proposed in accordance with the design requirements of the Stormwater Management handbook. The Erosion & Sedimentation Control Plan provides for the installation of siltation barriers, temporary basins, temporary construction entrances and outlines intermediary measures to control runoff during construction and after construction.

Standard #2: The proposed development peak discharge rates for the total off-site flow are less than or equal to pre-development discharge rates for the 2 year, 10 year, 25 year, and 100 year storm events for the design points analyzed. Attached calculations show how the site mitigates the increased flow rates due to surface changes from the site development.

Standard #3: The proposed project proposes all roof runoff to be infiltrated into the ground. Recharge will also be accomplished along the grass swale located along the driveway. The existing settling basin will be monitored throughout the construction to ensure that the runoff depths are designed to drain in under 72 hours as required by the Policy.

Standard #4: Over 80% TSS shall occur based on the BMP measurements provided. The treatment train varies for each section. TSS worksheets are provided in the report for each treatment train in the site.

Standard #5: The proposed development will not generate higher potential pollutant loads and therefore will not require additional BMP practices.

Standard #6: The proposed project is not near a critical area.

Standard #7: The proposed project is not a redevelopment project.

Standard #8: Erosion and sediment control measures are proposed as part of the proposed project.

Standard #9: An Operation & Maintenance plan is provided within this document

Standard #10: This project does not propose any illicit discharges.

STANDARD #3 –LOSS OF ANNUAL RECHARGE

The site is developed. Recharge Basins at Duplexes are constructed for recharge following the Stormwater Regulations. Soils were found to be Class A permeability.

The table below shows the required and provided recharge volumes for the project. As shown, the proposed condition exceeds the minimum requirement for the additional impervious areas.

Recharge Volume Summary

Soil Type	Recharge Factor (In. runoff)	Existing Impervious Area (sf)	Additional Impervious Area (sf)	Min. Req. Recharge Volume (cf)
A	0.60	0	0	0
B	0.35	0	0	0
C	0.25	0	86,790	1,808
D	0.10	0	0	0
Total Required				1,808

Standard #3 Only Applies to Additional Impervious

Provided Recharge Volume (cf)			
Basin 1	5,222		
Basin 2	3,075		
Basin 3	0		
Basin 4	7,840		
Total Provided			16,137

Recharge Volume Calculation:

$$R_v = F \times I$$

R_v = Required Recharge Volume

F = Recharge Factor

I = Total Impervious Area (Proposed)

$$R_v = (0.25") / (1' / 12") \times 86,790 \text{ s.f.} = 1,808 \text{ cf (Required)}$$

Provided Infiltration is 16,137cf taken from Stage Storage Worksheet

Drawdown Calculation:

Pond #1

$$Time_{drawdown} = \frac{R_v}{(K)(Bottom\ Area)}$$

R_v = Storage Volume (5,222 s.f.)

K = Saturated (1.02 in/hour)

Bottom Area = Bottom Area of Recharge Structure (1,184 s.f.)

5,222 c.f. / (1.02 in/hour)(1inch/12foot)(1,184 s.f.) = 52 hours

Pond #2

$$Time_{drawdown} = \frac{R_v}{(K)(Bottom\ Area)}$$

R_v = Storage Volume (3,075 s.f.)

K = Saturated 1.02 in/hour)

Bottom Area = Bottom Area of Recharge Structure (1745 s.f.)

3,075 c.f. / (1.02 in/hour)(1inch/12foot)(1,745 s.f.) = 21 hours

Pond #3

$$Time_{drawdown} = \frac{R_v}{(K)(Bottom\ Area)}$$

R_v = Storage Volume (5,702 s.f.)

K = Saturated 2.42 in/hour)(Material in basin is a coarse sand with cobbles)

Bottom Area = Bottom Area of Recharge Structure (721 s.f.)

5,702 c.f. / (2.42 in/hour)(1inch/12foot)(721 s.f.) = 39 hours

Pond #4

$$Time_{drawdown} = \frac{R_v}{(K)(Bottom\ Area)}$$

Where:

R_v = Storage Volume (7,840 s.f.)

K = Saturated 1.02 in/hour)

Bottom Area = Bottom Area of Recharge Structure (3,637 s.f.)

7,840 c.f. / (1.02 in/hour)(1inch/12foot)(3,637 s.f.) = 25 hours

STANDARD #4- 80% TSS REMOVAL

REQUIRED WATER QUALITY VOLUME:

ESTIMATED PROPOSED NEW IMPERVOUS AREA= 86,790 S.F.

Water Quality Volume		
Required Treatment Volume	1.0	Inches Over Impervious Areas
Watershed Series	Paved Area	Water Quality Volume
P-1 (pavement)	32,428	2,703
P-2	27,755	2,303
P-3	4,207	350
P-4	22,700	7,840

The design of the drainage system is such that the site is routed through a series of treatment BMP's meeting the Standard. The attached TSS worksheets show the water treatment prior to the existing basin located on. No bypass is designed of the BMP's reducing the WQV.

Water Quality Volume Calculation:

P-1 (Basin 1)

$$Wv = F \times I$$

Wv = Required Water Quality Volume

F = Water Factor

I = Total Impervious Area

$$Rv = (1") / (1' / 12") \times 32,428 \text{ s.f.} = 2,703 \text{ cf (Required)}$$

Provided 2,826 c.f.

P-2(Basin 2)

$$Wv = F \times I$$

Wv = Required Water Quality Volume

F = Water Factor

$$Rv = (1") / (1' / 12") \times 27,755 \text{ s.f.} = 2,303 \text{ cf (Required)}$$

Provided 2,330 c.f.

P-3(Basin 3)

$$Wv = F \times I$$

Wv = Required Water Quality Volume

F = Water Factor

$$Rv = (1") / (1' / 12") \times 4,207 \text{ s.f.} = 350 \text{ cf (Required)}$$

Provided 2,228 c.f.

P-4(Basin 4)

$$Wv = F \times I$$

Wv = Required Water Quality Volume

F = Water Factor

$$Rv = (1") / (1' / 12") \times 22,700 \text{ s.f.} = 1,891 \text{ cf (Required) Provided 7,840 c.f.}$$

INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
2. Select BMP from Drop Down Menu
3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Version 1, Automated: Mar. 4, 2008

Location: basin1

B

BMP¹

C

TSS Removal

Rate¹

D

Starting TSS

Load*

E

Amount

Removed (C*D)

F

Remaining

Load (D-E)

Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
Sediment Forebay	0.25	0.75	0.19	0.56
Infiltration Basin	0.80	0.56	0.45	0.11
Sediment Forebay	0.25	0.11	0.03	0.08
	0.00	0.08	0.00	0.08

TSS Removal Calculation Worksheet

Total TSS Removal =

92%

Separate Form Needs to be Completed for Each Outlet or BMP Train

Project: J-16

Prepared By: pml

Date: 1/8/2024

*Equals remaining load from previous BMP (E) which enters the BMP

INSTRUCTIONS:

- 1. In BMP Column, click on Blue Cell to Activate Drop Down Menu
- 2. Select BMP from Drop Down Menu
- 3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Location: basin2

TSS Removal Calculation Worksheet					
B	C	D	E	F	
BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)	
Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75	
Sediment Forebay	0.25	0.75	0.19	0.56	
Infiltration Basin	0.80	0.56	0.45	0.11	
Sediment Forebay	0.25	0.11	0.03	0.08	
	0.00	0.08	0.00	0.08	

Separate Form Needs to be Completed for Each Outlet or BMP Train

Total TSS Removal =

Project:	1-16
Prepared By:	pml
Date:	1/8/2024

*Equals remaining load from previous BMP (E) which enters the BMP

STANDARD #9- OPERATION & MAINTENANCE

OPERATION & MAINTENANCE PLAN:

CURRENT OWNER & RESPONSIBLE PARTY:

Lobisser & Ferreira Construction (Contractor shall be responsible during construction)

FUTURE OWNER & RESPONSIBLE PARTY:

Lobisser & Ferreira Construction

DURING CONSTRUCTION:

SILT FENCE BARRIER:

The silt fence barrier shall be installed prior to construction.

During construction the contractor shall inspect the silt fence barrier on a weekly basis and after any significant rainstorm resulting in greater than 0.5" of rainfall. The barrier shall be inspected for any breaches or disturbed silt fence and repaired immediately.

After construction the barrier shall be maintained as stated above until all new areas are vegetated.

After construction these duties shall transfer to the property owner.

CONSTRUCTION ENTRANCE APRONS:

Construction aprons shall be installed to protect Tractor Supply Parking Lot and Route 9. The construction entrance apron shall be installed prior to commencement of construction and shall be inspected weekly. The construction entrance apron shall be replaced when debris becomes noticeable on the existing pavement surfaces leading to and from the construction site.

SLOPE STABILIZATION:

The slope stabilization controls shall be installed immediately upon obtaining final grades as shown on the project plans. Slopes in the swale area shall be stabilized according to the details provided. All 3:1 slopes established on-site shall be loamed and seeded as soon as weather

permits. Any 2:1 slopes established shall be covered with slope stabilization fabric, then loamed and seeded as soon as weather permits. Areas in failure shall be re-graded to final grade and stabilized as necessary.

TEMPORARY BASINS:

The temporary basins shall be inspected immediately after storm events and cleaned to remove sediment build-up. Outfalls shall be inspected for erosion or scouring. Additional rip rap shall be added as required to minimize erosion.

OUTLET CONTROL STRUCTURE:

Outlet control structures at basins have temporary stone or other filtration device installed around inlet to prevent sediment deposits. Sediment shall be removed when accumulation exceeds 1" depth on paved surfaces.

CHECK DAMS:

Check Dams shall be inspected weekly and after rainfall in excess of 0.5". Accumulated sediment shall be removed when depth exceeds 3" on the upstream sided of the dam. Stone or fabric shall be replaced when evidence of clogging is present.

CONSTRUCTION COMPLETION:

The entire stormwater management system shall be inspected upon completion of construction. Portions of the system containing sediment shall be cleaned and all sediment properly removed.

AFTER CONSTRUCTION:

SEDIMENT BASIN:

Sediment Basins shall be visually inspected monthly for accumulation of debris, slope failure, or stone displacement. Slopes shall be mowed quarterly. Bottom shall be swept, vacuumed of accumulated debris semi-annually.

CATCH BASIN & DROP INLET:

At a minimum, the catch basins shall be inspected and cleaned on a quarterly basis. It is preferred that collection of accumulated sediment shall be accomplished by means of vacuum pumping and not by means of a clamshell bucket. Disposal of accumulated sediment shall be performed in accordance with applicable local, state, and federal guidelines and regulations.

GRASS SWALES & INFILTRATION BASIN:

Inspect infiltration basin after major storm events (>3.0 inches) to verify stabilization and infiltration. Mow slopes, berms quarterly. Remove accumulated clippings from infiltration stone. Inspect basin semi-annually for the following:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in embankments
- Tree growth on embankments
- Condition of rip rap
- Sediment accumulation
- Turf health.

LONG TERM POLLUTION PREVENTION PLAN

The following are the material management practices that shall be used to reduce the risk of spills or other accidental exposure of materials and substances to stormwater runoff.

Good Housekeeping: The following good housekeeping practices will be followed on site during the construction project and continued upon completion of the construction activities.

1. A concerted effort shall be made to store only enough product required to complete a particular task.
2. All materials stored on site shall be stored in a neat and orderly fashion in their appropriate containers and, if possible, under a roof or other secure enclosure.
3. Products shall be kept in their original containers with the original manufacture's label.
4. Substances shall not be mixed with one another unless recommended by the manufacturer.
5. Whenever possible, all of a product shall be used up before disposing of the container.
6. Manufacture's recommendations for proper use and disposal shall be followed.
7. The site superintendent shall inspect daily to ensure proper use and disposal of materials on site.

Hazardous Products: The following practices are intended to reduce the risks associated with hazardous materials.

1. Products shall be kept in original containers unless they are not re-sealable.
2. Where feasible, the original label and material safety data shall be retained, whereas they contain important product information.
3. If surplus product must be disposed of, follow manufacturers or local and State recommended methods for proper disposal.

Product Specific Practices: The following product-specific practices shall be followed on site:
Petroleum Products:

1. All on site vehicles shall be monitored for leaks and receive regular preventative maintenance to reduce the risk of leakage.
2. Petroleum products shall be stored in tightly sealed containers which are clearly labeled.
3. Petroleum Products shall be stored in compliance with Fire Marshall regulations.

Bituminous Concrete:

Any bituminous concrete or asphalt substances used on site shall be applied according to the manufacturer's recommendations.

Fertilizers:

Fertilizers shall be applied in the minimum amounts recommended by the manufacturer. Once applied, fertilizers shall be worked into the soil to limit exposure to stormwater. Storage shall be in a covered shed or trailer. The contents of any partially-used bags of fertilizer shall be transferred to a sealable plastic bag or bin to avoid spills

Paints:

1. All containers shall be tightly sealed and stored when not required for use.
2. Excess paint shall not be discharged into any catch basin, drain manhole or any portion of the stormwater management system.
3. Excess paint shall be properly disposed of according to manufacturer's recommendations or State and local regulations.

Concrete Trucks:

Concrete trucks shall not be allowed to wash out or discharge surplus concrete or drum wash water on site.

SPILL CONTROL PRACTICES

In addition to the good housekeeping and material management practices discussed in the previous sections of this plan, the following practices shall be followed for spill prevention and cleanup:

1. Manufacturer's recommended methods for cleanup shall be readily available at the onsite trailer, and site personnel shall be made aware of the procedures and the location of the information.
2. Materials and equipment necessary for spill clean up shall be kept in the material storage area on site. Equipment and materials shall include, but not be limited to, brooms, dust pans, mops, rags, gloves, goggles, kitty litter, sand, sawdust and plastic and metal trash containers specifically for this purpose.
3. All spills shall be cleaned up immediately after discovery.
4. The spill area shall be kept well ventilated, and personnel shall wear appropriate protective clothing to prevent injury from contact with hazardous substance.
5. Spills of toxic or hazardous material shall be reported to the appropriate State and/or local authority in accordance with local and/or State regulations.
6. The spill prevention plan shall be adjusted to include measures to prevent a particular type of spill from reoccurring and instructions on how to clean up the spill if there is another occurrence. A description of the spill, what caused it, and the clean up measures shall also be included.
7. The "Manager" shall be the spill prevention and cleanup coordinator. The "Manager" shall designate at least three other site personnel who will be trained in the spill control practices identified above.

ILLICIT DISCHARGE COMPLIANCE STATEMENT

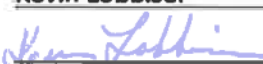
SITE ADDRESS: 47 Main Street, Upton Massachusetts
OWNER: Lobbiser Building Corp.
PLAN REFERENCE: DRAINAGE & UTILITIES PLAN PREPARED BY D&L Design Group, Inc.
DATE: December 29, 2023

As required by Standard 10 of the Massachusetts Stormwater Standards, I, the undersigned, being the authorized owner/responsible party of the above referenced property do hereby certify that no illicit discharges exist on the site and that the stormwater management system, as shown on the above referenced plan, does not contain or permit any illicit discharges to enter the stormwater management system. Furthermore discharges from interior building drains or plumbing within the buildings are prohibited. Illicit discharges do not include discharges from the following activities or facilities: firefighting, water line flushing, landscape irrigation, uncontaminated groundwater, potable water sources, foundation drains, air conditioning condensation, footing drains, individual resident car washing, flows from riparian habitats and wetlands, dechlorinated water from swimming pools, water used for street washing and water used to clean residential buildings without detergents.

The pollution prevention plan measures to implements in this project to prevent illicit discharges to the stormwater management system, including wastewater discharges and discharges of stormwater contaminated by contact with process wastes, raw materials, toxic pollutants, hazardous substances, oil, or grease, include:

1. Identifying the responsible personnel for the implementation of an effective Illicit Discharge Detection and Elimination [IDDE] program.
2. Identify potential sources of Illicit Discharges.
3. Implement the Spill Prevention and Control Plan contained in the property Stormwater Pollution Prevention Plan [SWPPP].

Further, I certify that the stormwater management system as shown on the referenced plan will be maintained in accordance with the conditions of the Long Term Pollution Prevention Plan.

NAME: Kevin Lobbiser
SIGNED: 
DATE: December 29, 2023

STORMWATER MANAGEMENT CHECKLIST



**Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program**

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

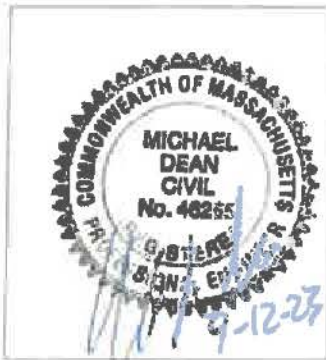
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☒ New development
☐ Redevelopment
☐ Mix of New Development and Redevelopment



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Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☒ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☒ Use of "country drainage" versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☒ Grass Channel
- ☐ Green Roof
- ☐ Other (describe): _____

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



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Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☒ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☒ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☐ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☐ Static
 - ☐ Simple Dynamic
 - ☐ Dynamic Field¹
- ☐ Runoff from all impervious areas at the site discharging to the Infiltration BMP.
- ☒ Runoff from all impervious areas at the site is *not* discharging to the Infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the Infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the Infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to Infiltration BMP if Dynamic Field method is used.



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Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

- ☒ The Infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☐ Documentation is provided showing that Infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☐ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☐ involves runoff from land uses with higher potential pollutant loads.
 - ☒ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



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Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

- ☒ The BMP is sized (and calculations provided) based on:
 - ☒ The ½" or 1" Water Quality Volume or
 - ☒ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☒ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the proprietary BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted *prior* to the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does *not* cover the land use.
- ☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☐ All exposure has been eliminated.
- ☐ All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



**Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program**

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☐ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has *not* been included in the Stormwater Report but will be submitted *before* land disturbance begins.
- ☐ The project is *not* covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☒ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☐ Description and delineation of public safety features;
 - ☒ Estimated operation and maintenance budget; and
 - ☒ Operation and Maintenance Log Form.
- ☐ The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- ☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted *prior* to the discharge of any stormwater to post-construction BMPs.

APPENDICES:

Soil Logs and Soil Map Information

Inspection & Maintenance Logs During Construction

Inspection & Maintenance Logs After Construction

Hydrocad Files

Pre-Development Watershed Map

Post-Development Watershed Map



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

Lobisser Building Corp.
Owner Name
47 Main Street
Street Address
Upton
City

MA
State

Maplot #
01568
Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade ☐ Repair

2. Soil Survey Available? ☒ Yes ☐ No If yes:

Soils
Soil Name

Soil Limitations

NRCS
Source

315B
Soil Map Unit

Glacial Till

Soil Parent material

Ridge
Landform

3. Surficial Geological Report Available? ☐ Yes ☒ No If yes:

Year Published/Source

Map Unit

Description of Geologic Map Unit:

4. Flood Risk Insurance Map Within a regulatory floodway? ☐ Yes ☒ No

5. Within a velocity zone? ☐ Yes ☒ No

6. Within a Mapped Wetland Area? ☐ Yes ☒ No If yes, MassGIS Wetland Data Layer:

7. Current Water Resource Conditions (USGS): August 2023
Month/Day/Year

Ranger: ☒ Above Normal

Wetland Type
☐ Normal

☐ Below Normal

8. Other references reviewed:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 10-11

Hole #

Date 8/3/23

Time 10 am

Clear 78

Latitude

Longitude

1. Land Use woodland
(e.g., woodland, agricultural field, vacant lot, etc.)

Vegetation wooded

Soil SCM6

Weather

Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

Description of Location:

2. Soil Parent Material: Glacial Till

Ridge
Landform

Front of Parcel
Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from: Open Water Body n/a test

Property Line 70 test

Drainage Way n/a test

Wetlands 50 test

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil ☐ Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No

If Yes: 0/80 Depth Weeping from Pit

0/72 Depth Standing Water in Hole

Soil Log

Depth (ft)	Soil Horizon / Layer	Soil Texture (USDA)	Soil Matrix Color- Munsell (planned)	Radiochromic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	SL	10YR4/4	N/A			N/A				Dth#10
12-21	B	SL	10YR4/6	N/A			N/A				
21-72	C	C.S.	2.5Y5/4	N/A			N/A	30			
0-10	A	SL	10YR4/4	N/A							Dth#11
10-20	B	SL	10YR4/6	N/A							
20-72	C	C.S.	2.5Y5/4	N/A				35			

Additional Notes:



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Longitude:

Stopes (%)

Description of Location:

Position on Landscape (SU, SH, BS, FS, TS)

Wetlands 30 feet

4. Unsuitable

☐ Yes ☒ No

If Yes:

☐ **Disturbed Soil**

☐ **F20 Material**

☐ Weathered/Fractured Rock

☐ **Bedrock**

3. **Glucuronidation observed?** ☐ Yes ☒ No

If yes: _____ Depth Weeping from PPL

Depth Standing Water in Hole

Soil Log

[illegible]

Additional Notes:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used:

- | | Obs. Hole #g | Obs. Hole #10/11 |
|---|--------------|------------------|
| <input checked="" type="checkbox"/> Depth observed standing water in observation hole | __ inches | 0/72 inches |
| <input checked="" type="checkbox"/> Depth weeping from side of observation hole | __ inches | 0/80 inches |
| <input type="checkbox"/> Depth to soil redoximorphic features (mottles) | __ inches | __ inches |
| <input type="checkbox"/> Depth to adjusted seasonal high groundwater (Sa)
(USGS methodology) | __ inches | __ inches |

Index Well Number

Reading Date

$$S_h = S_e - [S_r \times (OW_e - OW_{min}) / OW_d]$$

Obs. Hole/Water# _____ S_e _____ S_r _____ OW_e _____ OW_{min} _____ OW_d _____ S_h _____

2. Estimated Depth to High Groundwater: _____ inches

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

- a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
☒ Yes ☐ No
- b. If yes, at what depth was it observed (exclude A and O Horizons)?
Upper boundary: 30 inches Lower boundary: 72+ inches
- c. If no, at what depth was impervious material observed?
Upper boundary: _____ inches Lower boundary: _____ inches



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Peter Laviole SE#1332

Typed or Printed Name of Soil Evaluator / License #

Mary Overholt

Name of Approving Authority Witness

8/3/23

Date

2025

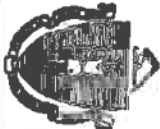
Expiration Date of License

Upton Conservation Agent

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Recapitation Test Form 12](#).

Field Diagrams: Use the area for field diagrams:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

Lobbeser Building Corp
Owner Name
47 Main Street
Street Address
Upton
City

MA
State

Map/Lat #
01568
Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade ☐ Repair

2. Soil Survey Available? ☒ Yes ☐ No If yes:

Scituate
Soil Name

Soil Limitations

NRCS
Source

315B
Soil Map Unit

Glacial Till

Soil Parent Material

Ridge
Landform

3. Surficial Geological Report Available? ☐ Yes ☒ No

If yes:

Year Published/Source

Map Unit

Description of Geologic Map Unit:

4. Flood Rate Insurance Map Within a regulatory floodway? ☐ Yes ☒ No

5. Within a velocity zone? ☐ Yes ☒ No

6. Within a Mapped Wetland Area? ☐ Yes ☒ No If yes, MassGIS Wetland Data Layer.

7. Current Water Resource Conditions (USGS):

August 2023
Month/Day/Year

Range: ☒ Above Normal

Wetland Type
☐ Normal

☐ Below Normal

8. Other references reviewed:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 588-2023

Hole # 588-2023 Date 8/3/23

Time 10 am

Clear 78

Weather

Latitude

Longitude: 4

1. Land Use woodland
(e.g., woodland, agricultural field, vacant lot, etc.)

wooded
Vegetation

Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

Description of Location:

2. Soil Parent Material: Glacial Till

Ridge
Landform

Front of Parcel
Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from: Open Water Body n/a test

Property Line 70 feet

Drainage Way n/a test
Drinking Water Well n/a test

Wetlands 50 feet

Other feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil ☐ Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If Yes: 480 Depth Weeping from P/L

 Depth Standing Water in Hole

Soil Log

Depth (ft)	Soil Horizon Layer	Soil Texture (USDA)	Soil Matrix Color- Munsell	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure Consistence (plastic)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones		
0-20	A	S.L.	10YR4/4	N/A			N/A			Df: n6
20-30	B	S.L.	10YR4/6	N/A			N/A			
30-60	C	L.S.	2.5Y5/4	30"	10YR5/8	25	25			
0-10	A	S.L.	10YR4/4	N/A						Df: n6
10-32	B	S.L.	10YR4/6	N/A						
32-62	C	L.S.	2.5Y5/4	N/A						

Additional Notes:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 788

2023 8/3/23

10

CLEAR 78

Latitude

Longitude

1. Land Use:

Woodland
(e.g., woodland, agricultural field, vacant lot, etc.)

Wooded
Vegetation

SOME
Surface Stones (e.g., cobbles, stones, boulders, etc.)

4
Slope (%)

Description of Location:

2. Soil Parent Material:

Glacial Till

Rite

Landform

Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from:

Open Water Body n/a feet

Drainage Way n/a feet

Wetlands 20 feet

4. Unsuitable

Materials Present: ☐ Yes ☒ No

Property Line 40 feet

Drinking Water Well n/a feet

Other _____ feet

5. Groundwater Observed: ☐ Yes ☒ No

If yes: 52/46 Depth Weeping from PI ☐ FFI Material ☐ Weathered/Fractured Rock ☐ Bedrock

52/46 Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon Layer	Soil Texture (USDA)	Soil Matrix: Color-Matrix (plumbe)	Pedoanthropic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-10	A	S.L.	10YR4/4	N/A							Dit#7
10-32	B	S.L.	10YR4/8	N/A							
32-60	C	L.S.	5Y7/2	41"	10YR5/8	25					
0-10	A	S.L.	10YR4/4	N/A							Dit#8
10-32	B	S.L.	10YR4/8	N/A							
32-70	C1	L.S.	5Y7/2	39	10YR5/8	25					

Additional Notes:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used:

- ☒ Depth observed standing water in observation hole
- Obs. Hole # 5/6 Obs. Hole # 7/8
- 48/0 inches 52/46 inches
- ☒ Depth weeping from side of observation hole
- 48/0 inches 52/46 inches
- ☒ Depth to soil redoximorphic features (mottles)
- 30/0 inches 41/39 inches
- ☐ Depth to adjusted seasonal high groundwater (S_h) (USGS methodology)
- _____ inches _____ inches

Index Well Number _____

Reading Date _____

$$S_h = S_e - [S_r \times (OW_e - OW_{max}) / OW_d]$$

Obs. Hole Well# _____ S_e _____ S_r _____ OW_e _____ OW_{max} _____ OW_d _____ S_h _____

2. Estimated Depth to High Groundwater: _____ inches

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

- a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

- b. If yes, at what depth was it observed (exclude A and O Horizons)?

- c. If no, at what depth was impervious material observed?

Upper boundary: 30 inches Lower boundary: _____ inches

Upper boundary: _____ inches Lower boundary: _____ inches



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Peter Lavoda SE#1332

Typed or Printed Name of Soil Evaluator / License #

Mary Overholt

Name of Approving Authority / Witness

8/3/23

Date

2025

Expiration Date of License

Upton Conservation Agent

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Revised Soil Test Form 12](#).

Field Diagrams: Use the area for field diagrams:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

Lobbeser Building Corp
Owner Name
47 Main Street
Street Address
Upton
City
MA
State
Map Unit #
01568
Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade ☐ Repair

2. Soil Survey Available? ☒ Yes ☐ No If yes:

NRCS
Source
315B
Soil Map Unit

Soil Name

Soil Limitations

Glacial Till

Soil Parent material

Ridge
Landform

3. Surficial Geological Report Available? ☐ Yes ☒ No

If yes:

Year Published/Source

Map Unit

Description of Geologic Map Unit

4. Flood Rate Insurance Map Within a regulatory floodway? ☐ Yes ☒ No

5. Within a velocity zone? ☐ Yes ☒ No

6. Within a Mapped Wetland Area? ☐ Yes ☒ No If yes, MassGIS Wetland Data Layer:

7. Current Water Resource Conditions (USGS): August 2023
Month/Day/Year
Range: ☒ Above Normal ☐ Normal ☐ Below Normal

8. Other references reviewed:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 162-2023

Hole # 8/3/23

10 am

Clear 78

Weather

Latitude

Longitude: 4

1. Land Use woodland
(e.g., woodland, agricultural field, vacant lot, etc.)

wooded
Vegetation

some
Surface Stones (e.g., cobbles, stones, boulders, etc.)

Slope (%)

Description of Location:

2. Soil Parent Material: Glacial Till

Ridge
Landsom

Front of Parcel
Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from: Open Water Body N/A feet

Property Line 70 feet

Drainage Way N/A feet

Wetlands 50 feet

4. Unavailable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil ☐ Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No

If Yes: 30"/28" Depth Weeping from PG

 Depth Standing Water in Hole

Soil Log

Depth (m)	Soil Horizon / Layer	Soil Texture (USDA)	Soil Electric Color (Munsell)	Radiomorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (pH)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-10	A	SL	10YR4/4	N/A			N/A				Dit#1
10-28	B	SL	10YR4/6	N/A			N/A				
28-60	C	LS	2.5Y5/4	36"	10YR5/8	25	25				
0-12	A	SL	10YR4/4	N/A							Dit#2
12-28	B	SL	10YR4/6	26"	10YR5/8	25	N/A				
28-72	C	LS	2.5Y5/4	N/A							

Additional Notes:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

Deep Observation Hole Number: 3&4

2023 Date

10 Time

CLEAR 78 Weather

Latitude

Longitude

1. Land Use:

Woodland
(e.g., woodland, agricultural field, vacant lot, etc.)

Wooded Vegetation

Some Surface Stones (e.g., cobbles, stones, boulders, etc.)

4 Slope (%)

Description of Location: _____

2. Soil Parent Material:

Glacial Till

Ride

Landform

Position on Landscape (SU, SH, BS, FS, TS)

3. Distances from: Open Water Body n/a feet

Property Line 40 feet

Drainage Way n/a feet

Wetlands 70 feet

Other _____ feet

4. Unsuitable

Materials Present ☐ Yes ☒ No

If Yes: ☐ Disturbed Soil ☐ Fill Material

☐ Weathered/Fractured Rock

☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No

If yes: 38"/46" Depth Weeping from PG

____ Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon Layer	Soil Texture (USDA)	Soil Mastic Color-Moist (Munsell)	Bedrock/epic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (pH)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	S.L.	10YR4/4	N/A							Dit#3
12-28	B	S.L.	10YR4/6	24"	10YR5/8	25					
28-72	C	L.S.	2.5Y5/4	N/A							
0-10	A	S.L.	10YR4/4	N/A							Dit#4
10-21	B	S.L.	10YR4/6	N/A							
21-41	C1	L.S.	5Y6/2	28	10YR5/8	25					
41-65	C2	M.S.	2.5Y5/4								

Additional Notes:



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used:

- ☐ Depth observed standing water in observation hole
- ☒ Depth weeping from side of observation hole
- ☒ Depth to soil redoximorphic features (mottles)
- ☐ Depth to adjusted seasonal high groundwater (S_h) (USGS methodology)
- | Obs. Hole #1/2 | Obs. Hole #3/4 |
|----------------|----------------|
| ____ inches | ____ inches |
| 36/28 inches | 24/28 inches |
| 28/28 inches | 24/28 inches |
| ____ inches | ____ inches |

Index Well Number

Flowing Date

$$S_h = S_o - [S_r \times (OW_c - OW_{max}) / OW_d]$$

Obs. Hole/Well# _____ S_o _____ S_r _____ OW_c _____ OW_{max} _____ OW_d _____ S_h _____

2. Estimated Depth to High Groundwater: _____ inches

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

- a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?
☒ Yes ☐ No
- b. If yes, at what depth was it observed (exclude A and O Horizons)?
Upper boundary: 24 inches Lower boundary: 72+ inches
- c. If no, at what depth was impervious material observed?
Upper boundary: _____ inches Lower boundary: _____ inches



Commonwealth of Massachusetts
City/Town of Upton

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Peter Lando SE#1332

Typed or Printed Name of Soil Evaluator / License #

Mary Overholt

Name of Approving Authority Witness

8/3/23

Date

2025

Expiration Date of License

Upton Conservation Agent

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

**Soil Map—Worcester County, Massachusetts, Southern Part
(MAIN STREET - PROJECT)**

MAP LEGEND

	Area of Interest (AOI)		Soil Area
	Area of Interest (AOI)		Stony Spot
	Soils		Very Stony Spot
	Soil Map Unit Polygon		Wet Spot
	Soil Map Unit Line		COT
	Soil Map Unit Point		Special Line Point
	Special Point Feature		Water Feature
	Mound		Stream and Canal
	Bunker Pit		Transportation
	Clay Spot		Road
	Clay Depression		Interstate Highways
	Gravel Pit		US Route
	Gravelly Spot		Major Road
	Landfill		Local Road
	Lean Fill		Background
	Marsh or Swamp		Aerial Photography
	Mine or Quarry		
	Mountainous Water		
	Perennial Water		
	Rock Outcrop		
	Saline Spot		
	Stony Spot		
	Severely Shaded Spot		
	Shrub		
	Wet or Dry		
	Soil Spot		

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Distortion of maps beyond the scale of mapping can cause misinterpretation of the detail of mapping and accuracy of soil type placement. The maps do not show the entire area of continuing soil that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: National Resources Conservation Service
Web Soil Survey URL:

Coordinate System: Web Mercator (NAD83/2011)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Worcester County, Massachusetts, Southern Part
Survey Area Date: Version 16, Sep 8, 2002

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2002—Jan 6, 2003

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in ACI	Percent of ACI
73A	Whitman fine sandy loam, 0 to 3 percent slopes, extremely stony	3.9	7.1%
300B	Montauk fine sandy loam, 3 to 8 percent slopes	5.1	9.2%
300C	Montauk fine sandy loam, 8 to 15 percent slopes	3.2	5.8%
302B	Montauk fine sandy loam, 0 to 8 percent slopes, extremely stony	17.1	31.0%
315B	Scituate fine sandy loam, 3 to 8 percent slopes	12.1	22.1%
317B	Scituate fine sandy loam, 3 to 8 percent slopes, extremely stony	1.2	2.2%
420B	Canton fine sandy loam, 3 to 8 percent slopes	10.9	19.9%
420C	Canton fine sandy loam, 8 to 15 percent slopes	0.3	0.6%
422C	Canton fine sandy loam, 8 to 15 percent slopes, extremely stony	1.1	2.1%
Totals for Area of Interest		55.9	100.0%

Worcester County, Massachusetts, Southern Part

302B—Montauk fine sandy loam, 0 to 8 percent slopes, extremely stony

Map Unit Setting

National map unit symbol: 2w80t

Elevation: 30 to 1,120 feet

Mean annual precipitation: 36 to 71 inches

Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: Not prime farmland

Map Unit Composition

Montauk, extremely stony, and similar soils: 85 percent

Minor components: 15 percent

*Estimates are based on observations, descriptions, and transects of
the map unit.*

Description of Montauk, Extremely Stony

Setting

Landform: Ground moraines, drumline, recessional moraines, hills

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Parent material: Coarse-loamy over sandy lodgment till derived
from gneiss, granite, and/or schist

Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material

A - 2 to 8 inches: fine sandy loam

Bw1 - 8 to 28 inches: fine sandy loam

Bw2 - 28 to 36 inches: sandy loam

2Cd - 36 to 74 inches: gravelly loamy sand

Properties and qualities

Slope: 0 to 8 percent

Surface area covered with cobbles, stones or boulders: 9.0 percent

Depth to restrictive feature: 20 to 43 inches to dense material

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low
to moderately high (0.00 to 1.42 in/hr)

Depth to water table: About 18 to 37 inches

Frequency of flooding: None

Frequency of ponding: None

Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)

Available water supply, 0 to 60 inches: Low (about 5.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7a

Hydrologic Soil Group: C

Ecological site: F144AY007CT - Well Drained Dense Till Uplands

Hydric soil rating: No

Minor Components

Soltuate, extremely stony

Percent of map unit: 8 percent

Landform: Ground moraines, hills, drumlins

Landform position (two-dimensional): Summit, backslope, footslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Canton, extremely stony

Percent of map unit: 5 percent

Landform: Hills

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Ridgebury, extremely stony

Percent of map unit: 2 percent

Landform: Depressions, ground moraines, hills, drainageways

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Head slope, base slope

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Worcester County, Massachusetts, Southern Part

Survey Area Data: Version 15, Sep 8, 2022

Worcester County, Massachusetts, Southern Part

315B—Scituate fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9bc9

Elevation: 200 to 950 feet

Mean annual precipitation: 32 to 50 inches

Mean annual air temperature: 45 to 60 degrees F

Frost-free period: 145 to 240 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Scituate and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scituate

Setting

Landform: Hills

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Base slope

Down-slope shape: Linear

Across-slope shape: Concave

Parent material: Friable coarse-loamy collan deposits over dense sandy lodgment till derived from granite and gneiss

Typical profile

H1 - 0 to 4 inches: sandy loam

H2 - 4 to 16 inches: gravelly sandy loam

H3 - 16 to 30 inches: loamy sand

H4 - 30 to 85 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 20 to 30 inches to dense material

Drainage class: Moderately well drained

Runoff class: High

Capacity of the most limiting layer to transmit water

(Ksat): Moderately low to moderately high (0.08 to 0.20 in/hr)

Depth to water table: About 17 to 35 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w

Hydrologic Soil Group: C

Ecological site: F144AY037MA - Moist Dense Till Uplands



Hydric soil rating: No

Minor Components

Montauk

Percent of map unit: 15 percent

Hydric soil rating: No

Ridgebury

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Worcester County, Massachusetts, Southern Part

Survey Area Data: Version 15, Sep 9, 2022

Hydrocad Files

J-10

WEEKLY
Inspection and Maintenance Log **FOR: Main St.**
DURING CONSTRUCTION **& After 1.0" Rain**

Components	Date
Erosion Control – Weekly	
Comments during insp.	
Note corrective measures performed & Date	
On Site Pavement	
Sweeping – as Needed	
Comments during insp.	
Note corrective measures performed & date	
Silt Fence & Composite Sock– Monthly	
Comments during insp.	
Note corrective measures performed & date	
Temporary Basin Area	
as Needed	
Comments during insp.	
Note corrective measures performed & date	
Construction Entrance	
as Needed	
Comments during insp.	
Note corrective measures performed & date	
Inspector _____ Title _____ Date _____	
Address _____ Tel# _____	

WEEKLY
Inspection and Maintenance Log
DURING CONSTRUCTION

FOR: Main St.
 & After 1.0" Rain

Components _____	Date
Notify Cons. Comm. Issues effecting Resource Areas Comments during insp.	
Note corrective measures performed & date	
Silt of Public (Charlton Road) Streets – Daily Comments during insp.	
Note corrective measures performed & date	
Stockpile Materials Ring with Composite Sock – Weekly Comments during insp.	
Note corrective measures performed & date	
Any Spill Fuel, Chemical- Daily Comments during insp.	
Note corrective measures performed & date	
Temporary Ground Cover Area – Weekly Comments during insp.	
Note corrective measures performed & date	
Temporary Stone at Access Drive as Needed Comments during insp.	
Note corrective measures performed & date	
Inspector _____	Title _____
Address _____	Date _____
_____	Tel# _____

WEEKLY
Inspection and Maintenance Log
DURING CONSTRUCTION

FOR: Main St.
 & After 1.0" Rain

Components	Date
Lawn Area / Mulch Area	
Erosion, Washouts	
Comments during insp.	
Note corrective measures performed & date	
Stone Aprons at Outfalls Exit as Needed	
Comments during insp.	
Note corrective measures performed & date	
Forebay as Needed	
Comments during insp.	
Note corrective measures performed & date	
Illicit Drainage Discharge	
Comments during insp.	
Note corrective measures performed & date	
Comments during insp.	
Note corrective measures performed & date	
Inspector	Title Date
Address	Tel#

J-016

**Inspection and Maintenance Log
AFTER CONSTRUCTION**

**FOR: Main Street
& After 3.0" Rain**

Components	Date
Basin#1	
- twice a year	
Comments during insp.	
Note corrective measures performed & Date	
Basin#2	
-twice a year	
Comments during insp.	
Note corrective measures performed & date	
Forebay#1	
-twice a year	
Comments during insp.	
Note corrective measures performed & date	
Forebay#2	
-twice a year	
Comments during insp.	
Note corrective measures performed & date	
Catch Basins	
- 8 inches of sediment or twice a year	
Comments during insp.	
Note corrective measures performed & date	
Inspector	Title Date
Address	Tel#

J-016

**Inspection and Maintenance Log
AFTER CONSTRUCTION**

**FOR: Main Street
& After 3.0" Rain**

Components	Date
Outlet Control Structure#1	
-8 inches of sediment or twice a year	
Comments during insp.	
Note corrective measures performed & date	
Outlet Control Structure #2	
Twice a year	
Comments during insp.	
Note corrective measures performed & date	
Basin#3	
-four times a year	
Comments during insp.	
Note corrective measures performed & date	
Level Spreader Basin 1	
-Twice a year	
Comments during insp.	
Note corrective measures performed & date	
Level Spreader Basin 2	
-Twice a year	
Comments during insp.	
Note corrective measures performed & date	
All Flared end sections and rip rap aprons	
- twice a year	
Comments during insp.	
Note corrective measures performed & date	
Inspector	Title
Date	
Components	Date

47 Main Street, Upton Ma Project# J-016

Riprap Stone Sizing – Drainage Outfalls

Method – ARS Rock Chutes (Slopes 2%-40%)

Reference: National Engineering Handbook, TS14C-8

#1–Rock Apron- Basin#1 (HydroCAD)

Slope = 0.02 ft/ft

q (100-yr) = 5.62 cfs

$$D_{50} = 12(1.923qS^{1.5})^{0.529}$$
$$= D_{50} = 12(1.923 \times 5.62 \times 0.02^{1.5})^{0.529}$$

D₅₀ = 3" required

D₅₀ = 8" provided

$$L = (1.8 (Q-5)/D^{1.5} + 10) =$$
$$= (1.8 (5.62-5)/3^{1.5} + 10) = 10.0 \text{ feet}$$

$$W1 = 3D = 3(3) = 9 \text{ feet}$$

$$W2 = 3D + 0.7L = 3(3) + 0.7(10) = 12 \text{ feet}$$

#2–Rock Apron- Basin#2 (HydroCAD)

Slope = 0.029 ft/ft

q (100-yr) 4.56 cfs

$$D_{50} = 12(1.923qS^{1.5})^{0.529}$$
$$= D_{50} = 12(1.923 \times 4.56 \times 0.029^{1.5})^{0.529}$$

D₅₀ = 4" required

D₅₀ = 8" provided

$$L = (1.8 (Q-5)/D^{1.5} + 10) =$$
$$= (1.8 (4.56-5)/4^{1.5} + 10) = 10 \text{ feet}$$

$$W1 = 3D = 3(4) = 12 \text{ feet}$$

$$W2 = 3D + 0.7L = 3(4) + 0.7(10) = 12 \text{ feet}$$

#3–Rock Apron- Basin#3

Slope = 0.01 ft/ft

q (100-yr) 4.92 cfs

$$D_{50} = 12(1.923qS^{1.5})^{0.529}$$
$$= D_{50} = 12(1.923 \times 4.92 \times 0.01^{1.5})^{0.529}$$

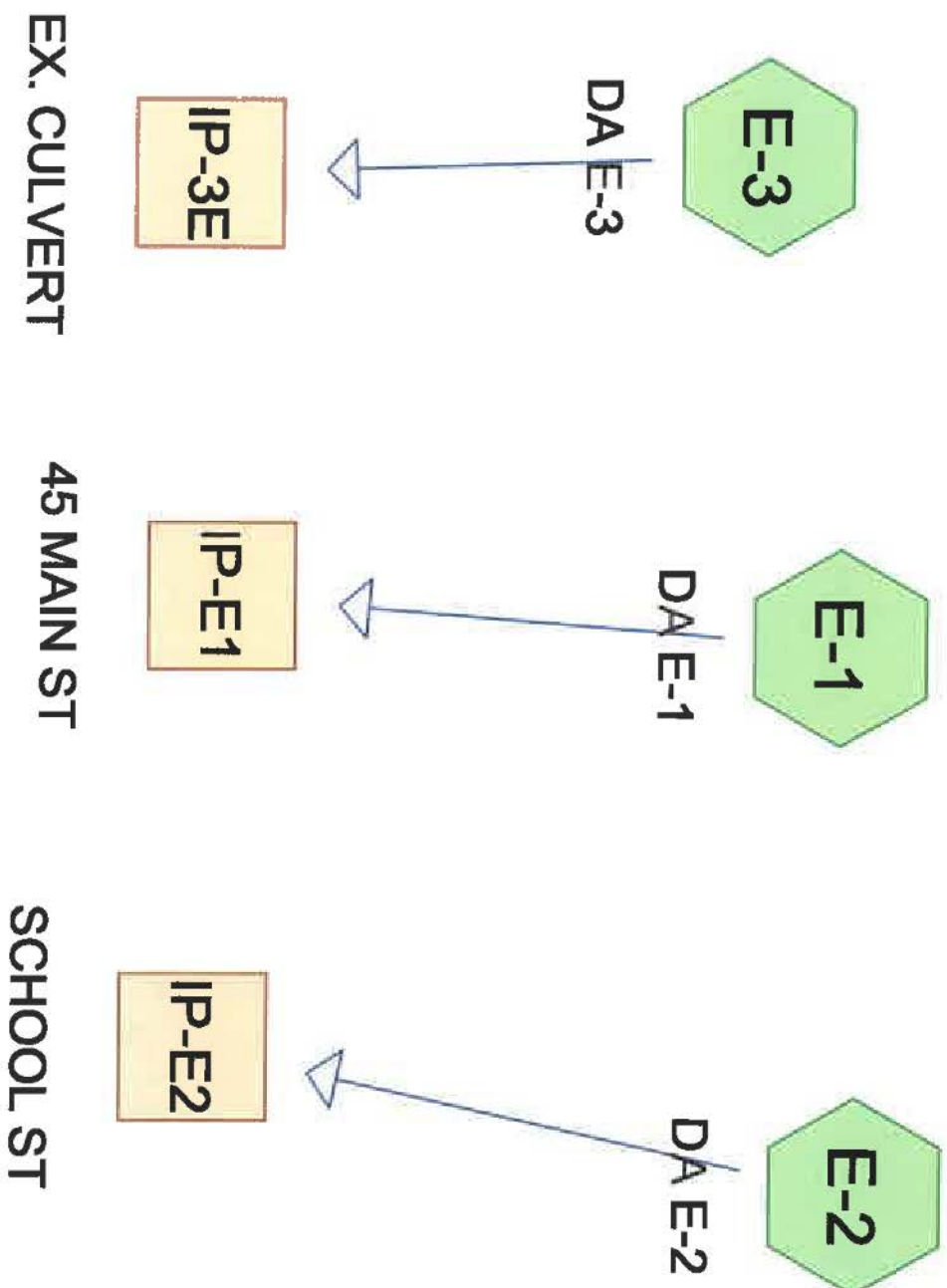
D₅₀ = 4" required

D₅₀ = 8" provided

$$L = (1.8 (Q-5)/D^{1.5} + 10) =$$
$$= (1.8 (4.92-5)/4^{1.5} + 10) = 10 \text{ feet}$$

$$W1 = 3D = 3(4) = 12 \text{ feet}$$

$$W2 = 3D + 0.7L = 3(4) + 0.7(10) = 12 \text{ feet}$$



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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2YR	Type III 24-hr		Default	24.00	1	3.39	2
2	10YR	Type III 24-hr		Default	24.00	1	5.11	2
3	25YR	Type III 24-hr		Default	24.00	1	6.19	2
4	100YR	Type III 24-hr		Default	24.00	1	7.85	2

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.530	74	>75% Grass cover, Good, HSG C (E-1, E-2)
0.278	98	Paved parking, HSG C (E-1, E-2)
7.258	70	Woods, Good, HSG C (E-1, E-2, E-3)
9.067	72	TOTAL AREA

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
9.067	HSG C	E-1, E-2, E-3
0.000	HSG D	
0.000	Other	
9.067		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	1.530	0.000	0.000	1.530	>75% Grass cover, Good	E-1, E-2
0.000	0.000	0.278	0.000	0.000	0.278	Paved parking	E-1, E-2
0.000	0.000	7.258	0.000	0.000	7.258	Woods, Good	E-1, E-2, E-3
0.000	0.000	9.067	0.000	0.000	9.067	TOTAL AREA	

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Type III 24-hr 2YR Rainfall=3.39"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E-1: DA E-1

Runoff Area=175,987 sf 4.02% Impervious Runoff Depth>0.95"
Flow Length=560' Tc=7.1 min CN=72 Runoff=4.43 cfs 0.321 af

Subcatchment E-2: DA E-2

Runoff Area=197,469 sf 2.56% Impervious Runoff Depth>0.90"
Flow Length=470' Tc=9.3 min CN=71 Runoff=4.30 cfs 0.341 af

Subcatchment E-3: DA E-3

Runoff Area=21,482 sf 0.00% Impervious Runoff Depth>0.85"
Tc=6.0 min CN=70 Runoff=0.49 cfs 0.035 af

Reach IP-3E: EX CULVERT

Inflow=0.49 cfs 0.035 af
Outflow=0.49 cfs 0.035 af

Reach IP-E1: 45 MAIN ST

Inflow=4.43 cfs 0.321 af
Outflow=4.43 cfs 0.321 af

Reach IP-E2: SCHOOL ST

Inflow=4.30 cfs 0.341 af
Outflow=4.30 cfs 0.341 af

Total Runoff Area = 9.067 ac Runoff Volume = 0.697 af Average Runoff Depth = 0.92"
96.93% Pervious = 8.788 ac 3.07% Impervious = 0.278 ac

PRE-DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Subcatchment E-1: DA E-1

Runoff = 4.43 cfs @ 12.11 hrs, Volume= 0.321 af, Depth> 0.95"
Routed to Reach IP-E1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)	CN	Description			
7,074	98	Paved parking, HSG C			
35,876	74	>75% Grass cover, Good, HSG C			
133,037	70	Woods, Good, HSG C			
175,987	72	Weighted Average			
168,913		95.98% Pervious Area			
7,074		4.02% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	40	0.0550	3.78		Shallow Concentrated Flow, TRAVEL PATH A TO B Unpaved Kv= 16.1 fps
0.5	110	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C Unpaved Kv= 16.1 fps
6.4	410	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D Woodland Kv= 5.0 fps
7.1	560	Total			

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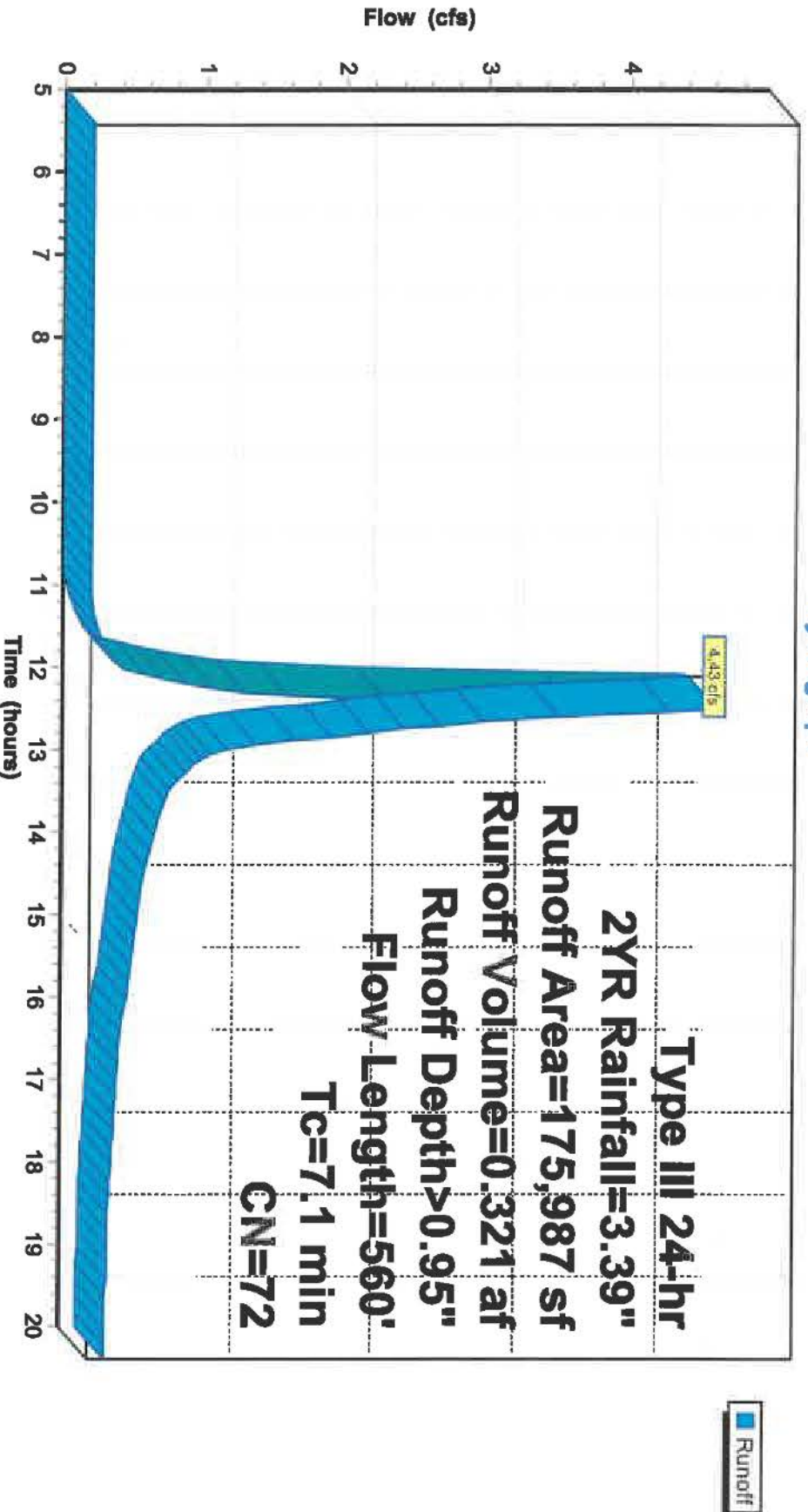
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment E-1: DA E-1

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Subcatchment E-2: DA E-2

Runoff = 4.30 cfs @ 12.15 hrs, Volume= 0.341 af, Depth> 0.90"
Routed to Reach IP-E2 : SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)	CN	Description
5,057	98	Paved parking, HSG C
30,786	74	>75% Grass cover, Good, HSG C
161,626	70	Woods, Good, HSG C
197,469	71	Weighted Average
192,412		97.44% Pervious Area
5,057		2.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	40	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n=0.150 P2=3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
2.9	225	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
2.6	125	0.0250	0.79		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Woodland Kv= 5.0 fps
9.3	470	Total			

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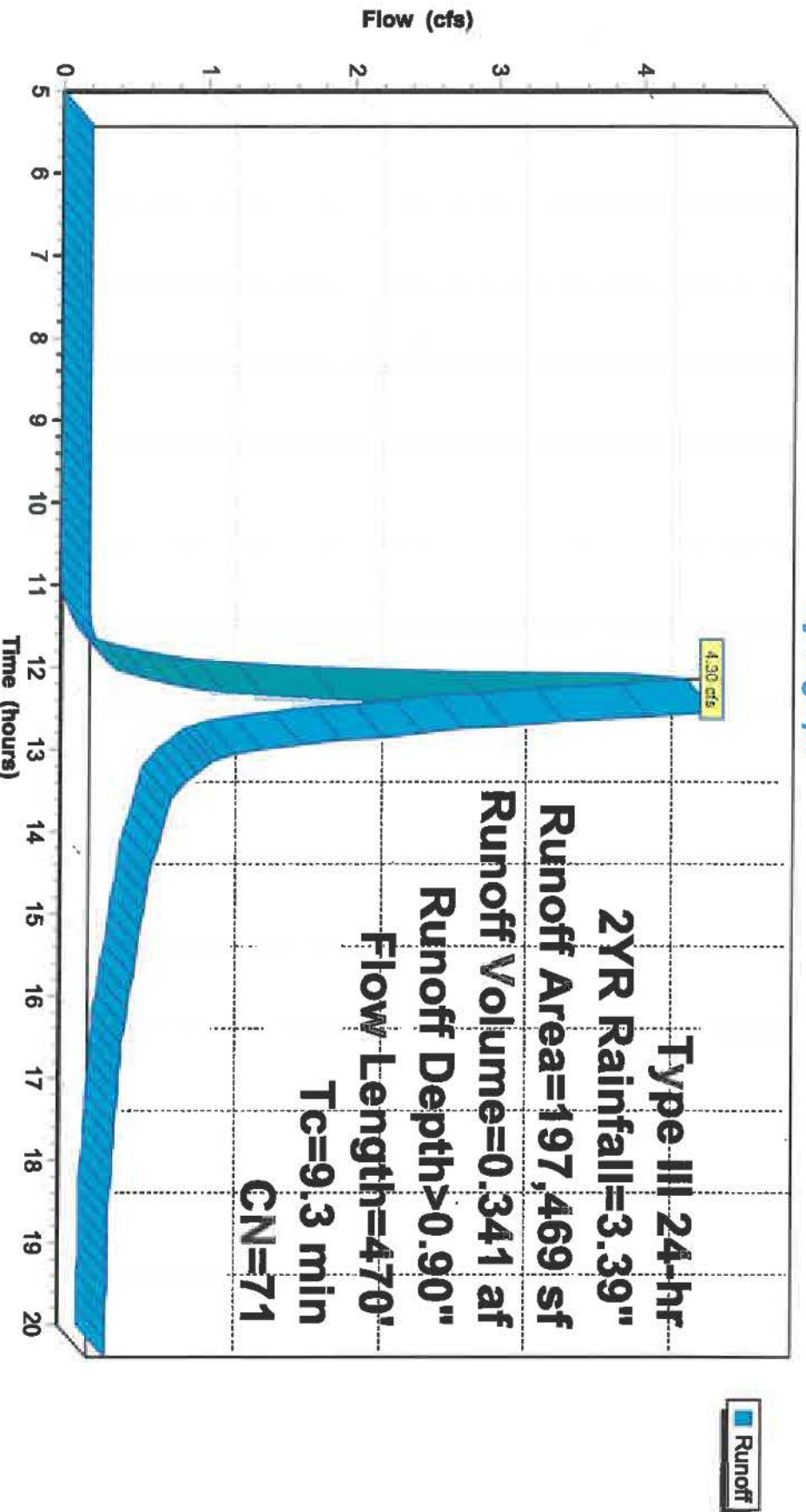
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment E-2: DA E-2

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"

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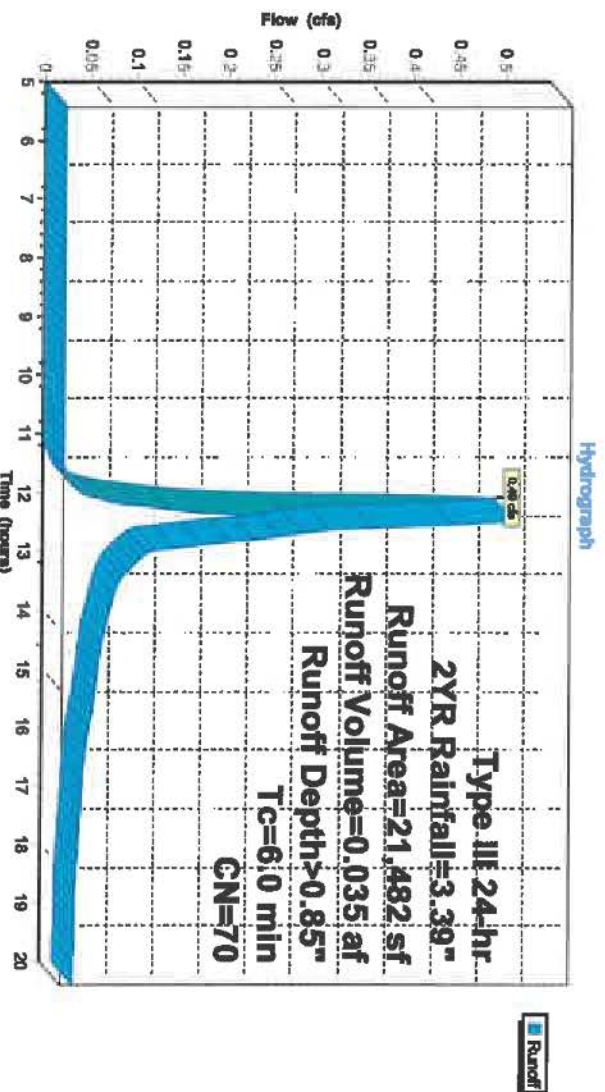
Summary for Subcatchment E-3: DA E-3

Runoff = 0.49 cfs @ 12.10 hrs, Volume= 0.035 af, Depth> 0.85"
Routed to Reach IP-3E : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)		CN	Description		
21,482	70	Woods, Good, HSG C			
21,482		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Subcatchment E-3: DA E-3



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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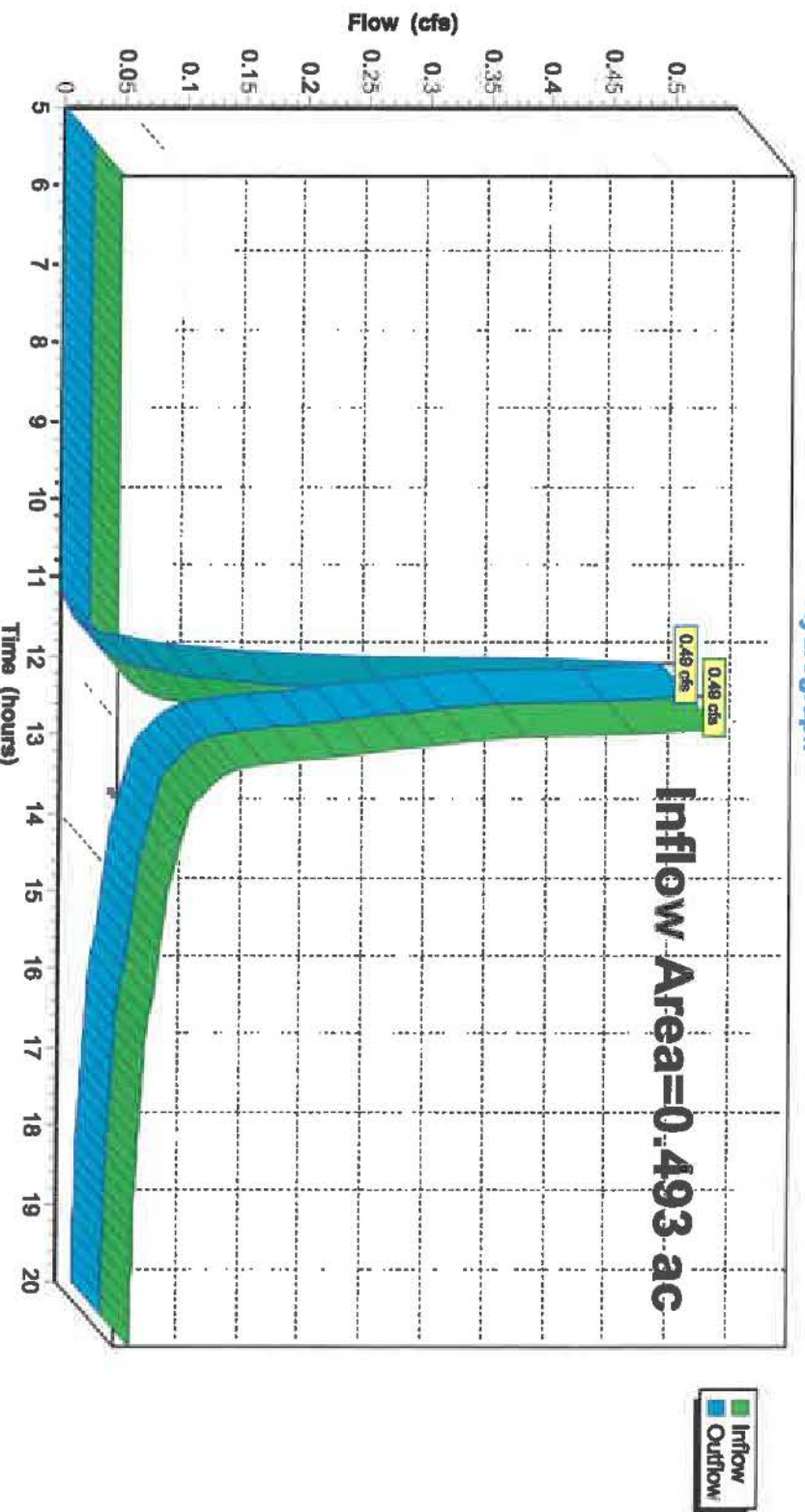
Summary for Reach IP-3E: EX. CULVERT

Inflow Area = 0.493 ac, 0.00% Impervious, Inflow Depth > 0.85" for 2YR event
Inflow = 0.49 cfs @ 12.10 hrs, Volume= 0.035 af
Outflow = 0.49 cfs @ 12.10 hrs, Volume= 0.035 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-3E: EX. CULVERT

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"

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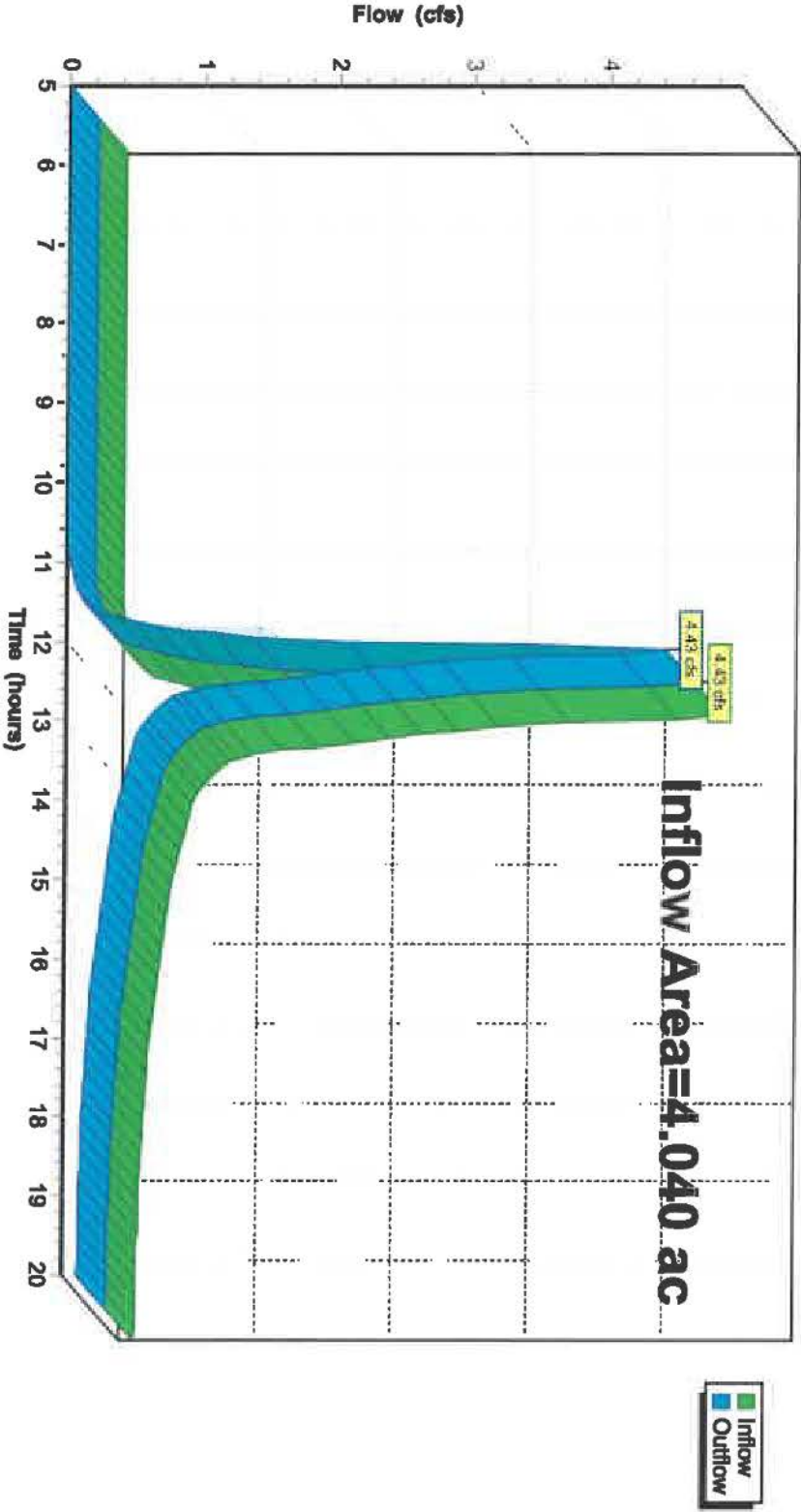
Summary for Reach IP-E1: 45 MAIN ST

Inflow Area = 4.040 ac, 4.02% Impervious, Inflow Depth > 0.95" for 2YR event
Inflow = 4.43 cfs @ 12.11 hrs, Volume= 0.321 af
Outflow = 4.43 cfs @ 12.11 hrs, Volume= 0.321 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E1: 45 MAIN ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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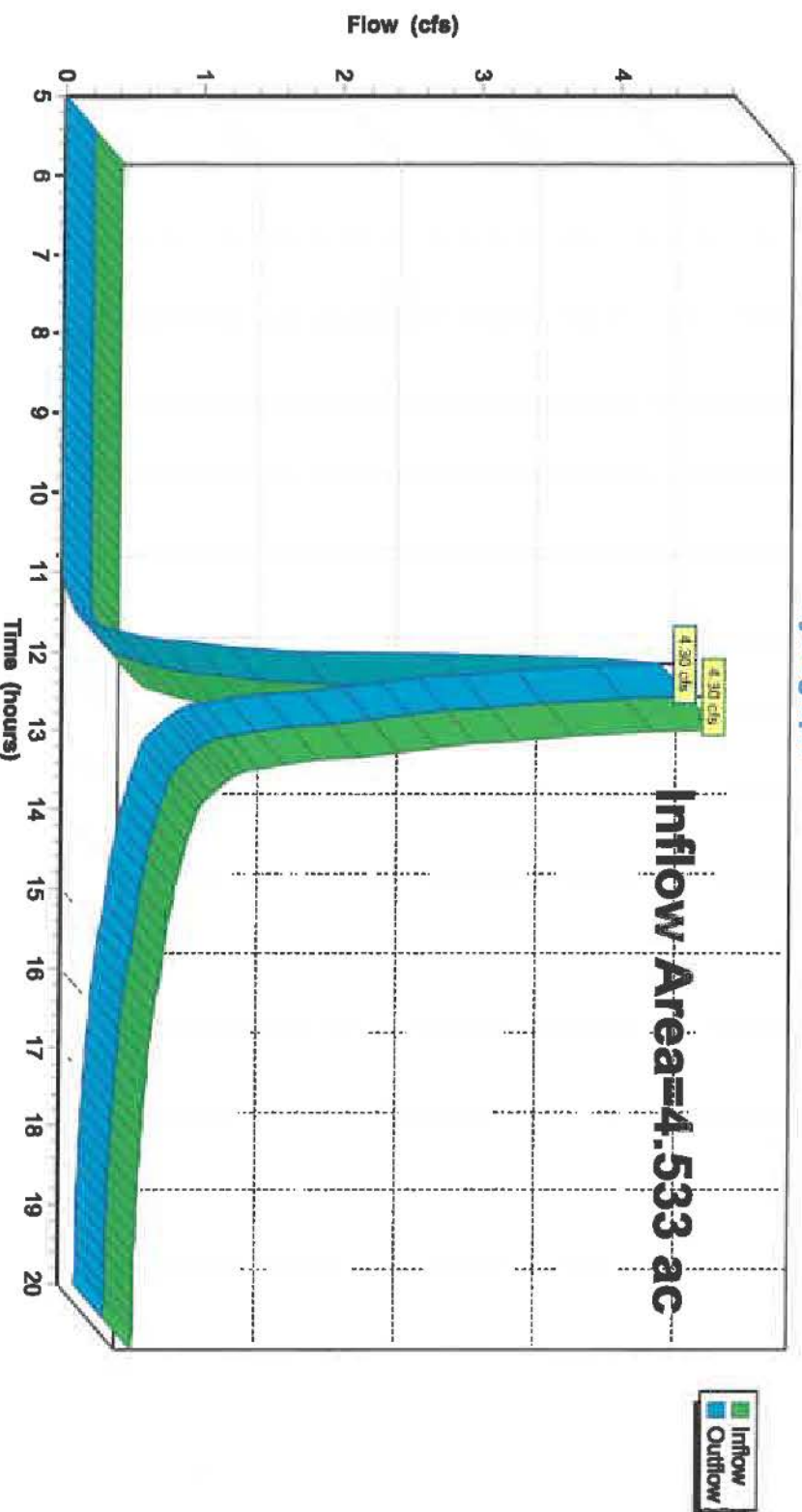
Summary for Reach IP-E2: SCHOOL ST

Inflow Area = 4.533 ac, 2.56% Impervious, Inflow Depth > 0.90" for 2YR event
Inflow = 4.30 cfs @ 12.15 hrs, Volume= 0.341 af
Outflow = 4.30 cfs @ 12.15 hrs, Volume= 0.341 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E2: SCHOOL ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 10YR Rainfall=5.1"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E-1: DA E-1

Runoff Area=175,987 sf 4.02% Impervious Runoff Depth>2.11"
Flow Length=560' Tc=7.1 min CN=72 Runoff=10.19 cfs 0.710 af

Subcatchment E-2: DA E-2

Runoff Area=197,469 sf 2.56% Impervious Runoff Depth>2.03"
Flow Length=470' Tc=9.3 min CN=71 Runoff=10.14 cfs 0.766 af

Subcatchment E-3: DA E-3

Runoff Area=21,482 sf 0.00% Impervious Runoff Depth>1.95"
Tc=6.0 min CN=70 Runoff=1.18 cfs 0.080 af

Reach IP-3E: EX. CULVERT

Inflow=1.18 cfs 0.080 af
Outflow=1.18 cfs 0.080 af

Reach IP-E1: 45 MAIN ST

Inflow=10.19 cfs 0.710 af
Outflow=10.19 cfs 0.710 af

Reach IP-E2: SCHOOL ST

Inflow=10.14 cfs 0.766 af
Outflow=10.14 cfs 0.766 af

Total Runoff Area = 9.067 ac Runoff Volume = 1.556 af Average Runoff Depth = 2.06"
96.93% Pervious = 8.788 ac 3.07% Impervious = 0.278 ac

PRE-DEVELOPMENT1-8-24

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Type III 24-hr 10YR Rainfall=5.11"
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Summary for Subcatchment E-1: DA E-1

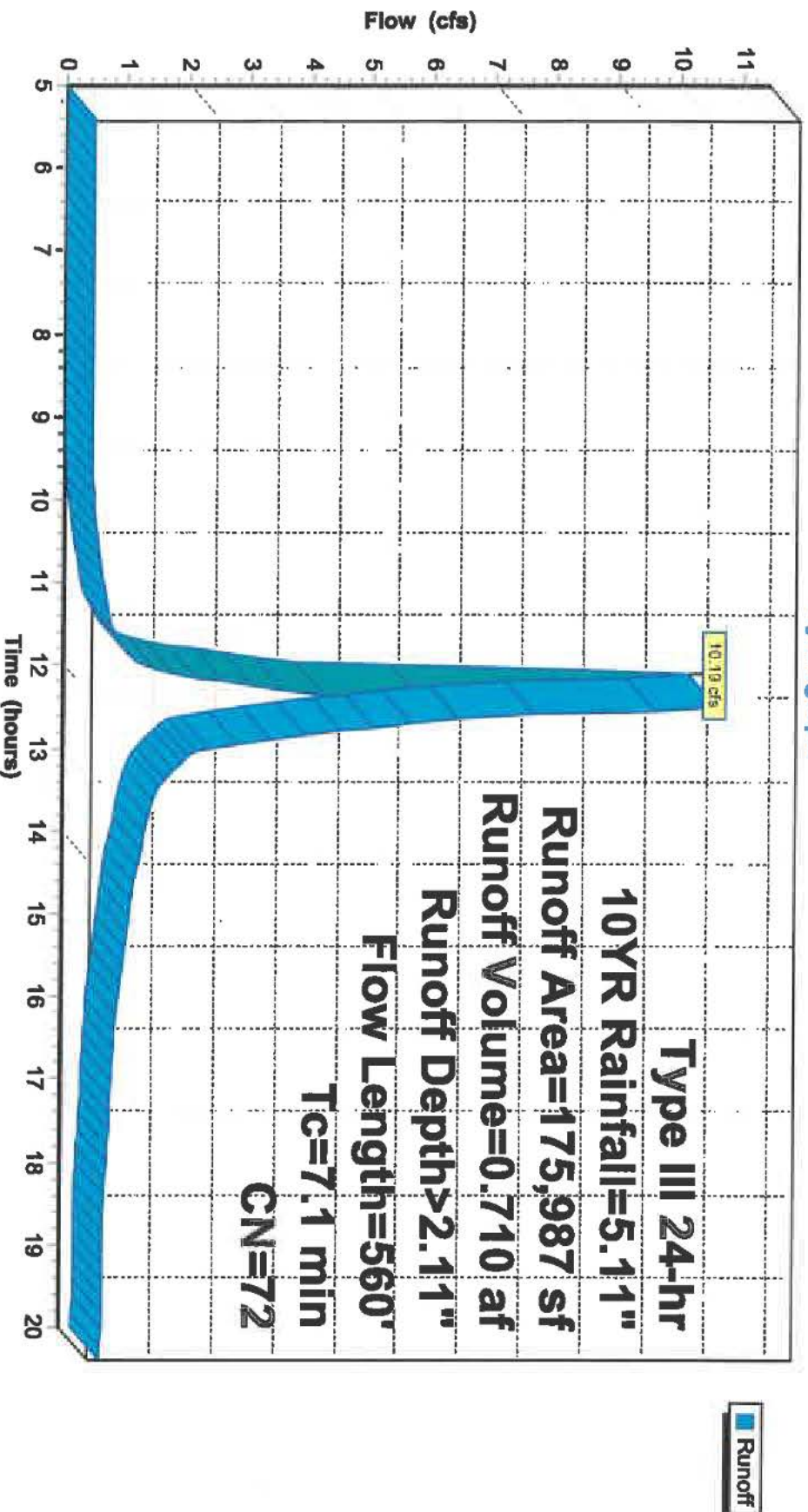
Runoff = 10.19 cfs @ 12.11 hrs, Volume= 0.710 af, Depth> 2.11"
Routed to Reach IP-E1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)	CN	Description		
7,074	98	Paved parking, HSG C		
35,876	74	>75% Grass cover, Good, HSG C		
133,037	70	Woods, Good, HSG C		
175,987	72	Weighted Average		
168,913		95.98% Pervious Area		
7,074		4.02% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft) Velocity (ft/sec) Capacity (cfs) Description		
0.2	40	0.0550 3.78	Shallow Concentrated Flow, TRAVEL PATH A TO B Unpaved Kv= 16.1 fps	
0.5	110	0.0450 3.42	Shallow Concentrated Flow, TRAVEL PATH B TO C Unpaved Kv= 16.1 fps	
6.4	410	0.0450 1.06	Shallow Concentrated Flow, TRAVEL PATH C TO D Woodland Kv= 5.0 fps	
7.1	560	Total		

Subcatchment E-1: DA E-1

Hydrograph



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Summary for Subcatchment E-2: DA E-2

Runoff = 10.14 cfs @ 12.14 hrs, Volume= 0.766 af, Depth> 2.03"
Routed to Reach IP-E2: SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)	CN	Description
5,057	98	Paved parking, HSG C
30,786	74	>75% Grass cover, Good, HSG C
161,626	70	Woods, Good, HSG C
197,469	71	Weighted Average
192,412		97.44% Pervious Area
5,057		2.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	40	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n=0.150 P2=3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
2.9	225	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
2.6	125	0.0250	0.79		Shallow Concentrated Flow, TARVEL PATH D TO E
					Woodland Kv= 5.0 fps
9.3	470	Total			

PRE-DEVELOPMENT1-8-24

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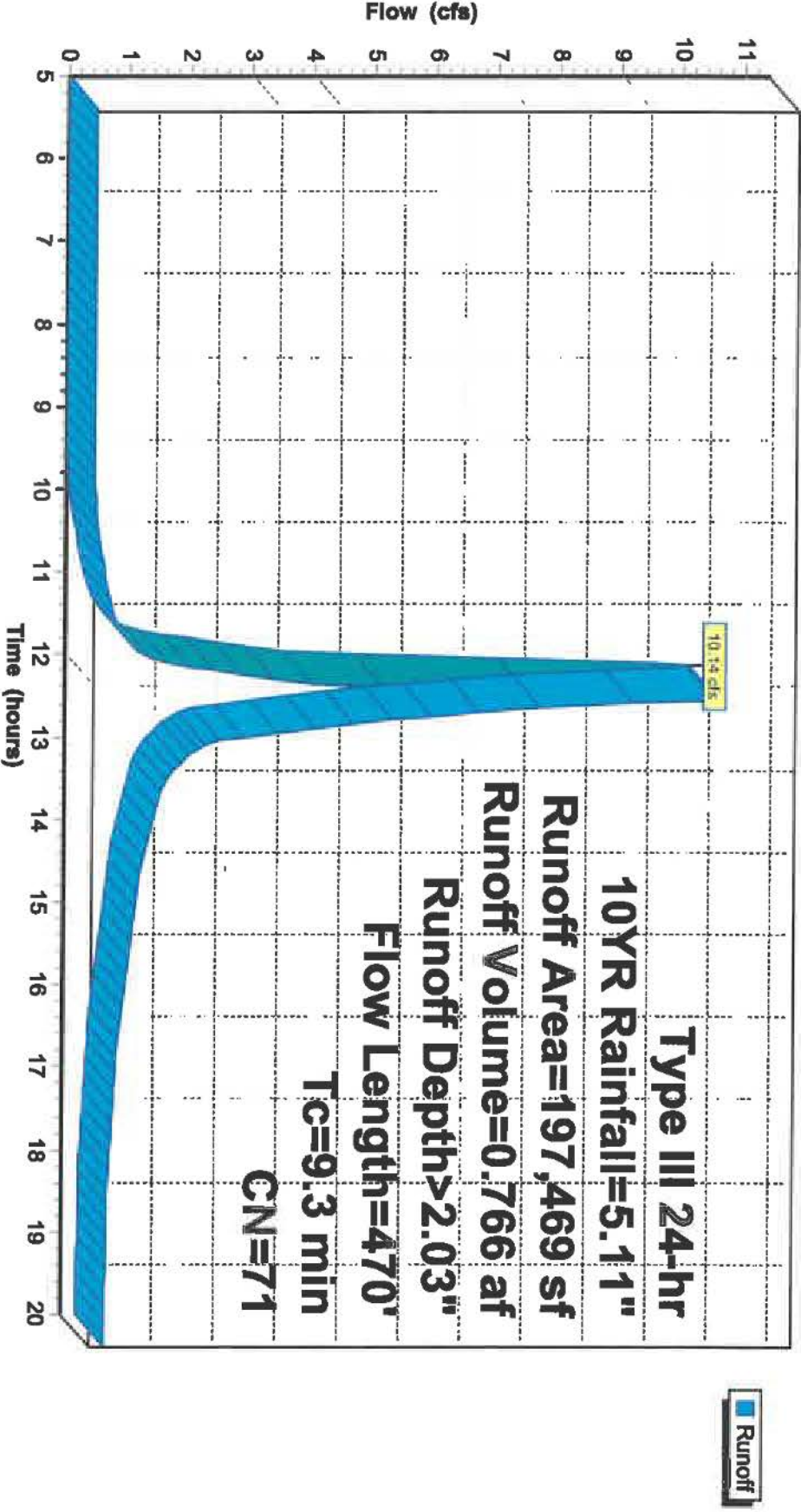
Type III 24-hr 10YR Rainfall=5.11"

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Subcatchment E-2: DA E-2

Hydrograph



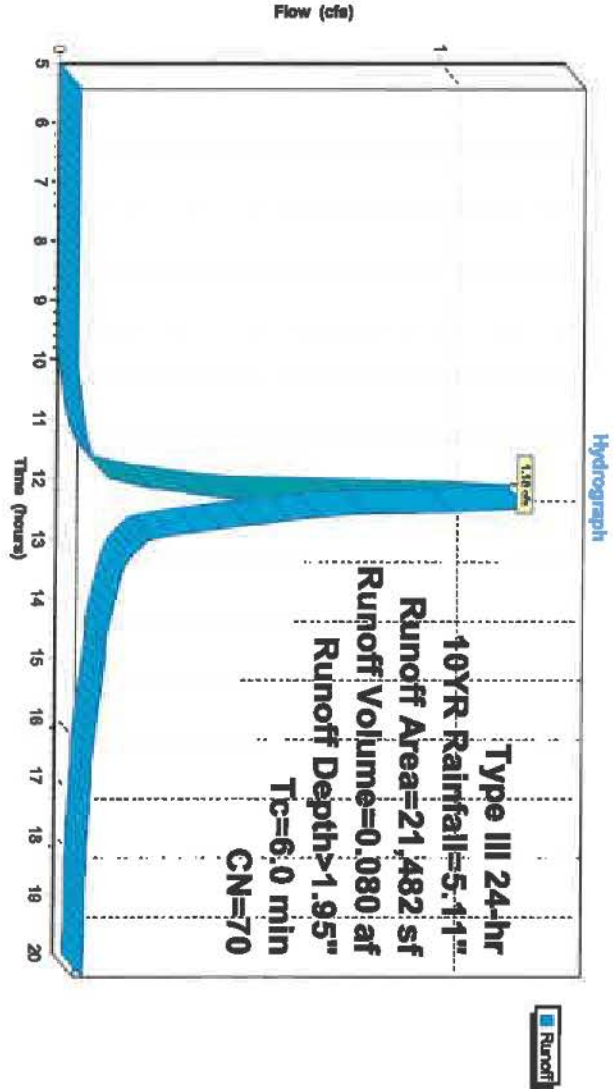
Summary for Subcatchment E-3: DA E-3

Runoff = 1.18 cfs @ 12.10 hrs, Volume= 0.080 af, Depth> 1.95"
Routed to Reach IP-3E : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)		CN	Description		
21,482	70	Woods, Good, HSG C			
21,482		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Subcatchment E-3: DA E-3



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Type III 24-hr 10YR Rainfall=5.11"

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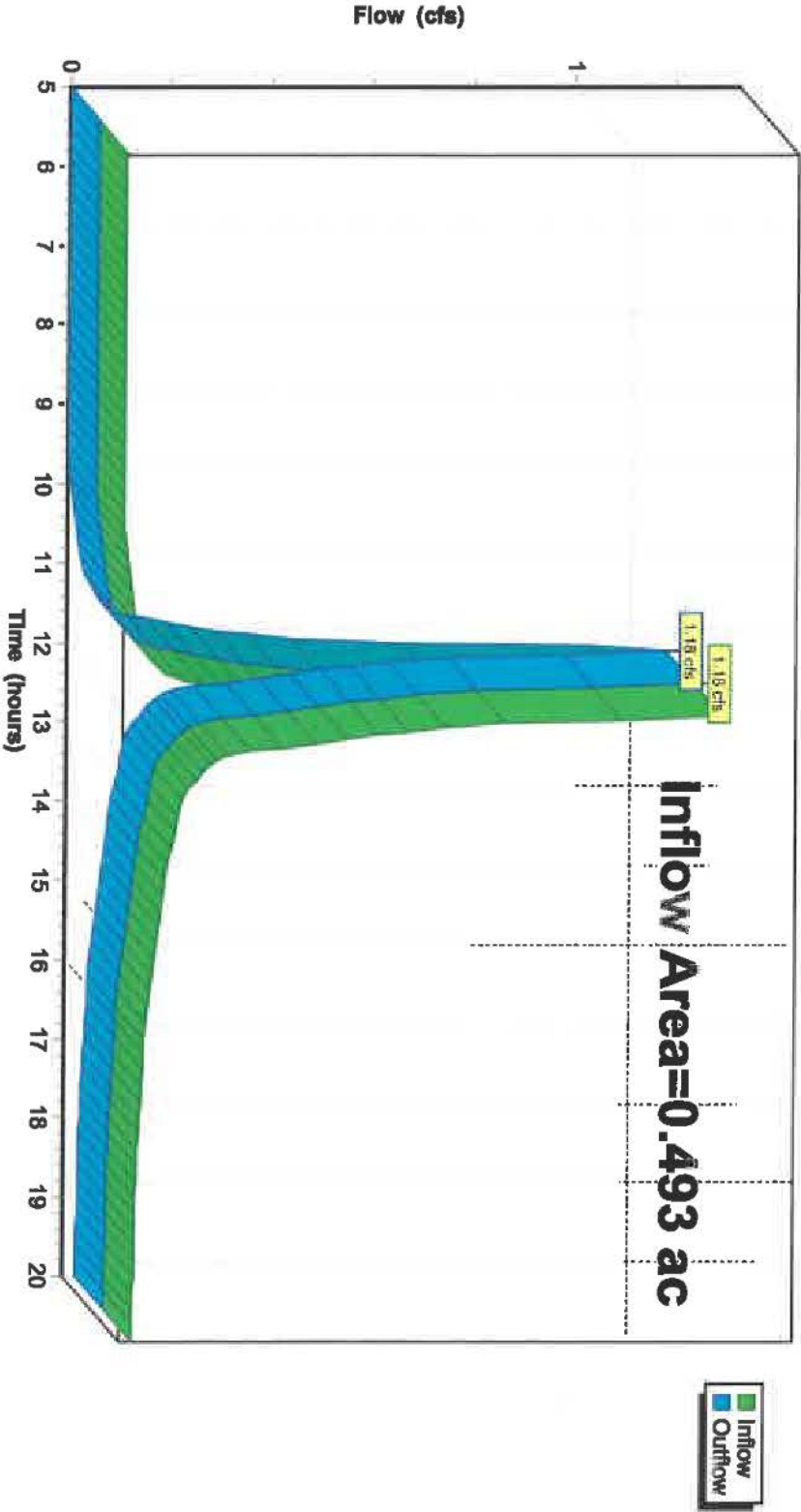
Summary for Reach IP-3E: EX. CULVERT

Inflow Area = 0.493 ac, 0.00% Impervious, Inflow Depth > 1.95" for 10YR event
Inflow = 1.18 cfs @ 12.10 hrs, Volume= 0.080 af
Outflow = 1.18 cfs @ 12.10 hrs, Volume= 0.080 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-3E: EX. CULVERT

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 10YR Rainfall=5.11"

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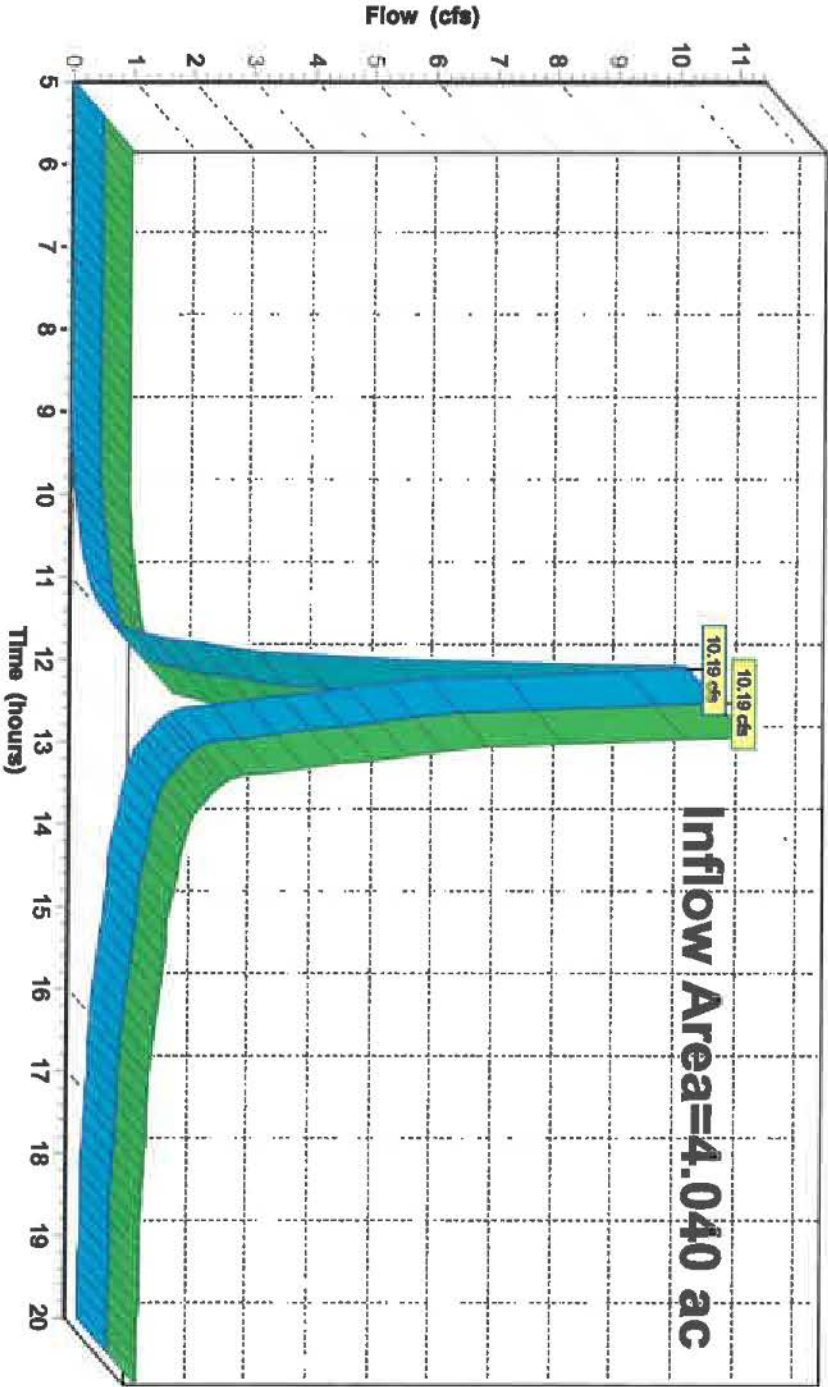
Summary for Reach IP-E1: 45 MAIN ST

Inflow Area = 4.040 ac, 4.02% Impervious, Inflow Depth > 2.11" for 10YR event
Inflow = 10.19 cfs @ 12.11 hrs, Volume= 0.710 af
Outflow = 10.19 cfs @ 12.11 hrs, Volume= 0.710 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E1: 45 MAIN ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 10YR Rainfall=5.11"

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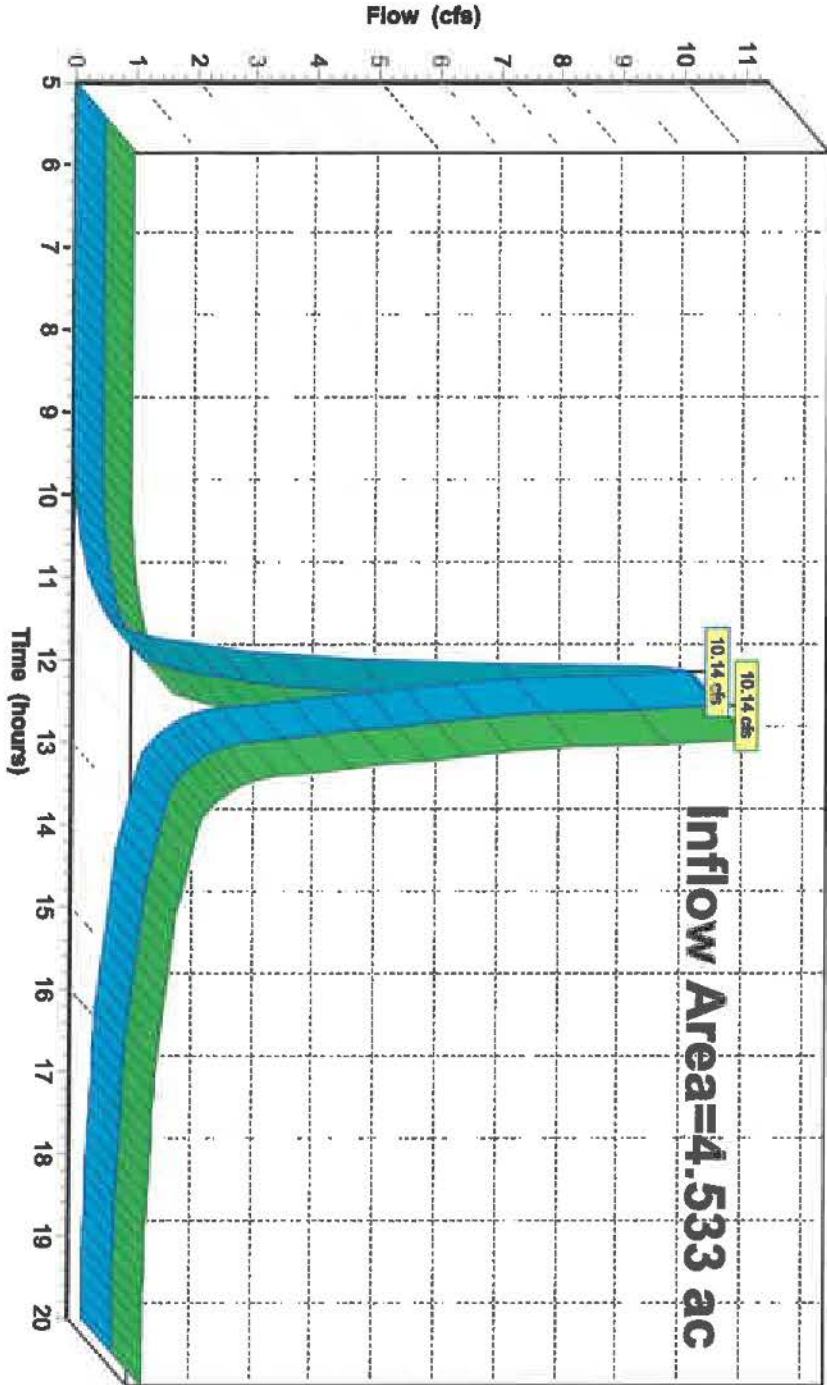
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Summary for Reach IP-E2: SCHOOL ST

Inflow Area = 4.533 ac, 2.56% Impervious, Inflow Depth > 2.03" for 10YR event
Inflow = 10.14 cfs @ 12.14 hrs, Volume= 0.766 af
Outflow = 10.14 cfs @ 12.14 hrs, Volume= 0.766 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E2: SCHOOL ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 25YR Rainfall=6.19"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E-1: DA E-1

Runoff Area=175,987 sf 4.02% Impervious Runoff Depth>2.93"
Flow Length=560' Tc=7.1 min CN=72 Runoff=14.16 cfs 0.985 af

Subcatchment E-2: DA E-2

Runoff Area=197,469 sf 2.56% Impervious Runoff Depth>2.83"
Flow Length=470' Tc=9.3 min CN=71 Runoff=14.21 cfs 1.069 af

Subcatchment E-3: DA E-3

Runoff Area=21,482 sf 0.00% Impervious Runoff Depth>2.74"
Tc=6.0 min CN=70 Runoff=1.67 cfs 0.113 af

Reach IP-3E: EX. CULVERT

Inflow=1.67 cfs 0.113 af
Outflow=1.67 cfs 0.113 af

Reach IP-E1: 45 MAIN ST

Inflow=14.16 cfs 0.985 af
Outflow=14.16 cfs 0.985 af

Reach IP-E2: SCHOOL ST

Inflow=14.21 cfs 1.069 af
Outflow=14.21 cfs 1.069 af

Total Runoff Area = 9.067 ac Runoff Volume = 2.167 af Average Runoff Depth = 2.87"
96.93% Pervious = 8.788 ac 3.07% Impervious = 0.278 ac

PRE-DEVELOPMENT1-8-24

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Type III 24-hr 25YR Rainfall=6.19"
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Summary for Subcatchment E-1: DA E-1

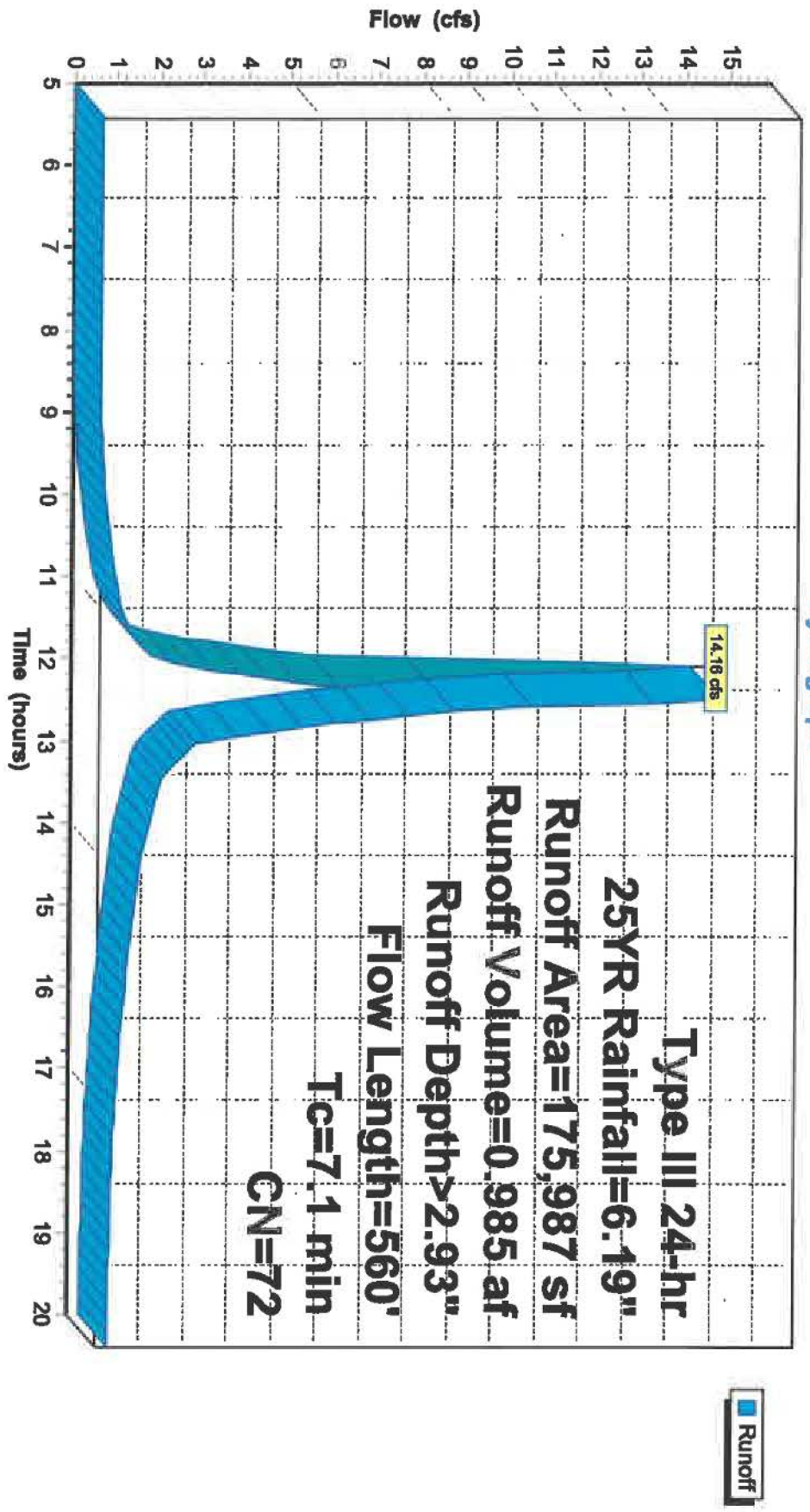
Runoff = 14.16 cfs @ 12.11 hrs, Volume= 0.985 af, Depth> 2.93"
Routed to Reach IP-E1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)	CN	Description			
7,074	98	Paved parking, HSG C			
35,876	74	>75% Grass cover, Good, HSG C			
133,037	70	Woods, Good, HSG C			
175,987	72	Weighted Average			
168,913		95.98% Pervious Area			
7,074		4.02% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	40	0.0550	3.78		Shallow Concentrated Flow, TRAVEL PATH A TO B Unpaved Kv= 16.1 fps
0.5	110	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C Unpaved Kv= 16.1 fps
6.4	410	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D Woodland Kv= 5.0 fps
7.1	560	Total			

Subcatchment E-1: DA E-1

Hydrograph



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Type III 24-hr 25YR Rainfall=6.19"

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Summary for Subcatchment E-2: DA E-2

Runoff = 14.21 cfs @ 12.14 hrs, Volume= 1.069 af, Depth> 2.83"
Routed to Reach IP-E2: SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)						CN	Description
5,057	98	Paved parking, HSG C					
30,786	74	>75% Grass cover, Good, HSG C					
161,626	70	Woods, Good, HSG C					
197,469	71	Weighted Average					
192,412		97.44% Pervious Area					
5,057		2.56% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
3.4	40	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B		
					Grass: Short n= 0.150 P2=3.20"		
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C		
					Unpaved Kv= 16.1 fps		
2.9	225	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D		
					Woodland Kv= 5.0 fps		
2.6	125	0.0250	0.79		Shallow Concentrated Flow, TARVEL PATH D TO E		
					Woodland Kv= 5.0 fps		
9.3	470	Total					

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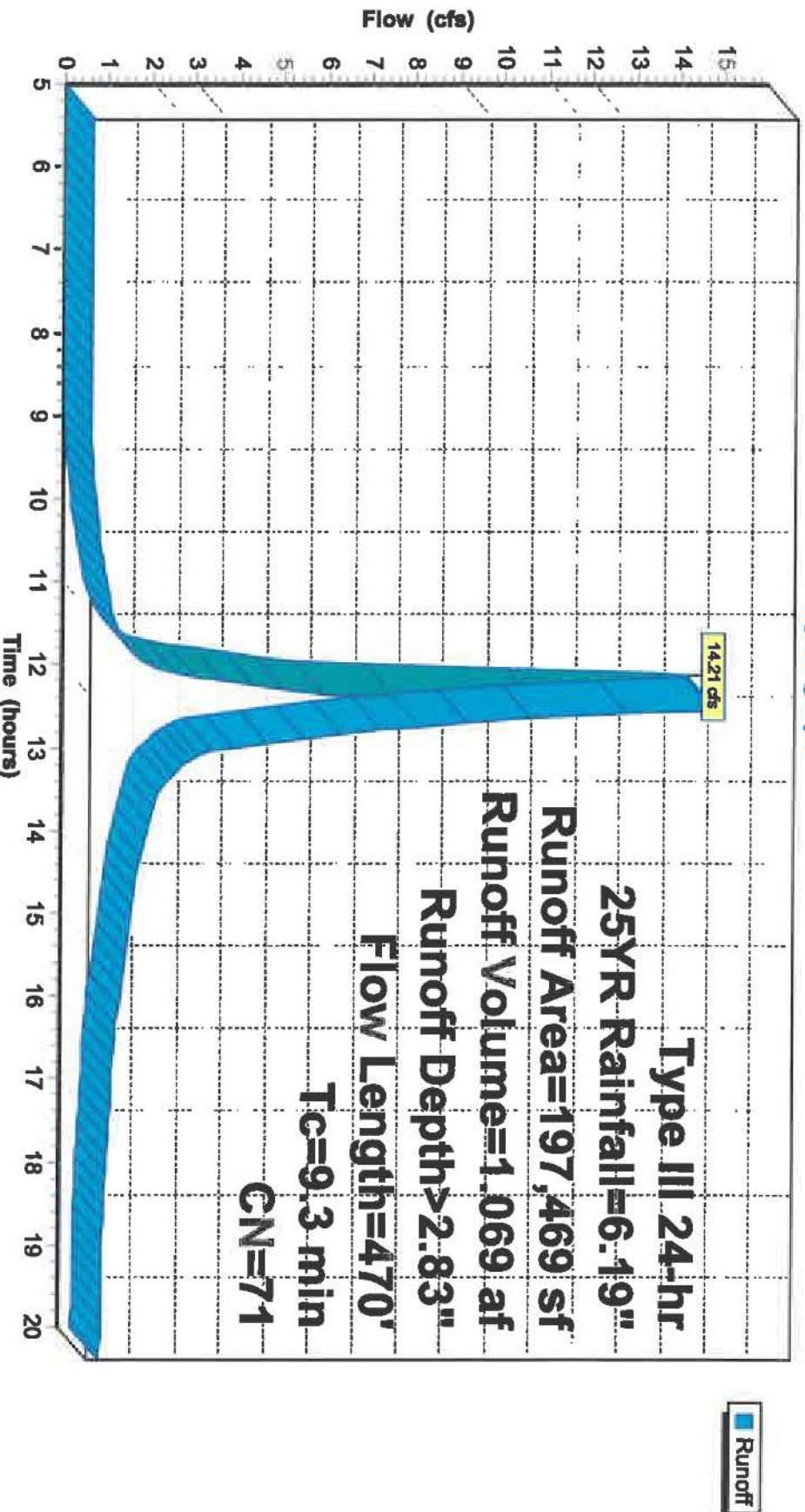
Type III 24-hr 25YR Rainfall=6.19"

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Subcatchment E-2: DA E-2

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Type III 24-hr 25YR Rainfall=6.19"

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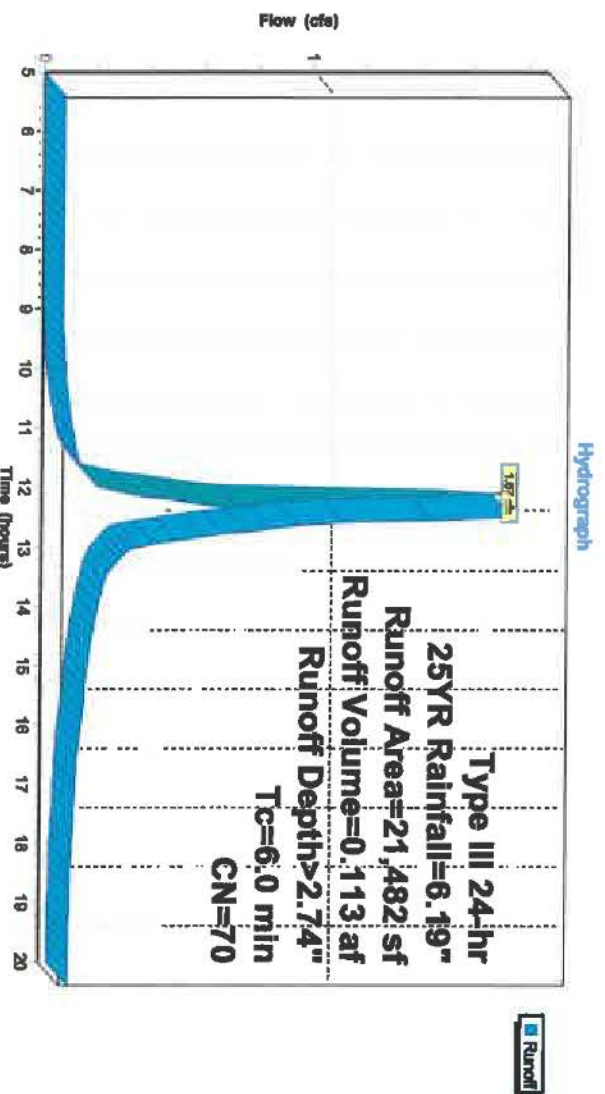
Summary for Subcatchment E-3: DA E-3

Runoff = 1.67 cfs @ 12.09 hrs, Volume= 0.113 af, Depth> 2.74"
Routed to Reach IP-3E : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)		CN	Description		
21,482		70	Woods, Good, HSG C		
21,482			100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Subcatchment E-3: DA E-3



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Type III 24-hr 25YR Rainfall=6.19"

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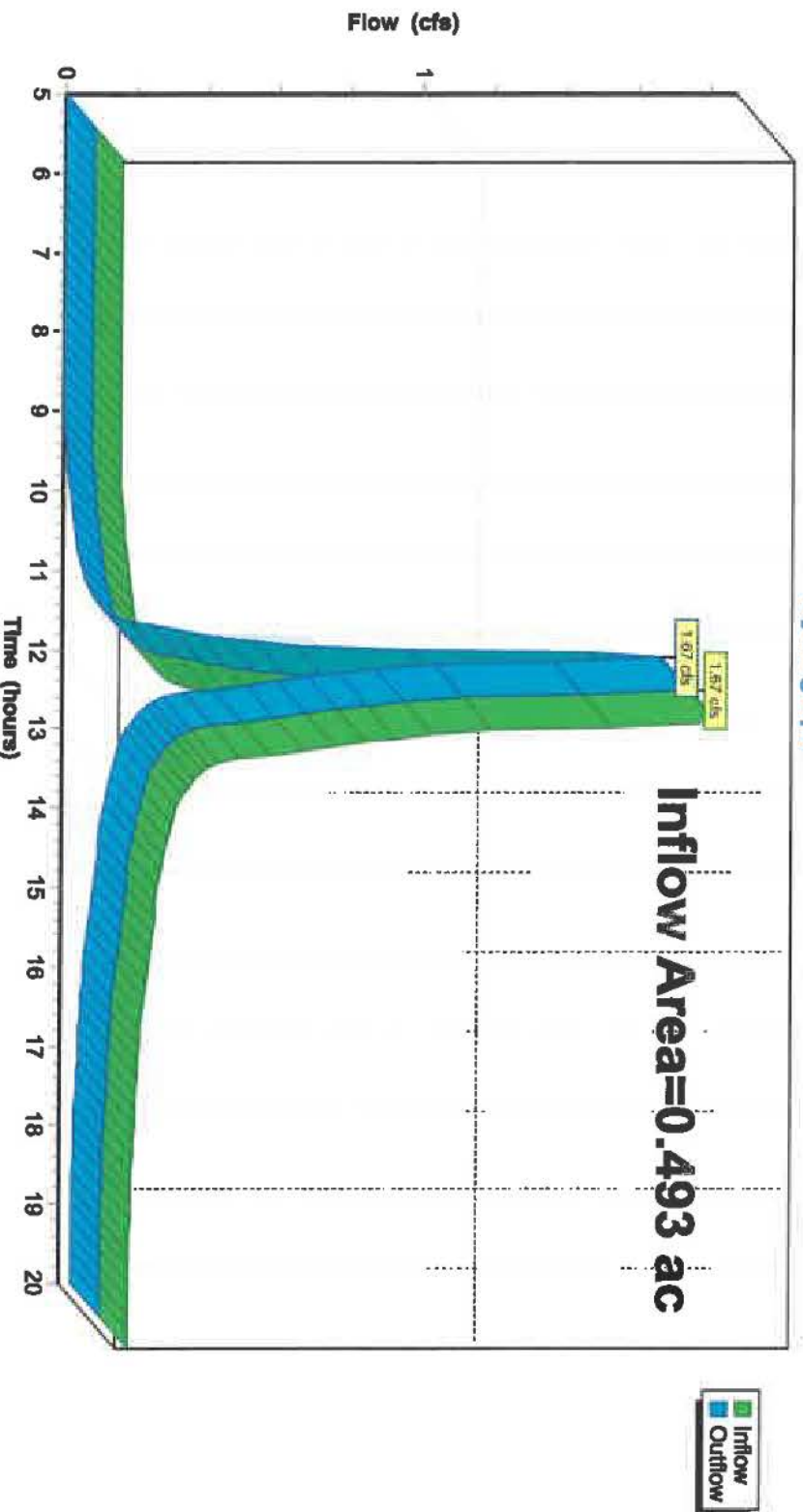
Summary for Reach IP-3E: EX. CULVERT

Inflow Area = 0.493 ac, 0.00% Impervious, Inflow Depth > 2.74" for 25YR event
Inflow = 1.67 cfs @ 12.09 hrs, Volume= 0.113 af
Outflow = 1.67 cfs @ 12.09 hrs, Volume= 0.113 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-3E: EX. CULVERT

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 25YR Rainfall=6.19"

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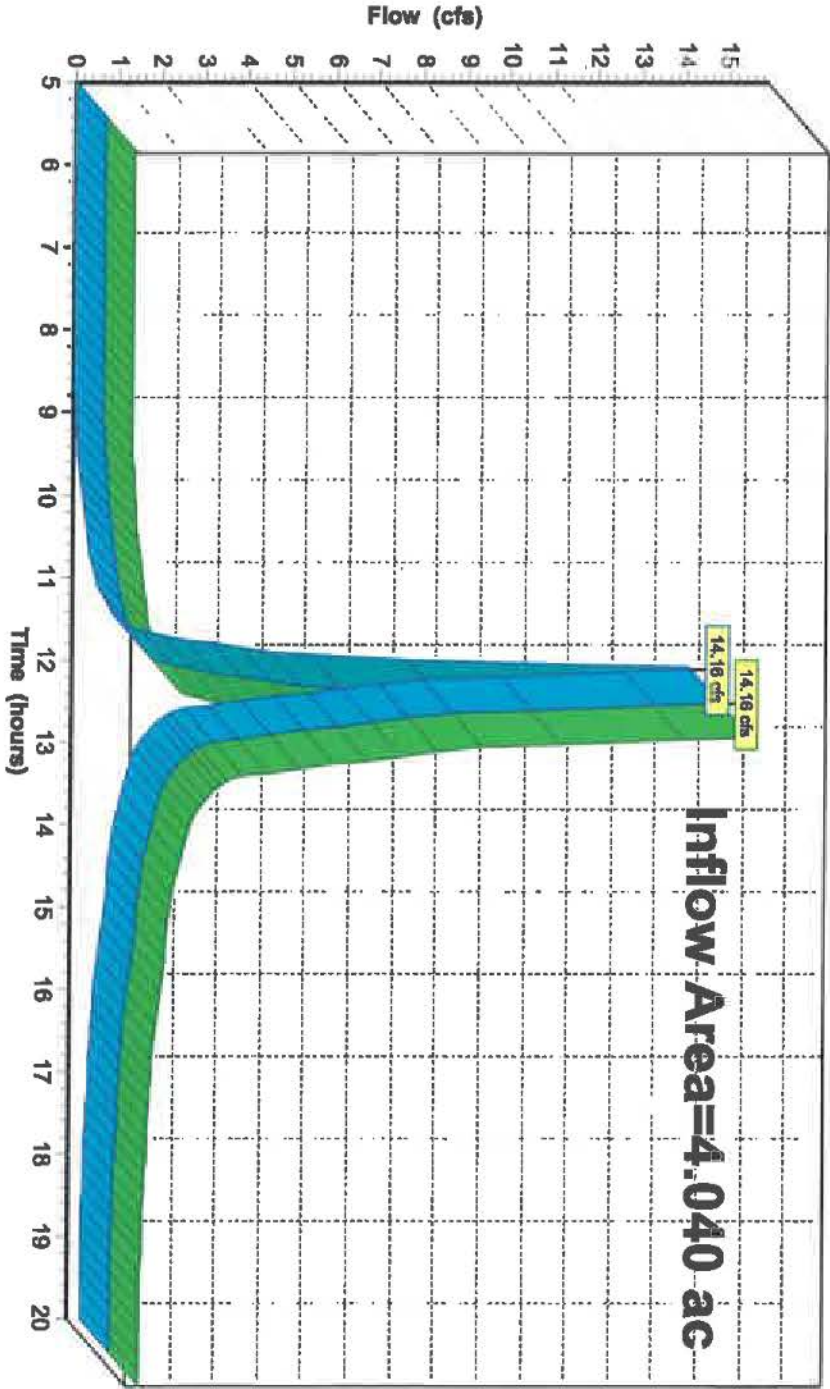
Summary for Reach IP-E1: 45 MAIN ST

Inflow Area = 4.040 ac, 4.02% Impervious, Inflow Depth > 2.93" for 25YR event
Inflow = 14.16 cfs @ 12.11 hrs, Volume= 0.985 af
Outflow = 14.16 cfs @ 12.11 hrs, Volume= 0.985 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E1: 45 MAIN ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 25YR Rainfall=6.19"

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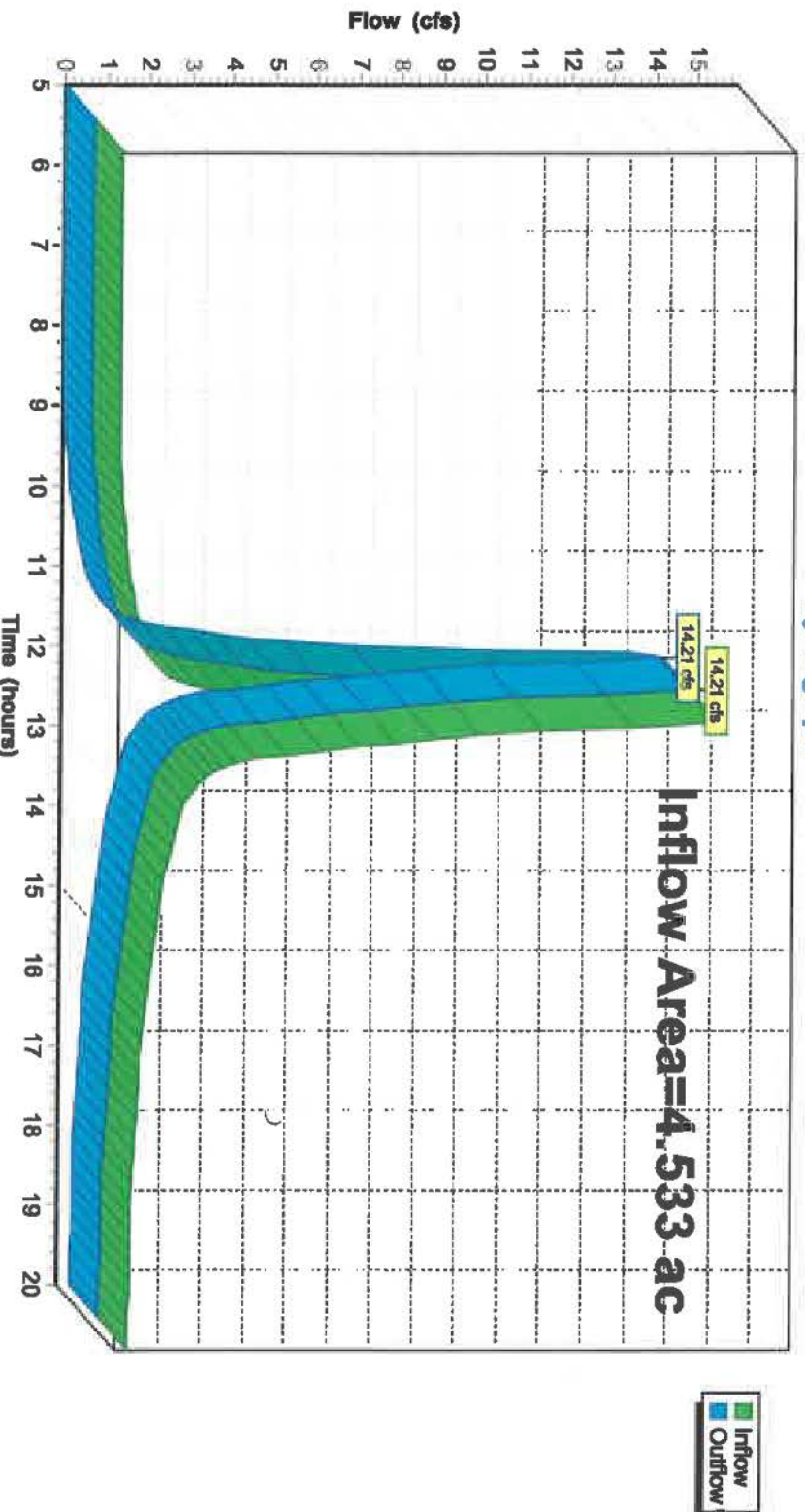
Page 32

Summary for Reach IP-E2: SCHOOL ST

Inflow Area = 4.533 ac, 2.56% Impervious, Inflow Depth > 2.83" for 25YR event
Inflow = 14.21 cfs @ 12.14 hrs, Volume= 1.069 af
Outflow = 14.21 cfs @ 12.14 hrs, Volume= 1.069 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E2: SCHOOL ST

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 100YR Rainfall=7.85"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E-1: DA E-1

Runoff Area=175,987 sf 4.02% Impervious Runoff Depth>4.26"
Flow Length=560' Tc=7.1 min CN=72 Runoff=20.54 cfs 1.435 af

Subcatchment E-2: DA E-2

Runoff Area=197,469 sf 2.56% Impervious Runoff Depth>4.15"
Flow Length=470' Tc=9.3 min CN=71 Runoff=20.77 cfs 1.567 af

Subcatchment E-3: DA E-3

Runoff Area=21,482 sf 0.00% Impervious Runoff Depth>4.04"
Tc=6.0 min CN=70 Runoff=2.46 cfs 0.166 af

Reach IP-3E: EX. CULVERT

Inflow=2.46 cfs 0.166 af
Outflow=2.46 cfs 0.166 af

Reach IP-E1: 45 MAIN ST

Inflow=20.54 cfs 1.435 af
Outflow=20.54 cfs 1.435 af

Reach IP-E2: SCHOOL ST

Inflow=20.77 cfs 1.567 af
Outflow=20.77 cfs 1.567 af

Total Runoff Area = 9.067 ac Runoff Volume = 3.168 af Average Runoff Depth = 4.19"
96.93% Pervious = 8.788 ac 3.07% Impervious = 0.278 ac

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Summary for Subcatchment E-1: DA E-1

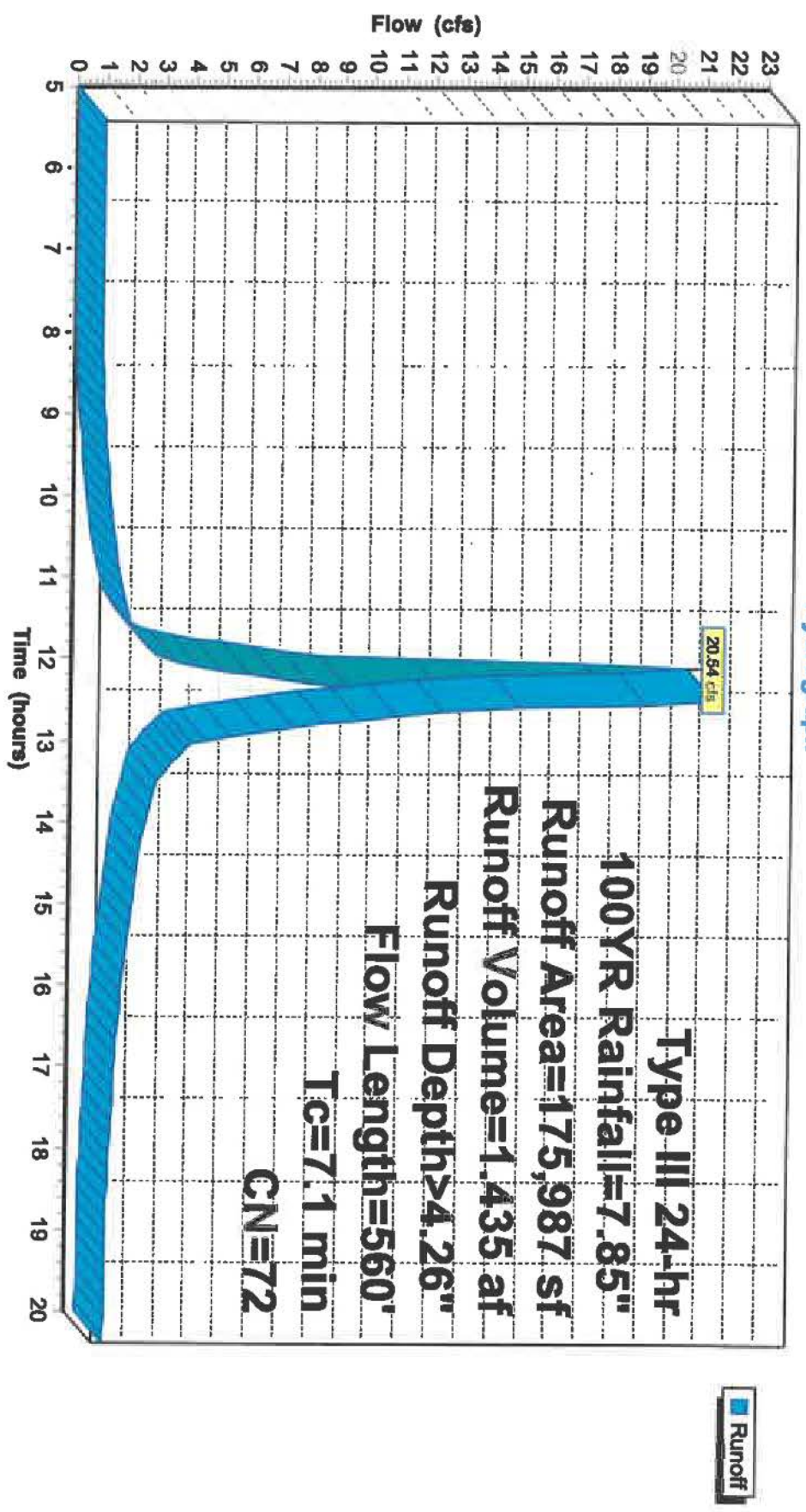
Runoff = 20.54 cfs @ 12.10 hrs, Volume= 1.435 af, Depth> 4.26"
Routed to Reach IP-E1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)	CN	Description			
7,074	98	Paved parking, HSG C			
35,876	74	>75% Grass cover, Good, HSG C			
133,037	70	Woods, Good, HSG C			
175,987	72	Weighted Average			
168,913		95.98% Pervious Area			
7,074		4.02% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	40	0.0550	3.78		Shallow Concentrated Flow, TRAVEL PATH A TO B Unpaved Kv= 16.1 fps
0.5	110	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C Unpaved Kv= 16.1 fps
6.4	410	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D Woodland Kv= 5.0 fps
7.1	560	Total			

Subcatchment E-1: DA E-1

Hydrograph



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Type III 24-hr 100YR Rainfall=7.85"
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Summary for Subcatchment E-2: DA E-2

Runoff = 20.77 cfs @ 12.13 hrs, Volume= 1.567 af, Depth> 4.15"
Routed to Reach IP-E2: SCHOOL ST

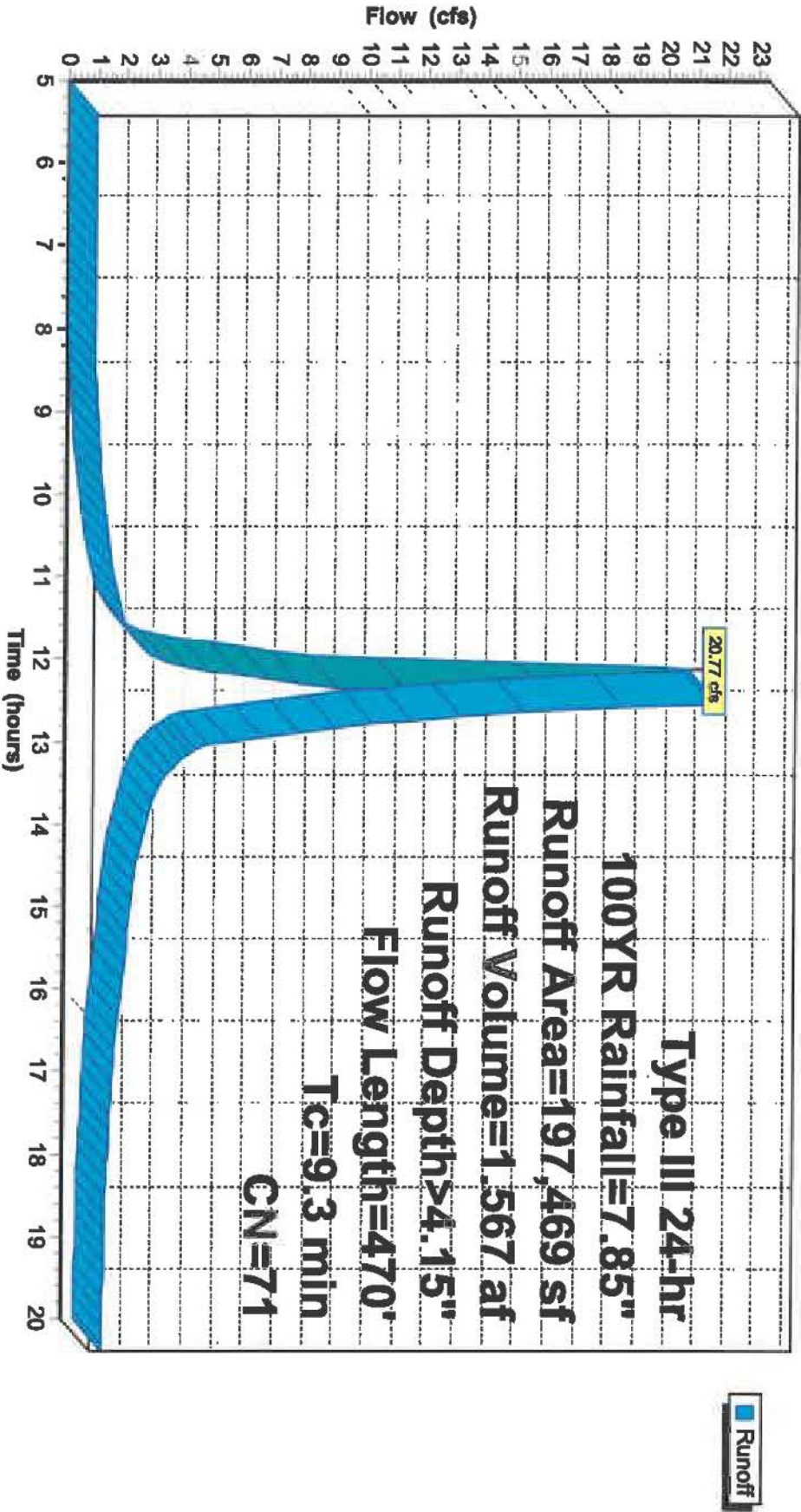
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)	CN	Description
5,057	98	Paved parking, HSG C
30,786	74	>75% Grass cover, Good, HSG C
161,626	70	Woods, Good, HSG C
197,469	71	Weighted Average
192,412		97.44% Pervious Area
5,057		2.56% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	40	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
2.9	225	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
2.6	125	0.0250	0.79		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Woodland Kv= 5.0 fps
9.3	470	Total			

Subcatchment E-2: DA E-2

Hydrograph



PRE-DEVELOPMENT1-8-24

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Type III 24-hr 100YR Rainfall=7.85"

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Summary for Subcatchment E-3: DA E-3

Runoff = 2.46 cfs @ 12.09 hrs, Volume= 0.166 af, Depth> 4.04"
Routed to Reach IP-3E : EX. CULVERT

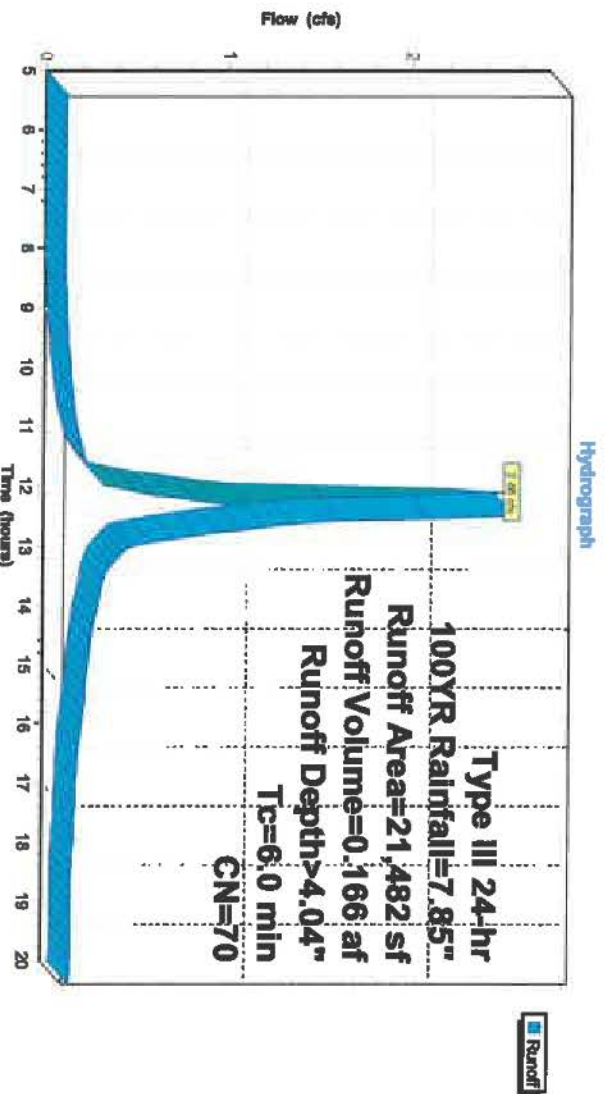
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)		CN	Description			
21,482	70	Woods, Good, HSG C				
21,482		100.00% Pervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	

6.0

Direct Entry, TRAVEL TIME

Subcatchment E-3: DA E-3

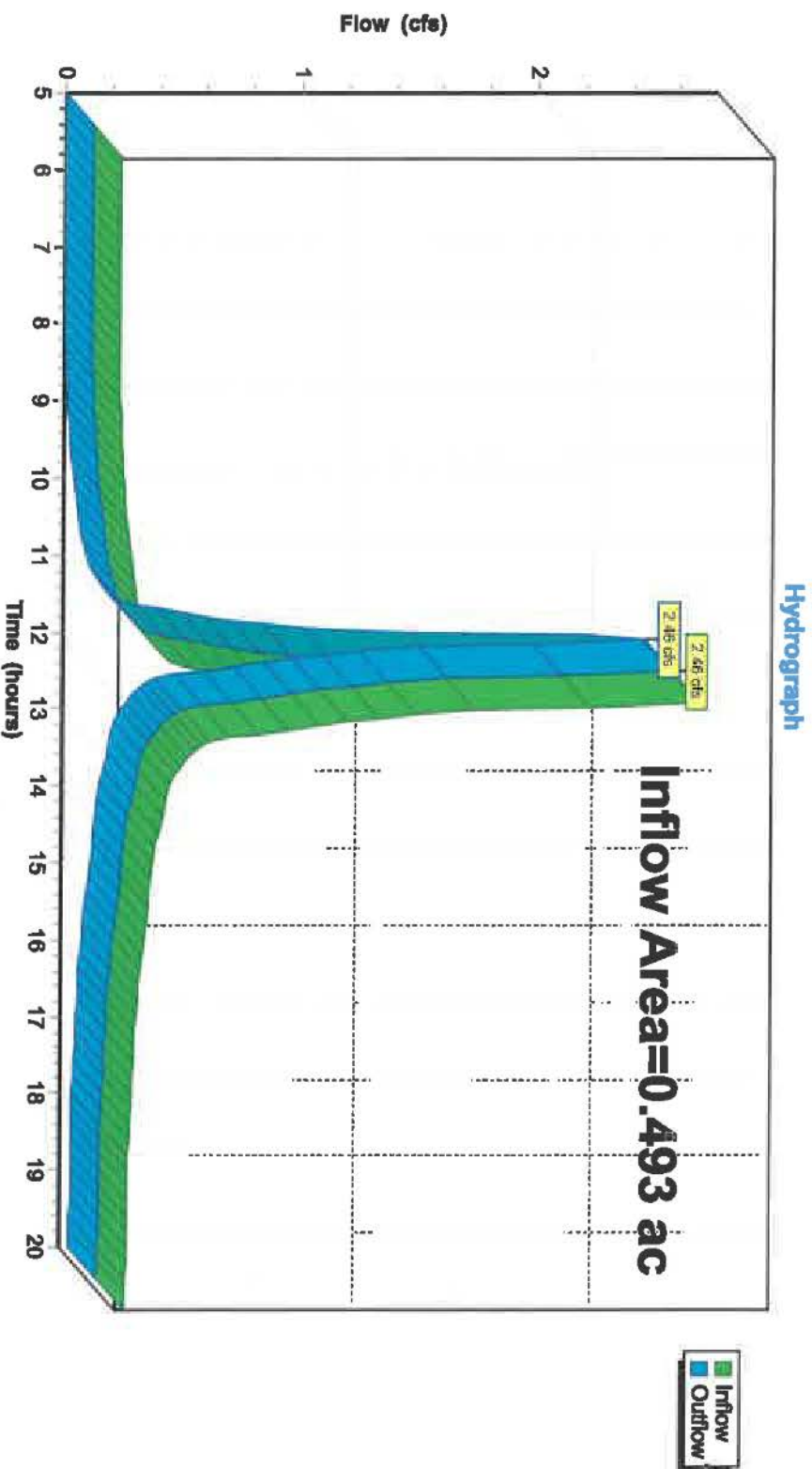


Summary for Reach IP-3E: EX. CULVERT

Inflow Area = 0.493 ac, 0.00% Impervious, Inflow Depth > 4.04" for 100YR event
Inflow = 2.46 cfs @ 12.09 hrs, Volume= 0.166 af
Outflow = 2.46 cfs @ 12.09 hrs, Volume= 0.166 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

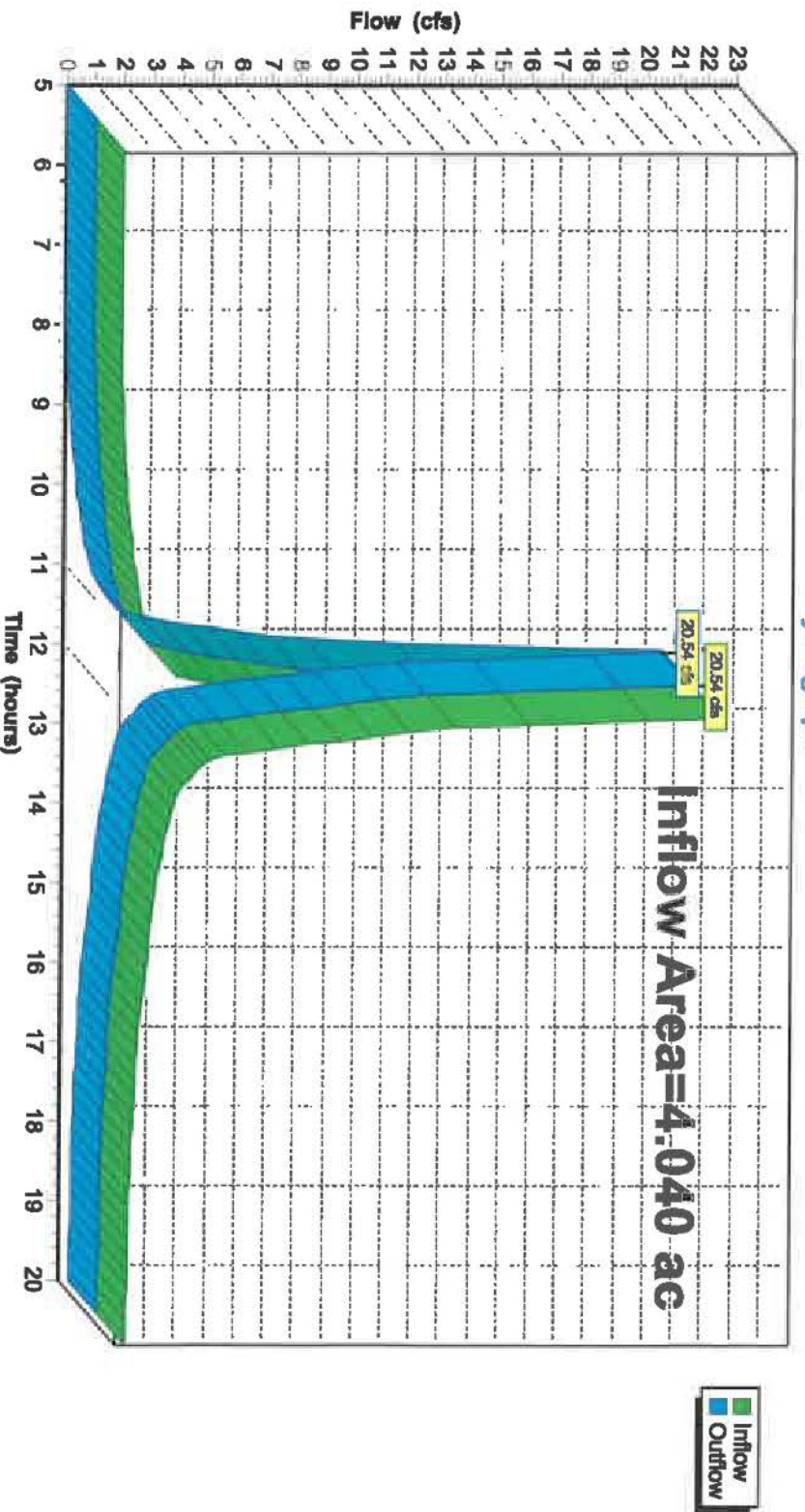
Reach IP-3E: EX. CULVERT



Summary for Reach IP-E1: 45 MAIN ST

Inflow Area = 4.040 ac, 4.02% Impervious, Inflow Depth > 4.26" for 100YR event
Inflow = 20.54 cfs @ 12.10 hrs, Volume= 1.435 af
Outflow = 20.54 cfs @ 12.10 hrs, Volume= 1.435 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E1: 45 MAIN ST
Hydrograph

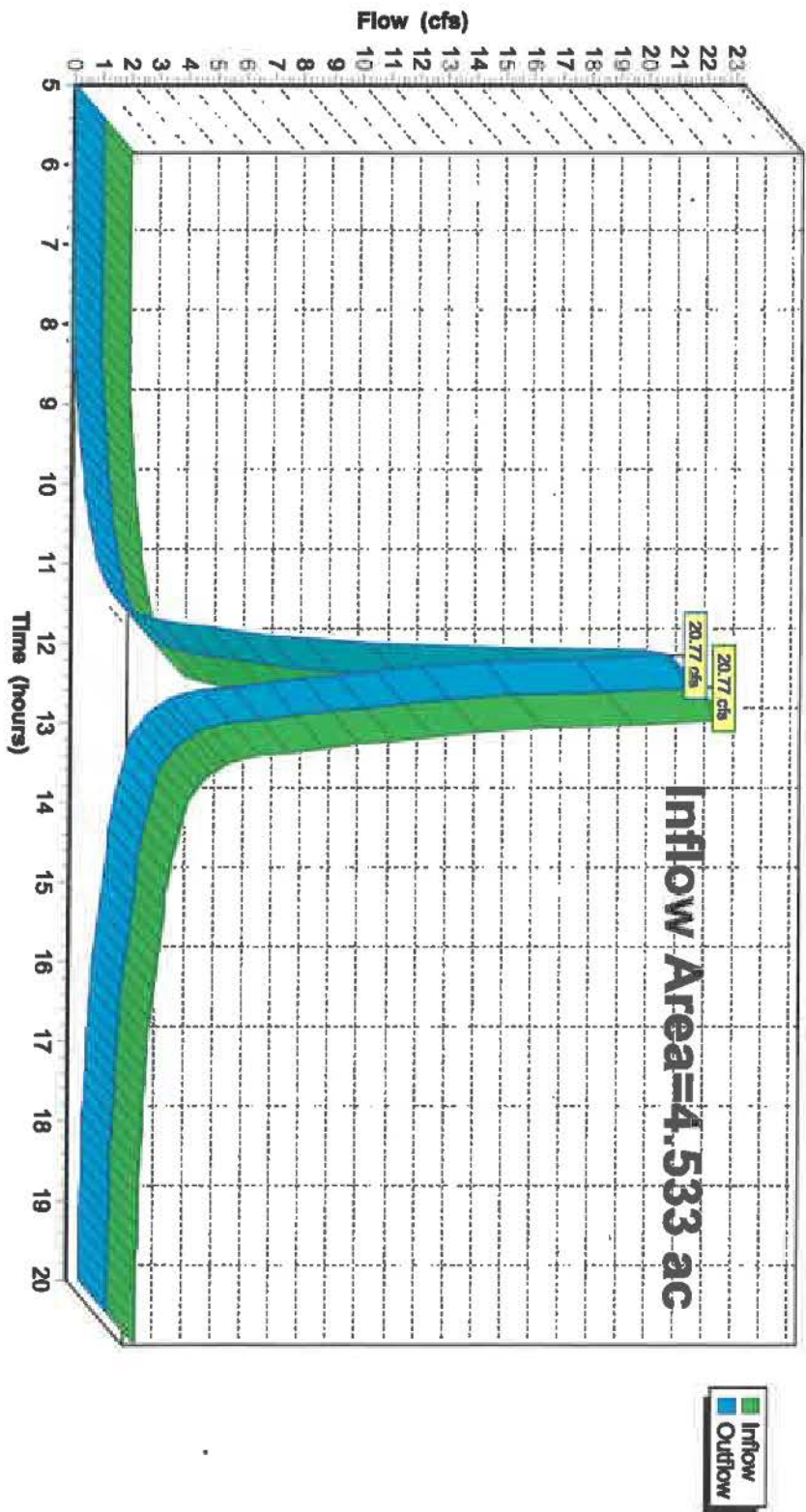


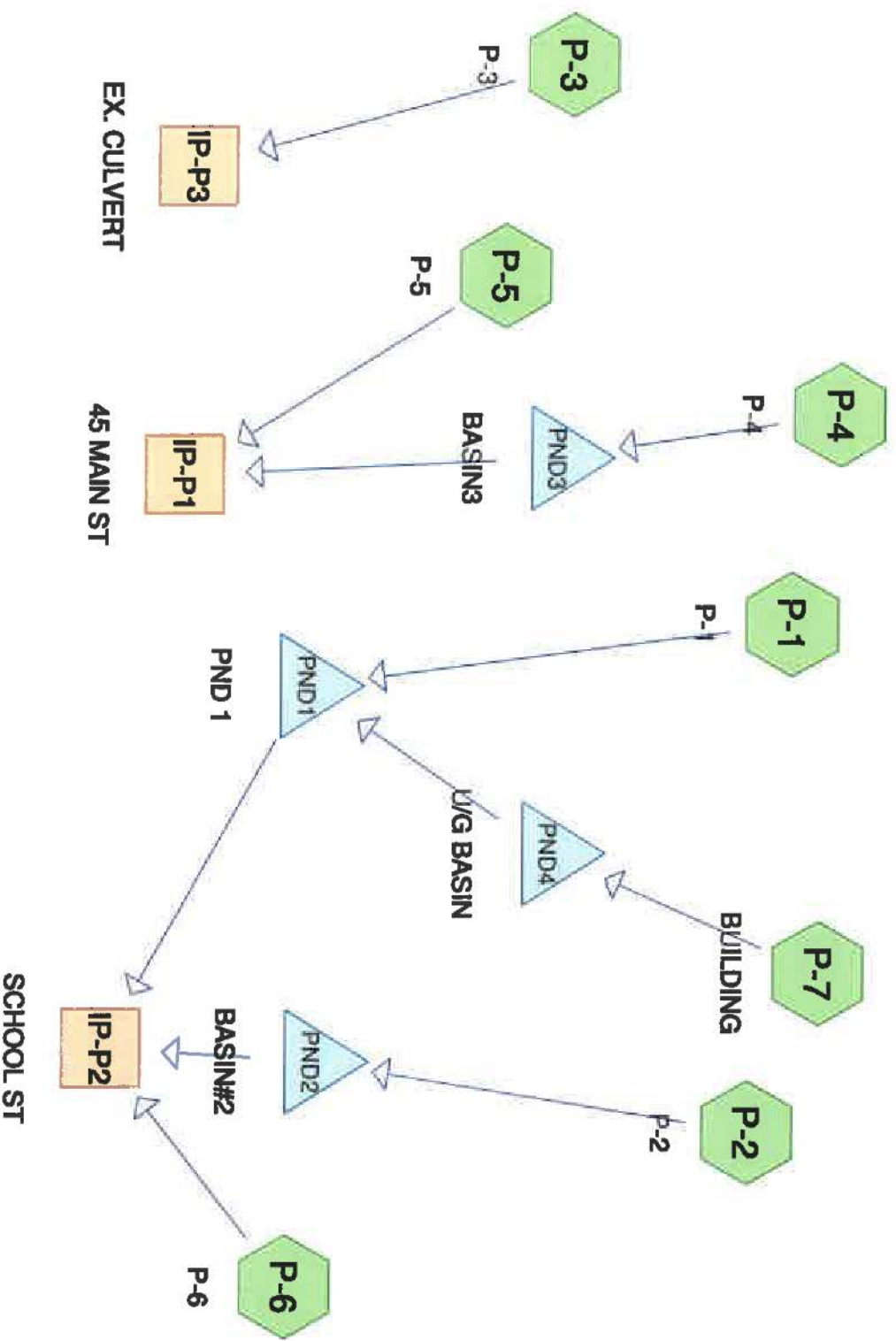
Summary for Reach IP-E2: SCHOOL ST

Inflow Area = 4.533 ac, 2.56% Impervious, Inflow Depth > 4.15" for 100YR event
Inflow = 20.77 cfs @ 12.13 hrs, Volume= 1.567 af
Outflow = 20.77 cfs @ 12.13 hrs, Volume= 1.567 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-E2: SCHOOL ST

Hydrograph





Routing Diagram for POST DEVELOPMENT1-8-24

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POST DEVELOPMENT1-8-24

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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2YR	Type III 24-hr		Default	24.00	1	3.39	2
2	10YR	Type III 24-hr		Default	24.00	1	5.11	2
3	25YR	Type III 24-hr		Default	24.00	1	6.19	2
4	100YR	Type III 24-hr		Default	24.00	1	7.85	2

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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
2.297	74	>75% Grass cover, Good, HSG C (P-1, P-2, P-3, P-4, P-6)
1.581	98	Paved parking, HSG C (P-1, P-2, P-3, P-4)
0.514	98	Roofs, HSG C (P-7)
6.584	70	Woods, Good, HSG C (P-1, P-2, P-4, P-5, P-6)
10.978	76	TOTAL AREA

POST DEVELOPMENT1-8-24

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Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
10.978	HSG C	P-1, P-2, P-3, P-4, P-5, P-6, P-7
0.000	HSG D	
0.000	Other	
10.978		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	2.297	0.000	0.000	2.297	>75% Grass cover, Good	P-1, P-2, P-3, P-4, P-6
0.000	0.000	1.581	0.000	0.000	1.581	Paved parking	P-1, P-2, P-3, P-4
0.000	0.000	0.514	0.000	0.000	0.514	Roofs	P-7
0.000	0.000	6.584	0.000	0.000	6.584	Woods, Good	P-1, P-2, P-4, P-5, P-6
0.000	0.000	10.978	0.000	0.000	10.978	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	P-1	0.00	0.00	288.0	0.0250	0.010	0.0	18.0	0.0	P-1
2	P-4	0.00	0.00	250.0	0.0250	0.010	0.0	15.0	0.0	P-4

POST DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+ Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: P-1

Runoff Area=81,268 sf 39.90% Impervious Runoff Depth>1.57"
Flow Length=618' Tc=7.1 min CN=82 Runoff=3.51 cfs 0.244 af

Subcatchment P-2: P-2

Runoff Area=82,391 sf 33.69% Impervious Runoff Depth>1.50"
Flow Length=330' Tc=7.1 min CN=81 Runoff=3.40 cfs 0.237 af

Subcatchment P-3: P-3

Runoff Area=8,272 sf 54.38% Impervious Runoff Depth>1.95"
Tc=6.0 min CN=87 Runoff=0.45 cfs 0.031 af

Subcatchment P-4: P-4

Runoff Area=56,906 sf 7.39% Impervious Runoff Depth>1.06"
Flow Length=505' Tc=5.9 min CN=74 Runoff=1.69 cfs 0.116 af

Subcatchment P-5: P-5

Runoff Area=145,259 sf 0.00% Impervious Runoff Depth>0.85"
Tc=10.0 min CN=70 Runoff=2.89 cfs 0.236 af

Subcatchment P-6: P-6

Flow Length=429' Runoff Area=81,691 sf 0.00% Impervious Runoff Depth>0.90"
Slope=0.0450 1' Tc=8.1 min CN=71 Runoff=1.83 cfs 0.141 af

Subcatchment P-7: BUILDING

Runoff Area=22,400 sf 100.00% Impervious Runoff Depth>2.95"
Tc=6.0 min CN=98 Runoff=1.65 cfs 0.126 af

Reach IP-P1: 45 MAIN ST

Inflow=2.89 cfs 0.250 af
Outflow=2.89 cfs 0.250 af

Reach IP-P2: SCHOOL ST

Inflow=1.83 cfs 0.280 af
Outflow=1.83 cfs 0.280 af

Reach IP-P3: EX. CULVERT

Inflow=0.45 cfs 0.031 af
Outflow=0.45 cfs 0.031 af

Pond PND1: PND 1

Peak Elev=325.06' Storage=5,834 cf Inflow=3.51 cfs 0.244 af
Discarded=0.07 cfs 0.048 af Primary=0.26 cfs 0.092 af Outflow=0.33 cfs 0.140 af

Pond PND2: BASIN#2

Peak Elev=325.69' Storage=6,221 cf Inflow=3.40 cfs 0.237 af
 Discarded=0.10 cfs 0.073 af Primary=0.08 cfs 0.047 af Outflow=0.18 cfs 0.120 af

Pond PND3: BASIN3

Peak Elev=332.34' Storage=2,622 cf Inflow=1.69 cfs 0.116 af
 Discarded=0.08 cfs 0.052 af Primary=0.08 cfs 0.014 af Outflow=0.15 cfs 0.066 af

Pond PND4: U/G BASIN

Peak Elev=325.81' Storage=2,739 cf Inflow=1.65 cfs 0.126 af
 Discarded=0.09 cfs 0.065 af Primary=0.00 cfs 0.000 af Outflow=0.09 cfs 0.085 af

Total Runoff Area = 10.978 ac Runoff Volume = 1.131 af Average Runoff Depth = 1.24"
80.91% Pervious = 8.882 ac 19.09% Impervious = 2.096 ac

POST DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Subcatchment P-1: P-1

Runoff = 3.51 cfs @ 12.11 hrs, Volume= 0.244 af, Depth> 1.57"
Routed to Pond PND1 : PND 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)	CN	Description
32,428	98	Paved parking, HSG C
26,882	74	>75% Grass cover, Good, HSG C
21,958	70	Woods, Good, HSG C
81,268	82	Weighted Average
48,840		60.10% Pervious Area
32,428		39.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.6	100	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.6	100	0.0200	2.87		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	288	0.0250	12.22	21.59	Pipe Channel, TRAVEL PATH E TO F
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.010
7.1	618	Total			

POST DEVELOPMENT1-8-24

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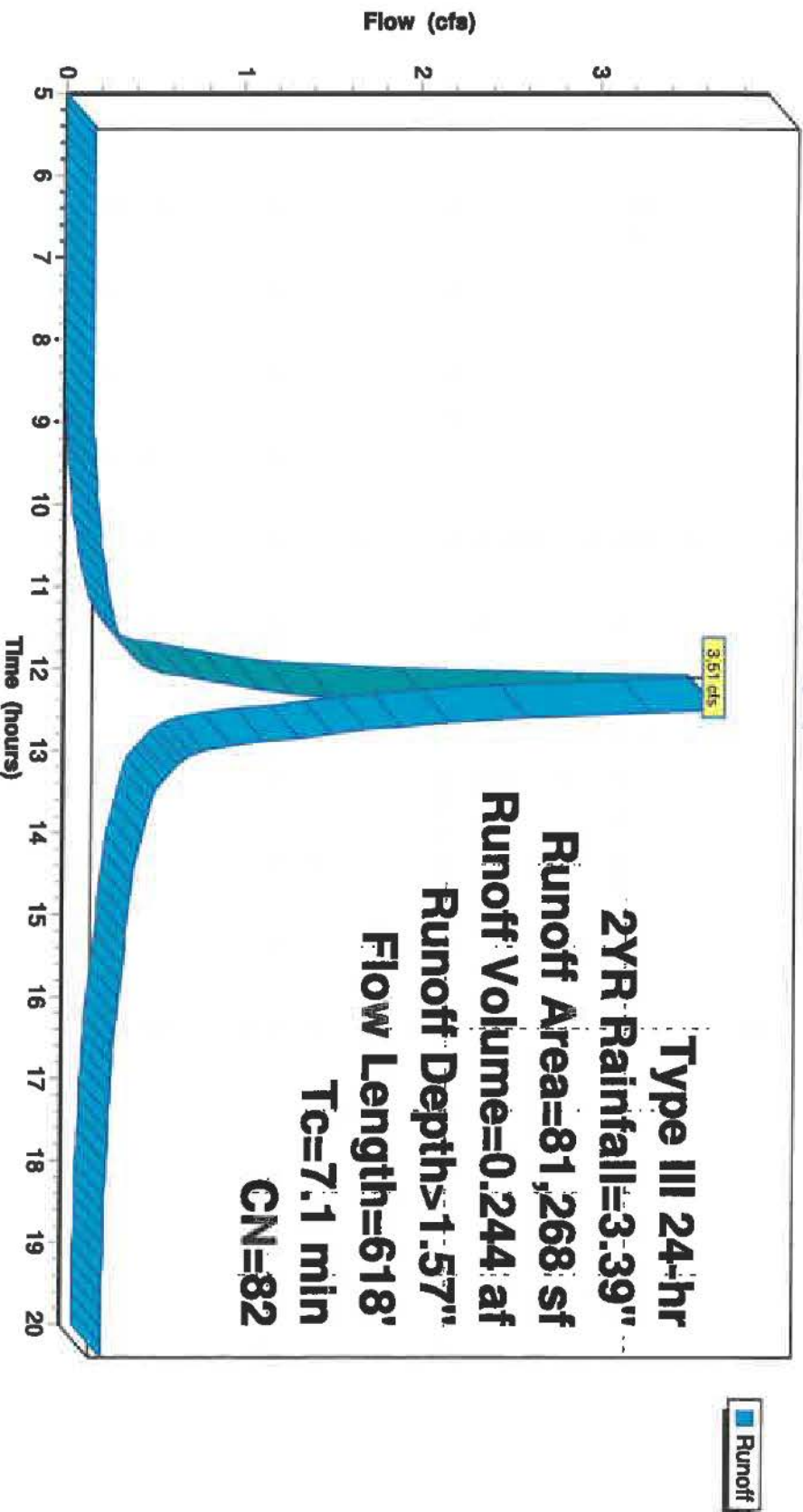
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment P-1: P-1

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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Subcatchment P-2: P-2

Runoff = 3.40 cfs @ 12.11 hrs, Volume= 0.237 af, Depth> 1.50"
Routed to Pond PND2 : BASIN#2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)	CN	Description			
27,755	98	Paved parking, HSG C			
34,256	74	>75% Grass cover, Good, HSG C			
20,380	70	Woods, Good, HSG C			
82,391	81	Weighted Average			
54,636		66.31% Pervious Area			
27,755		33.69% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0450	0.19		Sheet Flow, TRAVEL PATH A TO B
					Cultivated: Residue>20% n= 0.170 P2= 3.20"
0.7	135	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.9	145	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
7.1	330	Total			

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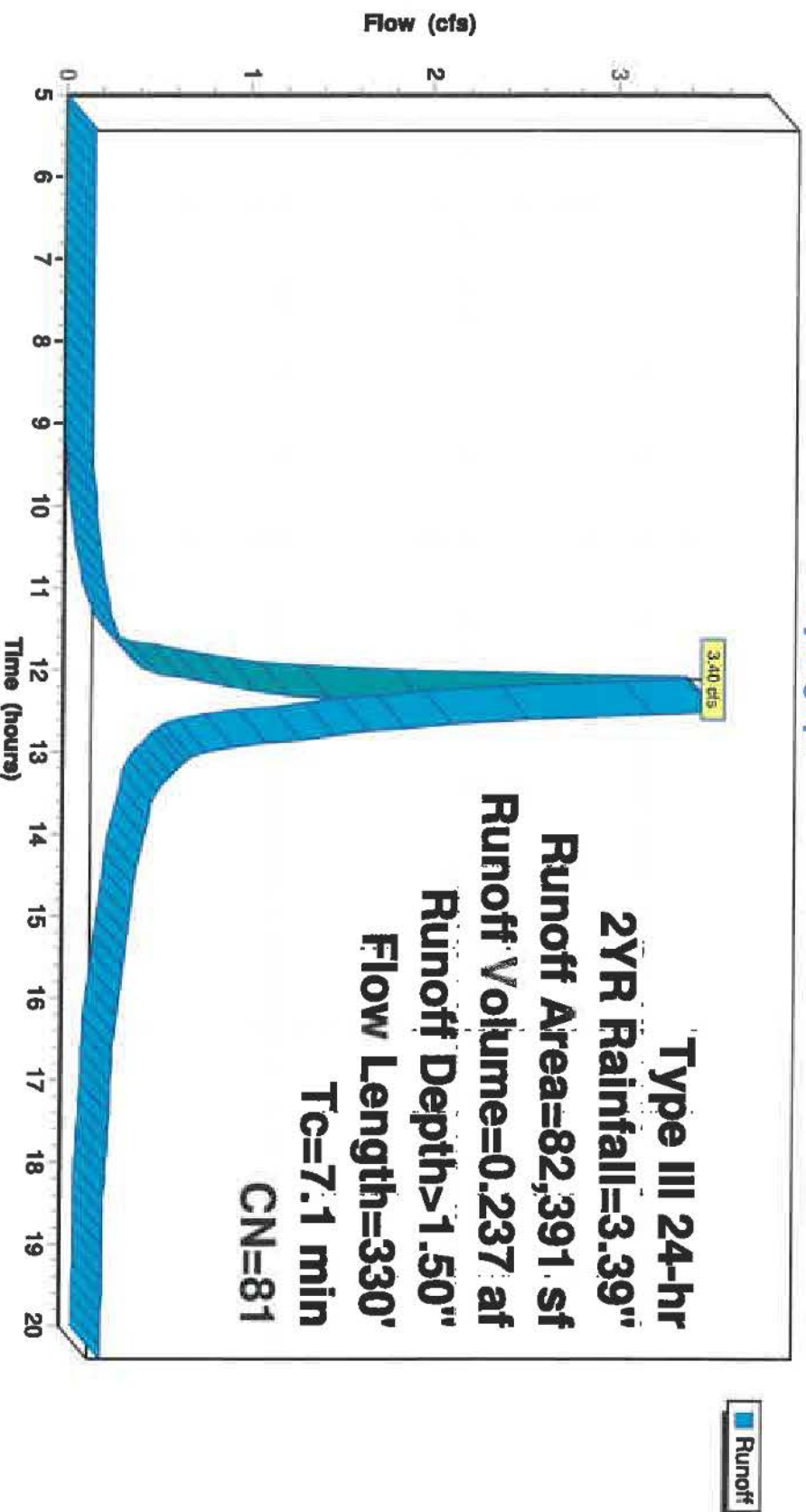
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment P-2: P-2

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"
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Summary for Subcatchment P-3: P-3

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 0.031 af, Depth> 1.95"
Routed to Reach IP-P3 : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)					CN	Description
4,498					98	Paved parking, HSG C
3,774					74	>75% Grass cover, Good, HSG C
8,272					87	Weighted Average
3,774						45.62% Pervious Area
4,498						54.38% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0					Direct Entry, TRAVEL PATH	

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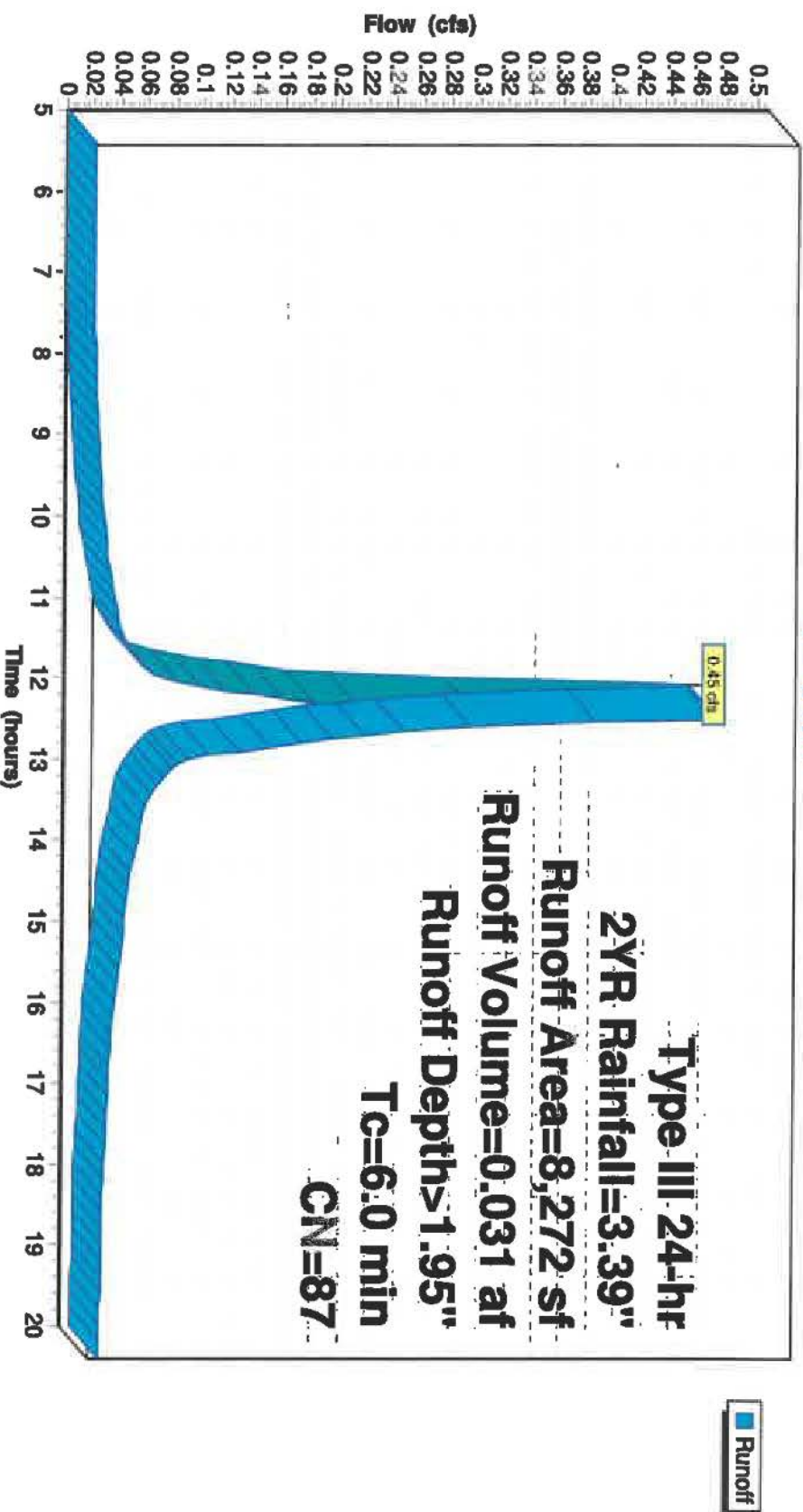
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment P-3: P-3

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"
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Summary for Subcatchment P-4: P-4

Runoff = 1.69 cfs @ 12.10 hrs, Volume= 0.116 af, Depth> 1.06"
Routed to Pond PND3 : BASIN3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)	CN	Description
4,207	98	Paved parking, HSG C
22,144	74	>75% Grass cover, Good, HSG C
30,555	70	Woods, Good, HSG C
56,906	74	Weighted Average
52,699		92.61% Pervious Area
4,207		7.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0350	0.19		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
0.5	100	0.4500	3.35		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.1	25	0.0250	3.21		Shallow Concentrated Flow, TARVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	250	0.0250	10.82	13.28	Pipe Channel, TRAVEL PATH F TO E
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.010
5.9	505	Total			

POST DEVELOPMENT1-8-24

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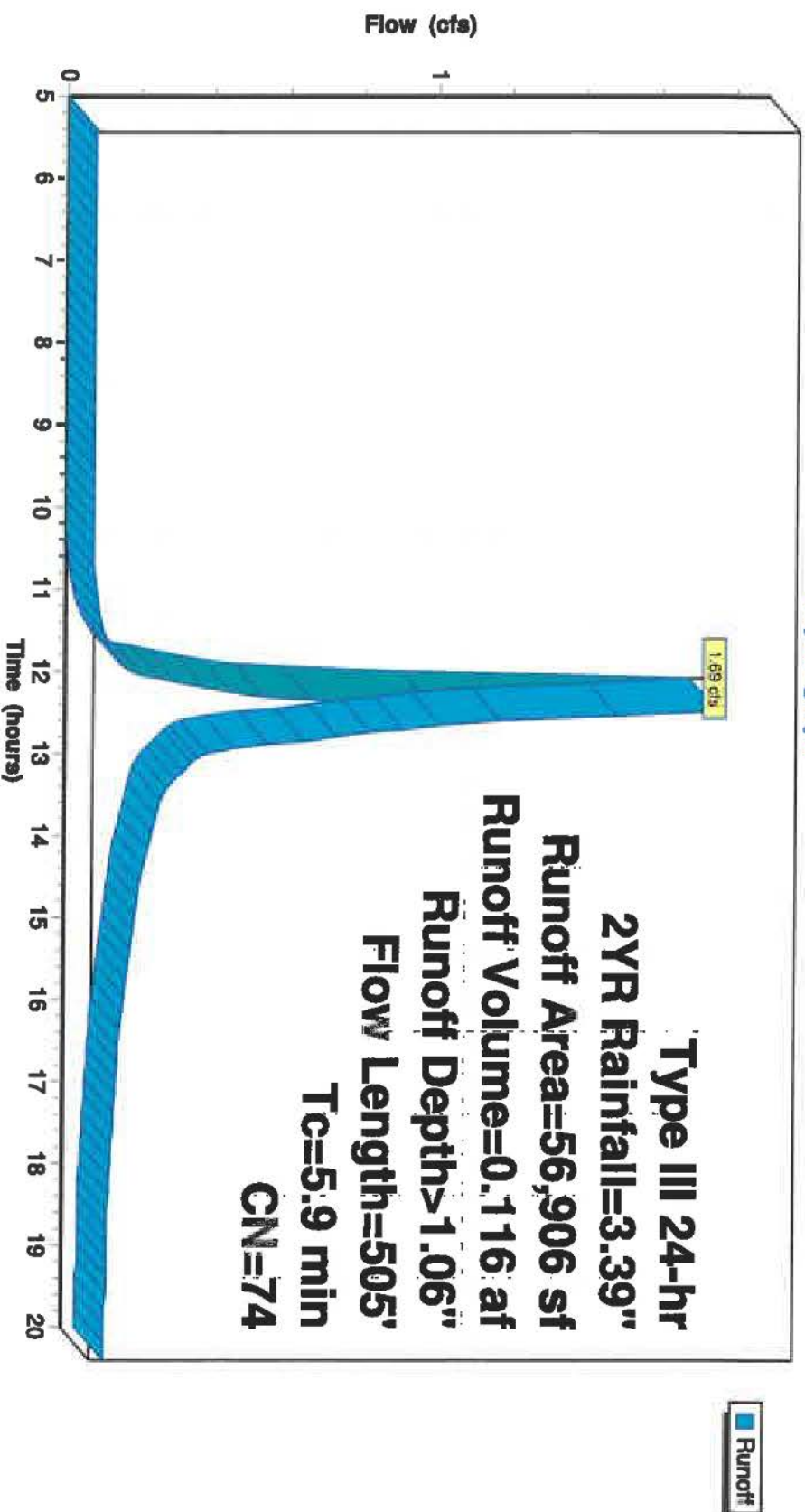
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment P-4: P-4

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Type III 24-hr 2YR Rainfall=3.39"

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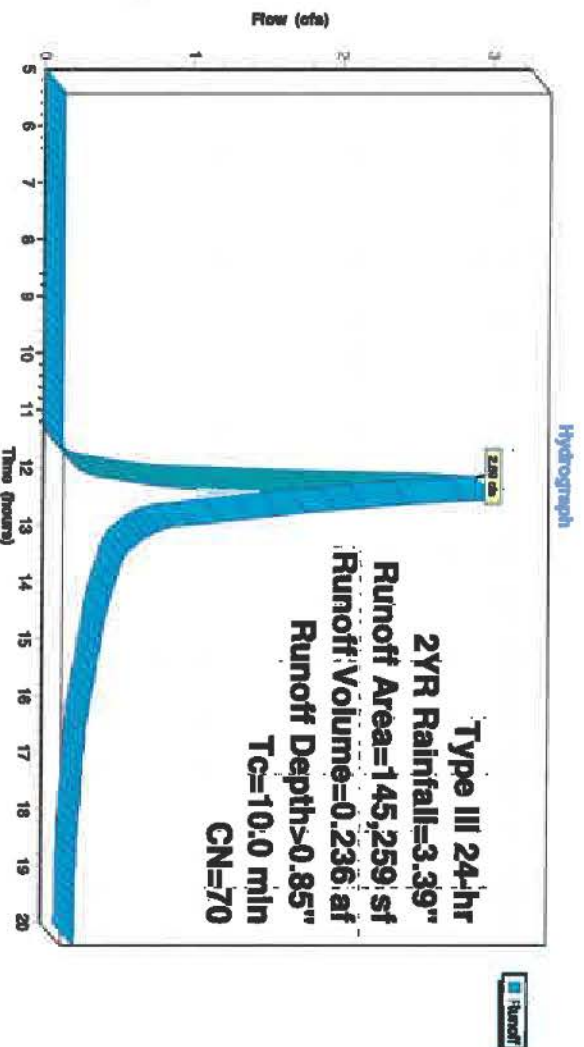
Summary for Subcatchment P-5: P-5

Runoff = 2.89 cfs @ 12.16 hrs, Volume= 0.236 af, Depth> 0.85"
Routed to Reach IP-P1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)		CN	Description		
145,259	70	Woods, Good, HSG C			
145,259		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Subcatchment P-5: P-5



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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Subcatchment P-6: P-6

Runoff = 1.83 cfs @ 12.13 hrs, Volume= 0.141 af, Depth> 0.90"
Routed to Reach IP-P2 : SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)		CN	Description		
13,023	74	>75%	Grass cover, Good, HSG C		
68,668	70	Woods, Good, HSG C			
81,691	71	Weighted Average			
81,691		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n=0.150 P2=3.20"
0.9	184	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
3.1	195	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
8.1	429	Total			

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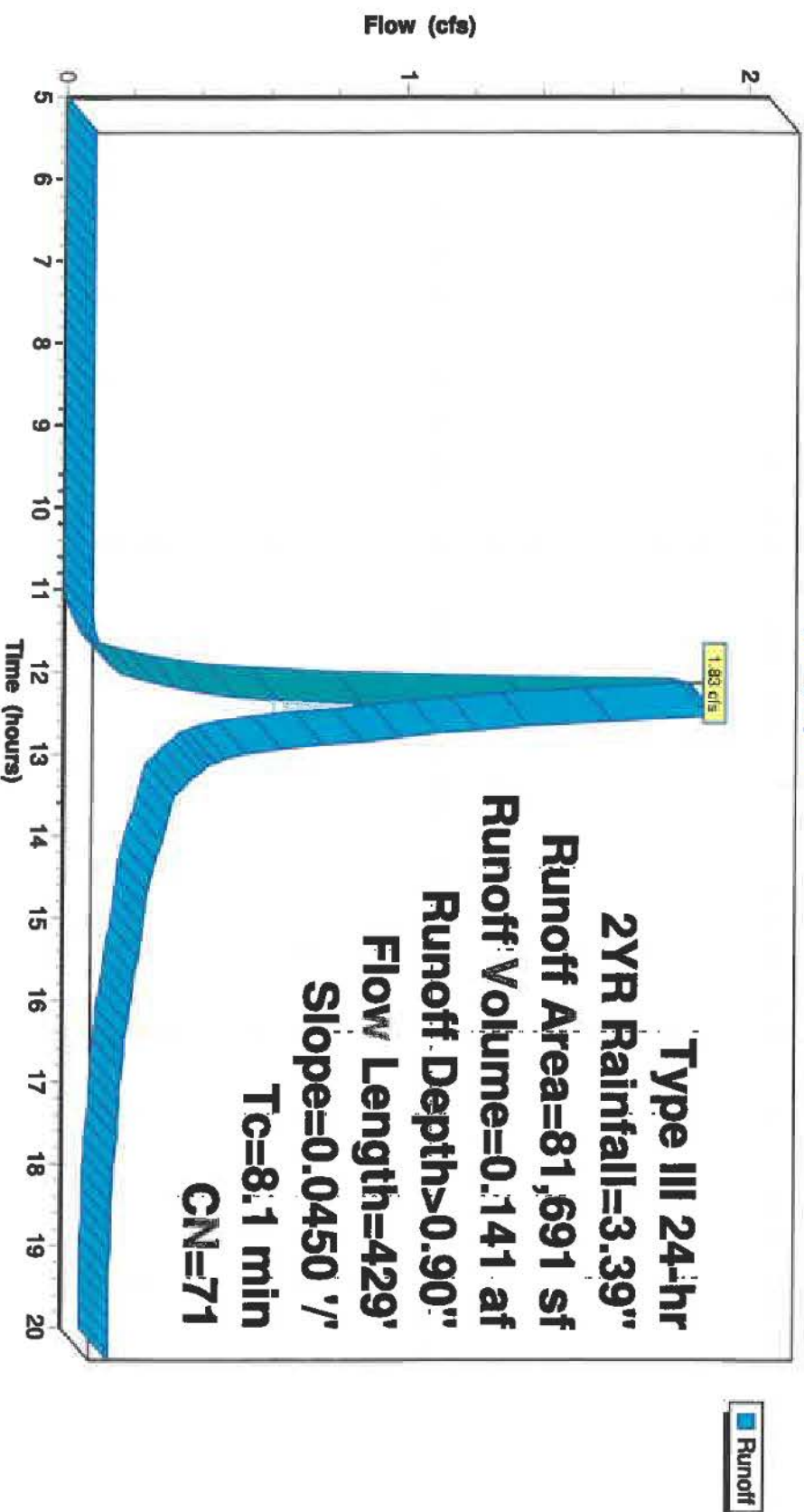
Type III 24-hr 2YR Rainfall=3.39"

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Subcatchment P-6: P-6

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Type III 24-hr 2YR Rainfall=3.39"

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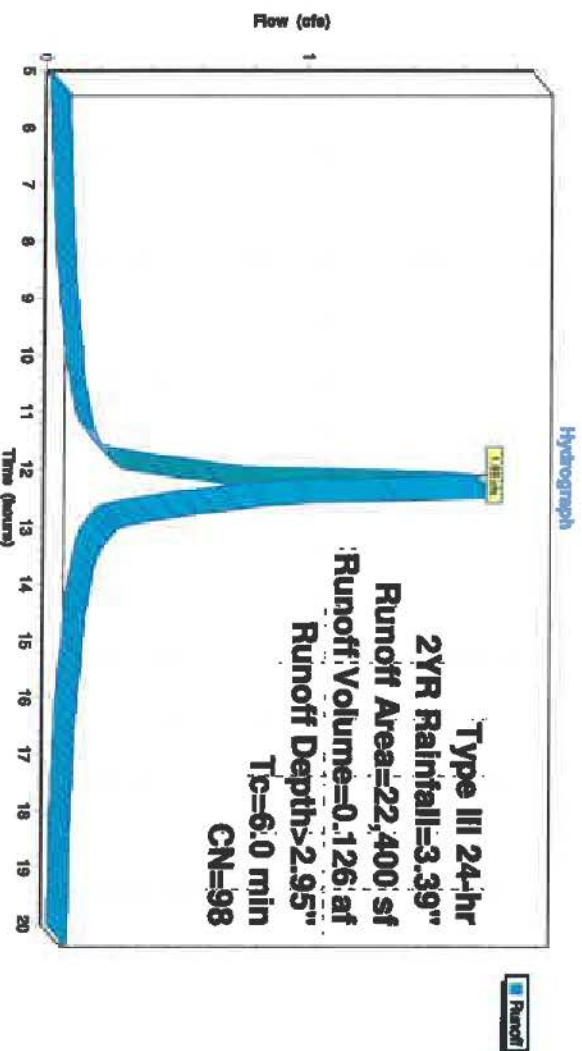
Summary for Subcatchment P-7: BUILDING

Runoff = 1.65 cfs @ 12.09 hrs, Volume= 0.126 af, Depth> 2.95"
Routed to Pond PND4 : U/G BASIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 2YR Rainfall=3.39"

Area (sf)		CN	Description		
22,400		98	Roofs, HSG C		
22,400			100.00% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description

Subcatchment P-7: BUILDING



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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Reach IP-P1: 45 MAIN ST

Inflow Area = 4.641 ac, 2.08% Impervious, Inflow Depth > 0.65" for 2YR event

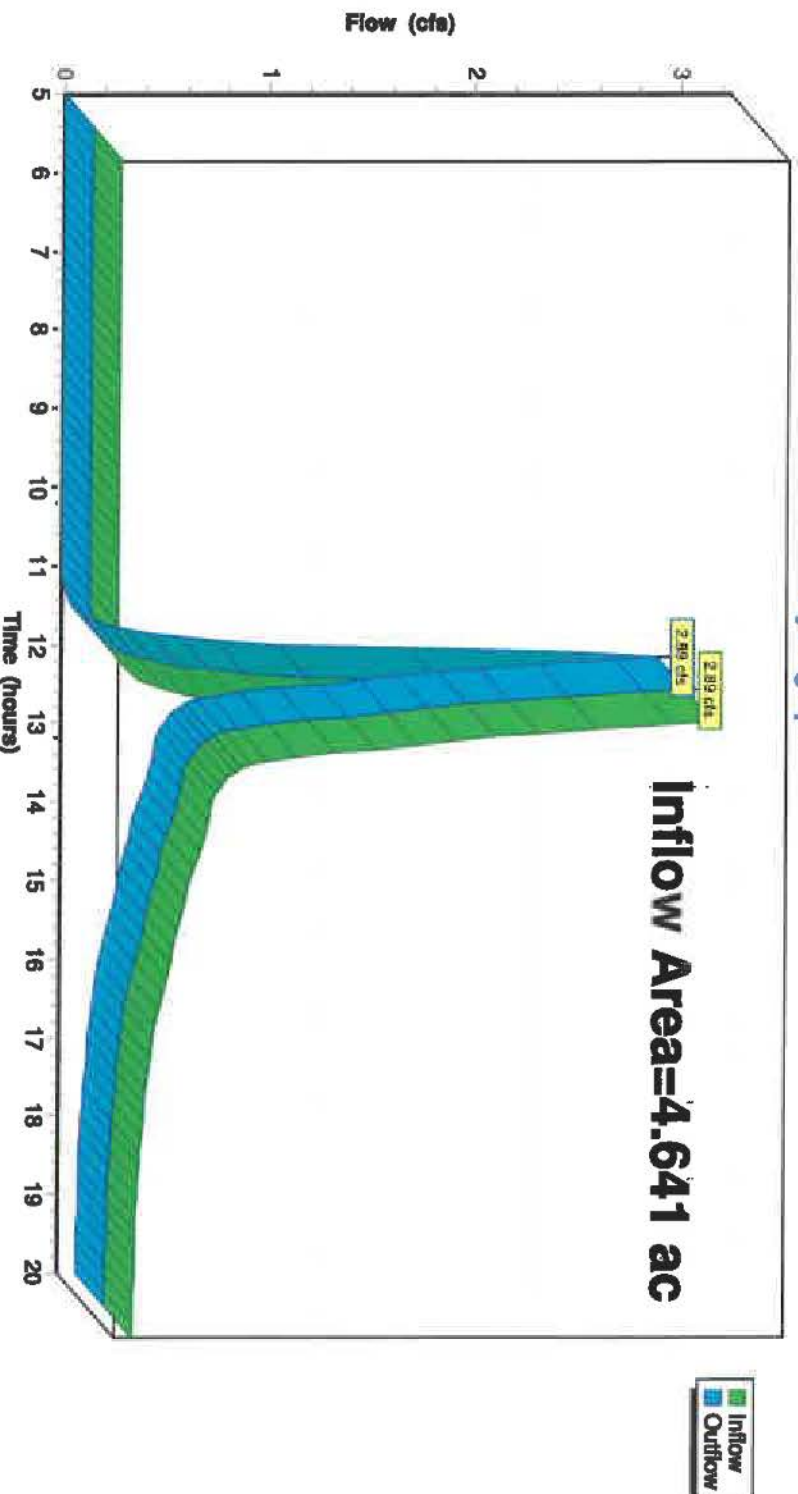
Inflow = 2.89 cfs @ 12.16 hrs, Volume= 0.250 af

Outflow = 2.89 cfs @ 12.16 hrs, Volume= 0.250 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P1: 45 MAIN ST

Hydrograph



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Type III 24-hr 2YR Rainfall=3.39"

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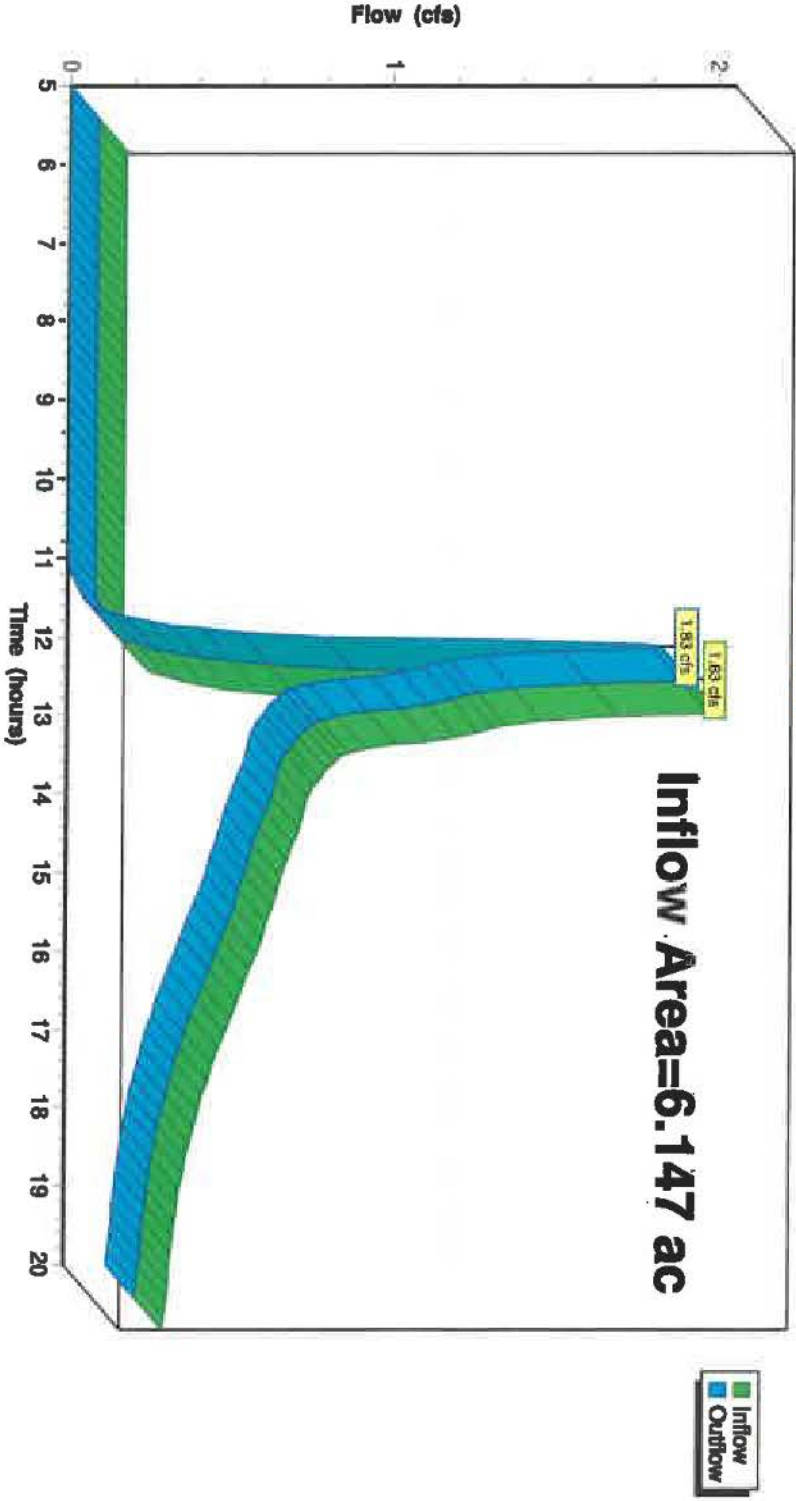
Summary for Reach IP-P2: SCHOOL ST

Inflow Area = 6.147 ac, 30.84% Impervious, Inflow Depth > 0.55" for 2YR event
Inflow = 1.83 cfs @ 12.13 hrs, Volume= 0.280 af
Outflow = 1.83 cfs @ 12.13 hrs, Volume= 0.280 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P2: SCHOOL ST

Hydrograph



POST DEVELOPMENT1-8-24

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Type III 24-hr 2YR Rainfall=3.39"

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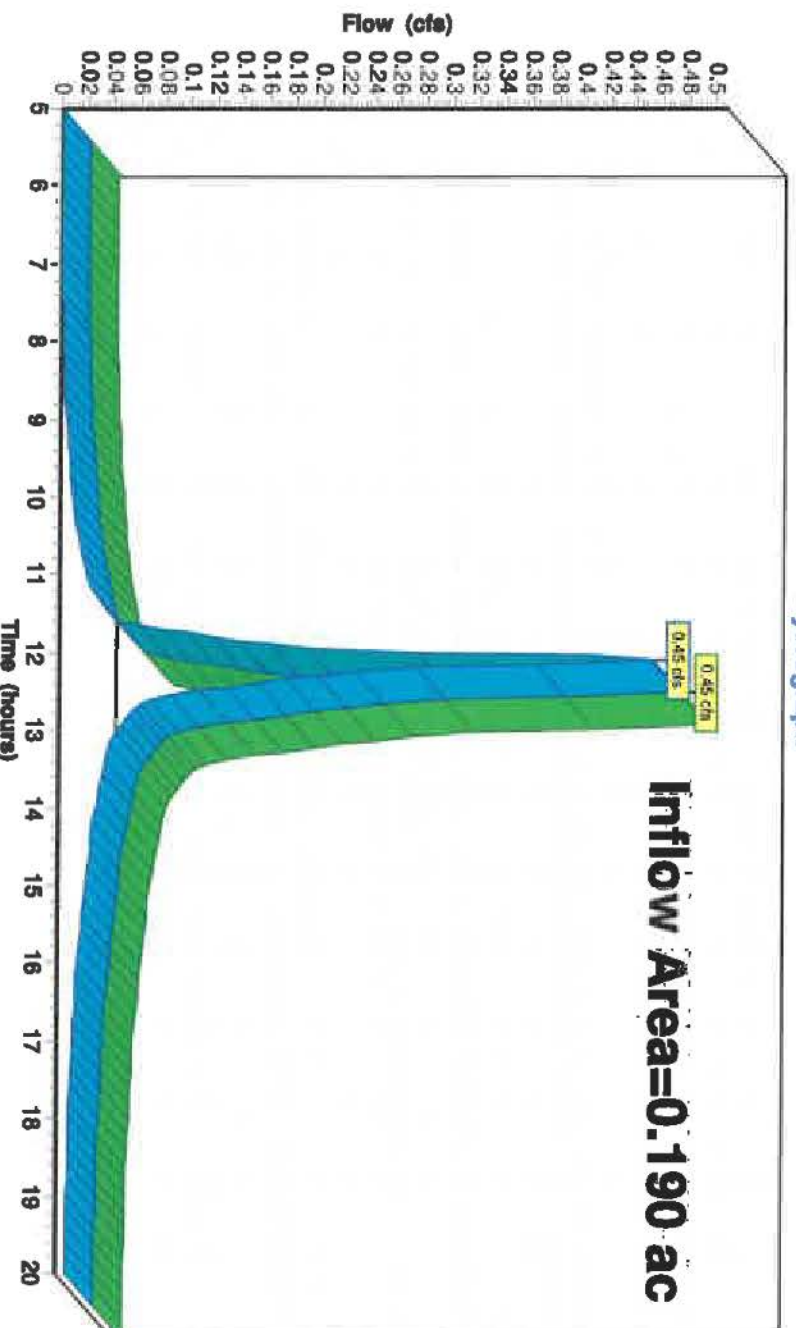
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Summary for Reach IP-P3: EX. CULVERT

Inflow Area = 0.190 ac, 54.38% Impervious, Inflow Depth > 1.95" for 2YR event
Inflow = 0.45 cfs @ 12.09 hrs, Volume= 0.031 af
Outflow = 0.45 cfs @ 12.09 hrs, Volume= 0.031 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P3: EX. CULVERT

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Summary for Pond PND1: PND 1

Inflow Area = 2.380 ac, 52.89% Impervious, Inflow Depth > 1.23" for 2YR event
 Inflow = 3.51 cfs @ 12.11 hrs, Volume= 0.244 af
 Outflow = 0.33 cfs @ 13.22 hrs, Volume= 0.140 af, Atten= 91%, Lag= 67.0 min
 Discarded = 0.07 cfs @ 13.22 hrs, Volume= 0.048 af
 Primary = 0.26 cfs @ 13.22 hrs, Volume= 0.092 af
 Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 325.06' @ 13.22 hrs Surf.Area= 2,910 sf Storage= 5,834 cf

Plug-Flow detention time= 185.5 min calculated for 0.140 af (57% of inflow)
 Center-of-Mass det. time= 106.4 min (903.7 - 797.3)

Volume Invert Avail.Storage Storage Description

#1	322.00'	23,430 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
322.00	1,030	0	0
324.00	2,125	3,155	3,155
326.00	3,600	5,725	8,880
328.00	5,300	8,900	17,780
329.00	6,000	5,650	23,430

Device Routing Invert Outlet Devices

#1	Discarded	322.00'	1.020 In/hr Exfiltration over Surface area
#2	Primary	324.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	327.50'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00			
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32			

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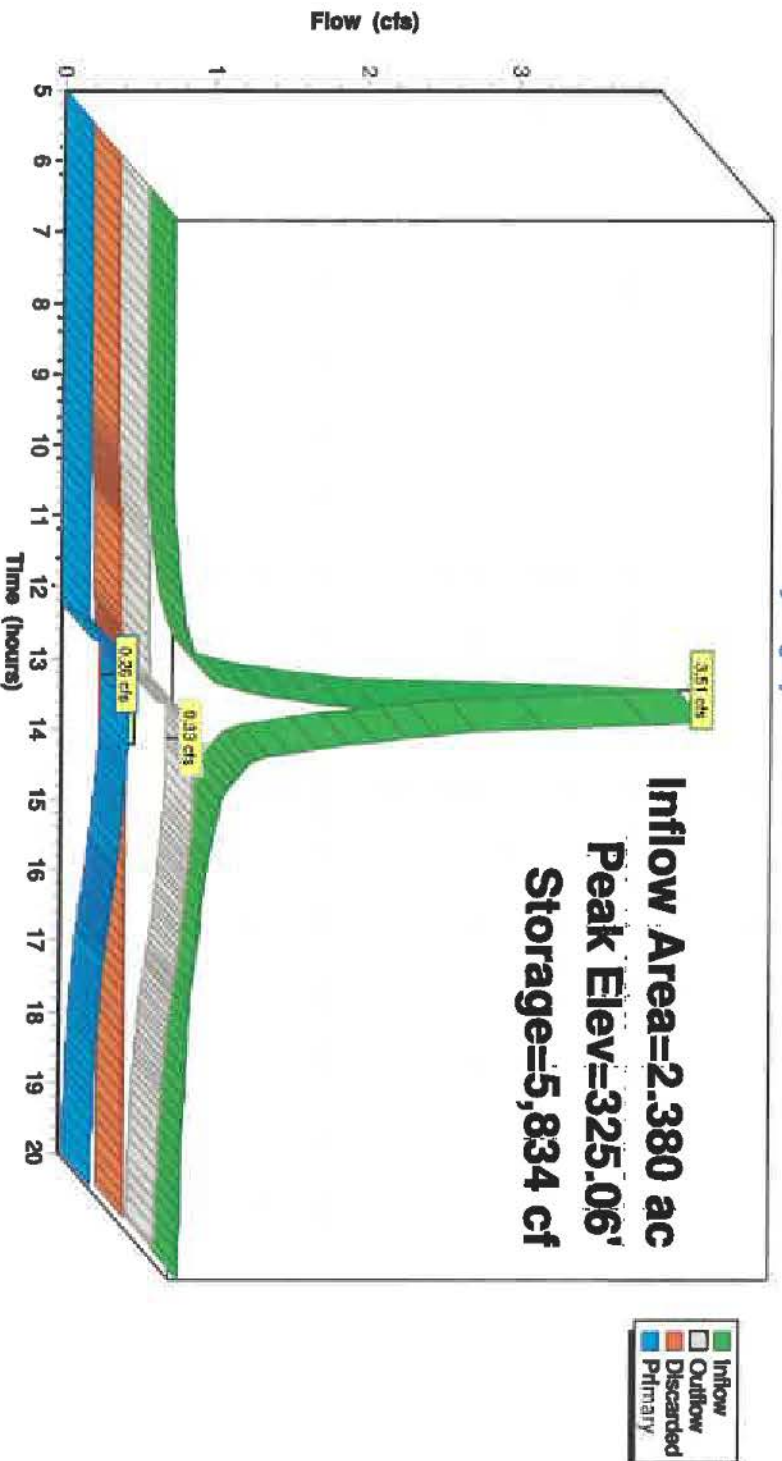
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Discarded OutFlow Max=0.07 cfs @ 13.22 hrs HW=325.06' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=0.26 cfs @ 13.22 hrs HW=325.06' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.26 cfs @ 3.04 fps)
3=Orifice/Grate (Controls 0.00 cfs)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND1: PND 1

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Type III 24-hr 2YR Rainfall=3.39"
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Summary for Pond PND2: BASIN#2

Inflow Area = 1.891 ac, 33.69% Impervious, Inflow Depth > 1.50" for 2YR event
Inflow = 3.40 cfs @ 12.11 hrs, Volume= 0.237 af
Outflow = 0.18 cfs @ 15.23 hrs, Volume= 0.120 af, Atten= 95%, Lag= 187.2 min
Discarded = 0.10 cfs @ 15.23 hrs, Volume= 0.073 af
Primary = 0.08 cfs @ 15.23 hrs, Volume= 0.047 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs,
Peak Elev= 325.69' @ 15.23 hrs Surf.Area= 4,070 sf Storage= 6,221 cf

Plug-Flow detention time= 220.5 min calculated for 0.120 af (51% of inflow)
Center-of-Mass det. time= 137.8 min (937.6 - 799.8)

Volume	Invert	Avail.Storage	Storage Description
#1	324.00'	20,173 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 21,436 cf Overall - 1,263 cf Embedded = 20,173 cf Cuttec R-330XLHD x 24 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
#2	324.00'	1,263 cf	
21,436 cf			Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
324.00	3,312	0	0
326.00	4,212	7,524	7,524
328.00	4,700	8,912	16,436
329.00	5,300	5,000	21,436

Device	Routing	Invert	Outlet Devices
#1	Discarded	324.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	325.00'	2.0" Vert. Office/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	327.30'	4.0" Vert. Office/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	328.00'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir

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Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.10 cfs @ 15.23 hrs HW=325.69' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.08 cfs @ 15.23 hrs HW=325.69' (Free Discharge)

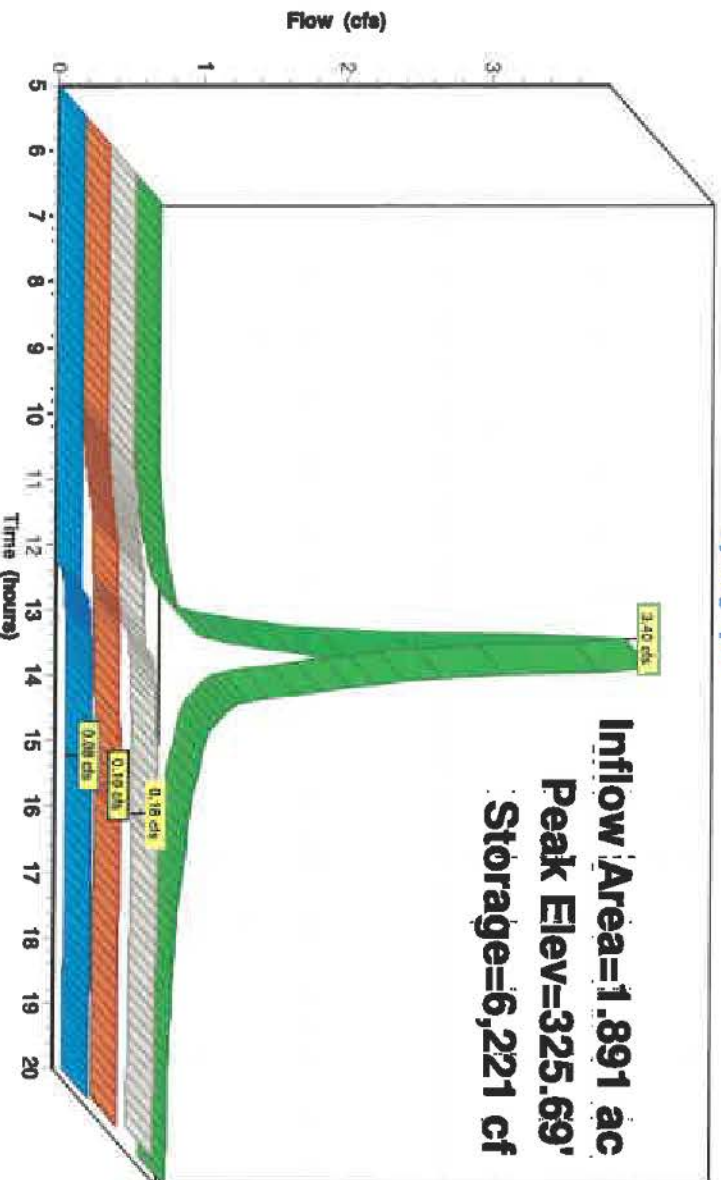
2=Orifice/Grate (Orifice Controls 0.08 cfs @ 3.74 fps)

3=Orifice/Grate (Controls 0.00 cfs)

4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND2: BASIN#2

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Type III 24-hr 2YR Rainfall=3.39"

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Summary for Pond PND3: BASIN3

Inflow Area = 1.306 ac, 7.39% Impervious, Inflow Depth > 1.06" for 2YR event
Inflow = 1.69 cfs @ 12.10 hrs, Volume= 0.116 af
Outflow = 0.15 cfs @ 13.69 hrs, Volume= 0.066 af, Atten= 91%, Lag= 95.4 min
Discarded = 0.08 cfs @ 13.69 hrs, Volume= 0.052 af
Primary = 0.08 cfs @ 13.69 hrs, Volume= 0.014 af
Routed to Reach IP-P1 : 45 MAIN ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 332.34' @ 13.69 hrs Surf.Area= 1,389 sf Storage= 2,622 cf

Plug-Flow detention time= 194.8 min calculated for 0.066 af (57% of inflow)
Center-of-Mass det. time= 110.7 min (926.3 - 815.6)

Volume Invert Avail.Storage Storage Description

#1 330.00' 7,937 cf Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
330.00	925	0	0
332.00	1,250	2,175	2,175
334.00	2,070	3,320	5,495
335.00	2,814	2,442	7,937

Device Routing Invert Outlet Devices

#1	Discarded	330.00'	2,400 In/hr Exfiltration over Surface area
#2	Primary	332.20'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	334.00'	20.0" long x 0.5' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00			
Coef. (English) 2.80 2.92 3.08 3.30 3.32			

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Type III 24-hr 2YR Rainfall=3.39"

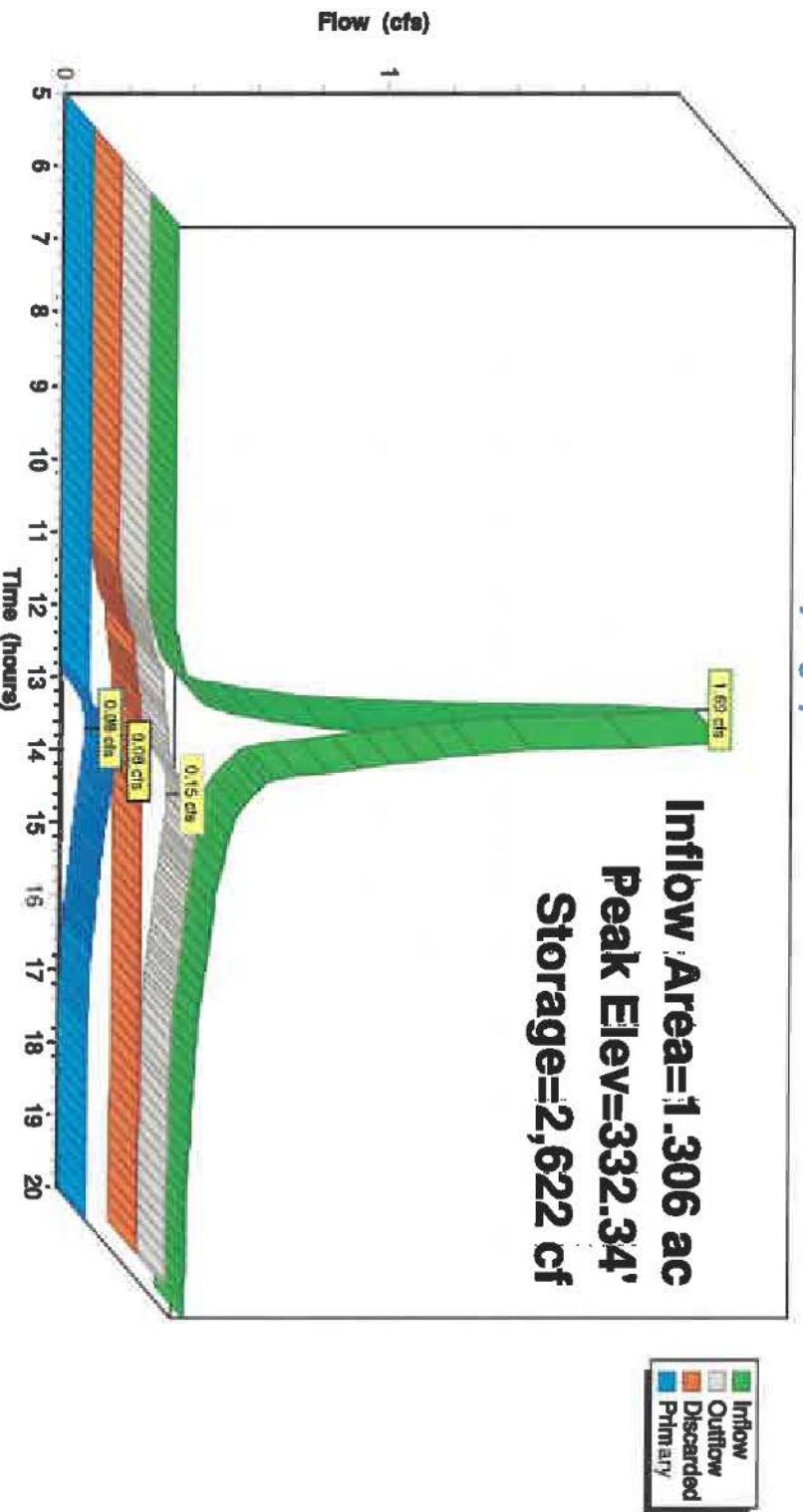
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Discarded OutFlow Max=0.08 cfs @ 13.69 hrs HW=332.34' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.08 cfs @ 13.69 hrs HW=332.34' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.08 cfs @ 1.27 fps)
3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND3: BASIN3

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Summary for Pond PND4: U/G BASIN

Inflow Area = 0.514 ac, 100.00% Impervious, Inflow Depth > 2.95" for 2YR event
Inflow = 1.65 cfs @ 12.09 hrs, Volume= 0.126 af
Outflow = 0.09 cfs @ 10.60 hrs, Volume= 0.085 af, Atten= 95%, Lag= 0.0 min
Discarded = 0.09 cfs @ 10.60 hrs, Volume= 0.085 af
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Routed to Pond PND1 : PND 1

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 325.81' @ 14.10 hrs Surf.Area= 3,637 sf Storage= 2,739 cf

Plug-Flow detention time= 169.8 min calculated for 0.085 af (67% of inflow)
Center-of-Mass det. time= 97.9 min (835.9 - 738.0)

Volume	Invert	Avail.Storage	Storage Description
#1	325.00'	2,587 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 12,739 cf Overall - 6,270 cf Embedded = 6,468 cf x 40.0% Voids Cuttec R-330XLHD x 120 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
#2	325.00'	6,270 cf	
8,857 cf Total Available Storage			

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
325.00	3,673	0	0
325.50	3,637	1,828	1,828
328.00	3,637	9,093	10,920
328.50	3,637	1,819	12,739

Device	Routing	Invert	Outlet Devices
#1	Discarded	325.00'	1.020 In/hr Exfiltration over Surface area
#2	Primary	326.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 2YR Rainfall=3.39"

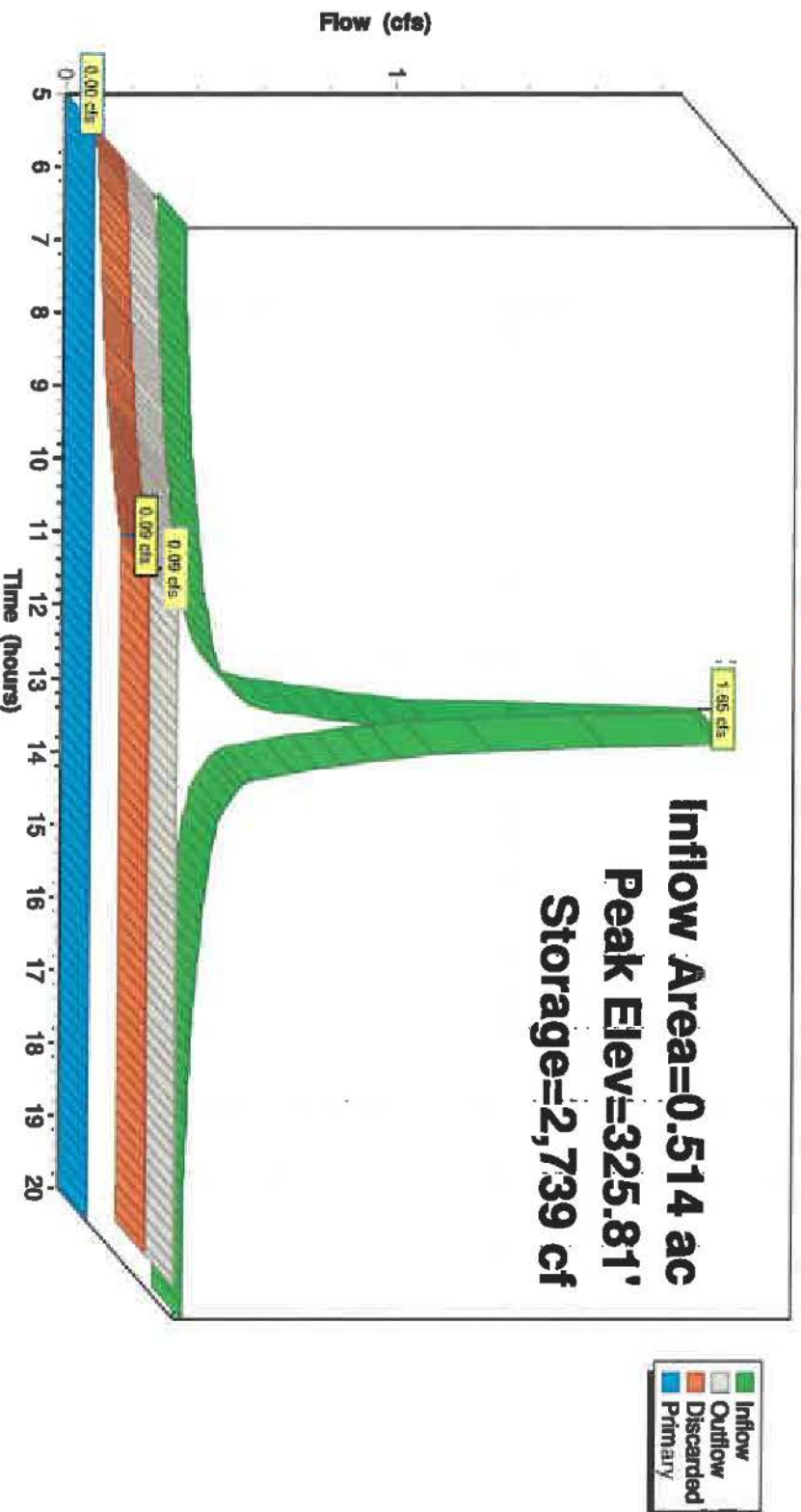
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Discarded OutFlow Max=0.09 cfs @ 10.60 hrs HW=325.04' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=325.00' (Free Discharge)
2=Orifice/Grate (Controls 0.00 cfs)

Pond PND4: U/G BASIN

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Type III 24-hr 10YR Rainfall=5.1"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: P-1

Runoff Area=81,268 sf 39.90% Impervious Runoff Depth>2.98"
Flow Length=618' Tc=7.1 min CN=82 Runoff=6.59 cfs 0.463 af

Subcatchment P-2: P-2

Runoff Area=82,391 sf 33.69% Impervious Runoff Depth>2.88"
Flow Length=330' Tc=7.1 min CN=81 Runoff=6.49 cfs 0.455 af

Subcatchment P-3: P-3

Runoff Area=8,272 sf 54.38% Impervious Runoff Depth>3.46"
Tc=6.0 min CN=87 Runoff=0.78 cfs 0.055 af

Subcatchment P-4: P-4

Runoff Area=56,906 sf 7.39% Impervious Runoff Depth>2.27"
Flow Length=505' Tc=5.9 min CN=74 Runoff=3.68 cfs 0.248 af

Subcatchment P-5: P-5

Runoff Area=145,259 sf 0.00% Impervious Runoff Depth>1.95"
Tc=10.0 min CN=70 Runoff=7.03 cfs 0.542 af

Subcatchment P-6: P-6

Flow Length=429' Runoff Area=81,691 sf 0.00% Impervious Runoff Depth>2.03"
Slope=0.0450 1' Tc=8.1 min CN=71 Runoff=4.38 cfs 0.317 af

Subcatchment P-7: BUILDING

Runoff Area=22,400 sf 100.00% Impervious Runoff Depth>4.52"
Tc=6.0 min CN=98 Runoff=2.51 cfs 0.194 af

Reach IP-P1: 45 MAIN ST

Inflow=8.08 cfs 0.671 af
Outflow=8.08 cfs 0.671 af

Reach IP-P2: SCHOOL ST

Inflow=4.76 cfs 0.696 af
Outflow=4.76 cfs 0.696 af

Reach IP-P3: EX. CULVERT

Inflow=0.78 cfs 0.055 af
Outflow=0.78 cfs 0.055 af

Pond PND1: PND 1

Peak Elev=326.58' Storage=11,097 cf Inflow=6.59 cfs 0.463 af
Discarded=0.10 cfs 0.065 af Primary=0.60 cfs 0.281 af Outflow=0.69 cfs 0.346 af

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Type III 24-hr 10YR Rainfall=5.11"

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Pond PND2: BASIN#2

Peak Elev=327.28' Storage=13,096 cf Inflow=6.49 cfs 0.455 af
Discarded=0.11 cfs 0.091 af Primary=0.16 cfs 0.098 af Outflow=0.26 cfs 0.188 af

Pond PND3: BASIN3

Peak Elev=332.97' Storage=3,587 cf Inflow=3.68 cfs 0.248 af
Discarded=0.09 cfs 0.061 af Primary=1.58 cfs 0.130 af Outflow=1.67 cfs 0.191 af

Pond PND4: U/G BASIN

Peak Elev=326.50' Storage=4,956 cf Inflow=2.51 cfs 0.194 af
Discarded=0.09 cfs 0.093 af Primary=0.00 cfs 0.000 af Outflow=0.09 cfs 0.093 af

Total Runoff Area = 10.978 ac Runoff Volume = 2,272 af Average Runoff Depth = 2.48"
80.91% Pervious = 8.882 ac 19.09% Impervious = 2.096 ac

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Type III 24-hr 10YR Rainfall=5.11"
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Summary for Subcatchment P-1: P-1

Runoff = 6.59 cfs @ 12.10 hrs, Volume= 0.463 af, Depth> 2.98"
Routed to Pond PND1 : PND 1

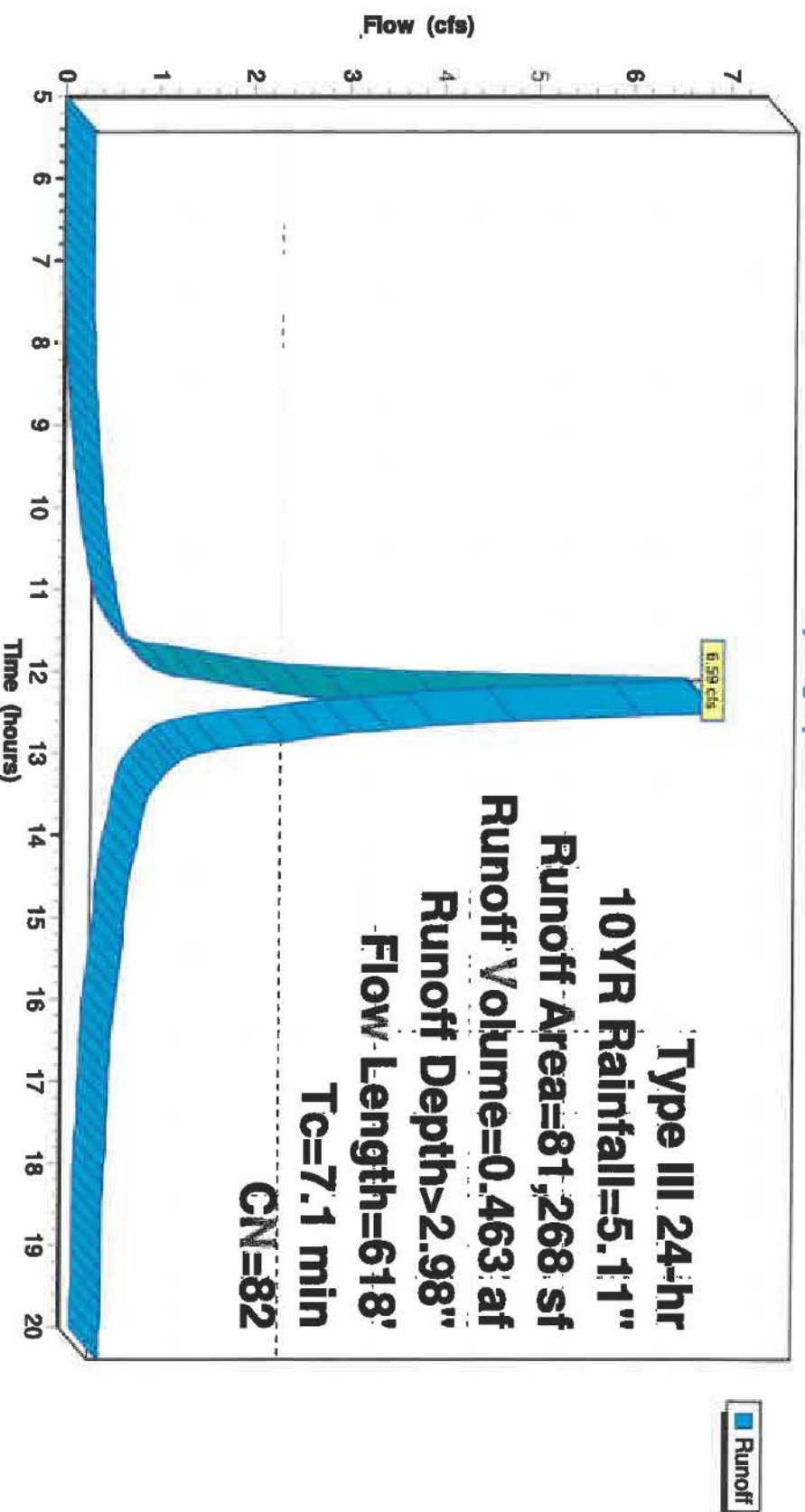
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)	CN	Description
32,428	98	Paved parking, HSG C
26,882	74	>75% Grass cover, Good, HSG C
21,958	70	Woods, Good, HSG C
81,268	82	Weighted Average
48,840		60.10% Pervious Area
32,428		39.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.6	100	0.0450	1.06		Shallow Concentrated Flow, TARVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.6	100	0.0200	2.87		Shallow Concentrated Flow, TARVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	288	0.0250	12.22	21.59	Pipe Channel, TARVEL PATH E TO F
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.010
7.1	618	Total			

Subcatchment P-1: P-1

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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Subcatchment P-2: P-2

Runoff = 6.49 cfs @ 12.10 hrs, Volume= 0.455 af, Depth> 2.88"
Routed to Pond PND2 : BASIN#2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)	CN	Description			
27,755	98	Paved parking, HSG C			
34,256	74	>75% Grass cover, Good, HSG C			
20,380	70	Woods, Good, HSG C			
82,391	81	Weighted Average			
54,636		66.31% Pervious Area			
27,755		33.69% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0450	0.19		Sheet Flow, TRAVEL PATH A TO B
					Cultivated: Residue>20% n= 0.170 P2= 3.20"
0.7	135	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.9	145	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
7.1	330	Total			

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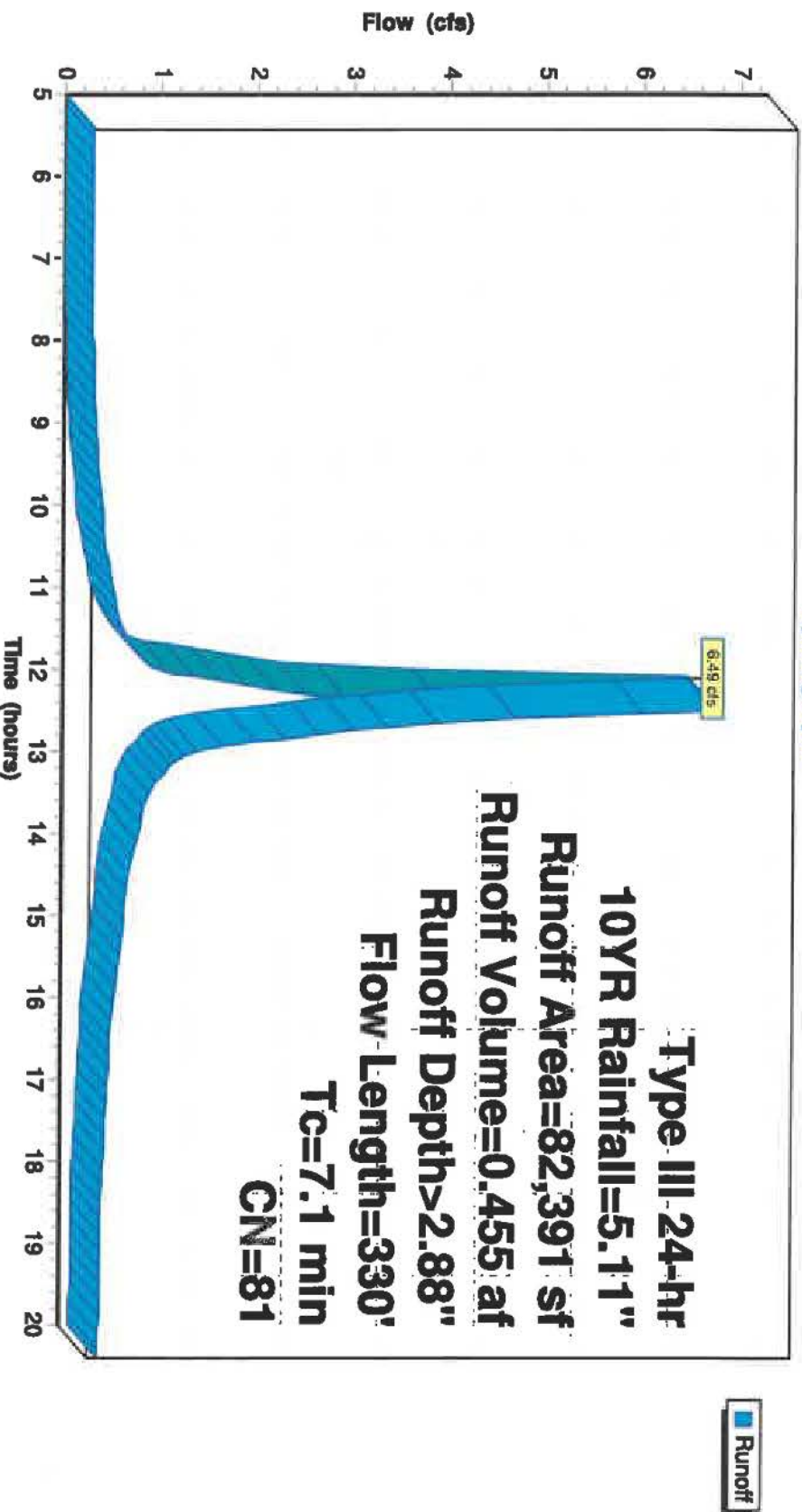
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Subcatchment P-2: P-2

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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Subcatchment P-3: P-3

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 0.055 af, Depth> 3.46"
Routed to Reach IP-P3 : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)		CN	Description		
4,498	98	Paved parking, HSG C			
3,774	74	>75% Grass cover, Good, HSG C			
8,272	87	Weighted Average			
3,774		45.62% Pervious Area			
4,498		54.38% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, TRAVEL PATH

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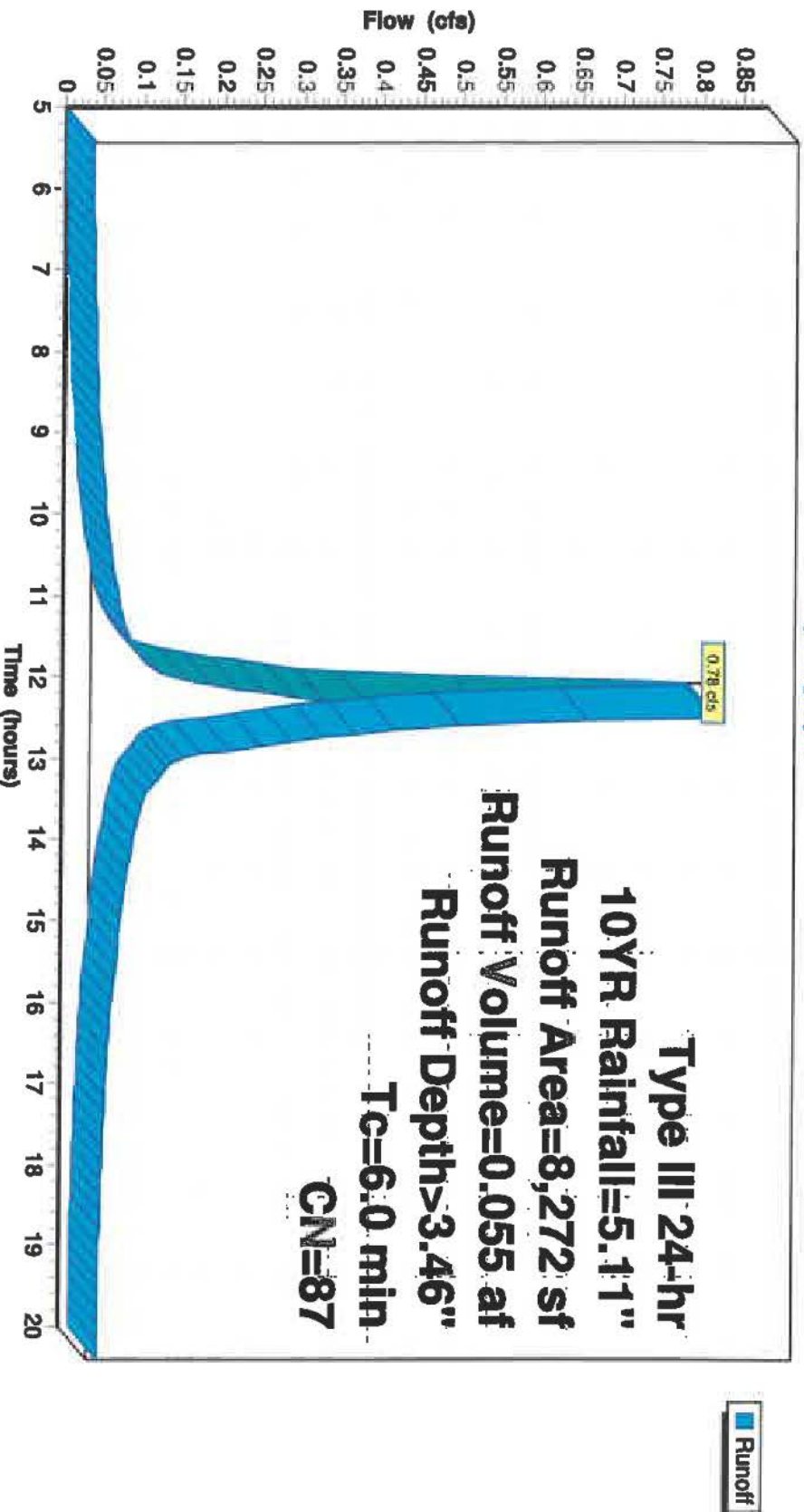
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Subcatchment P-3: P-3

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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Subcatchment P-4: P-4

Runoff = 3.68 cfs @ 12.09 hrs, Volume= 0.248 af, Depth> 2.27"
Routed to Pond PND3 : BASIN3

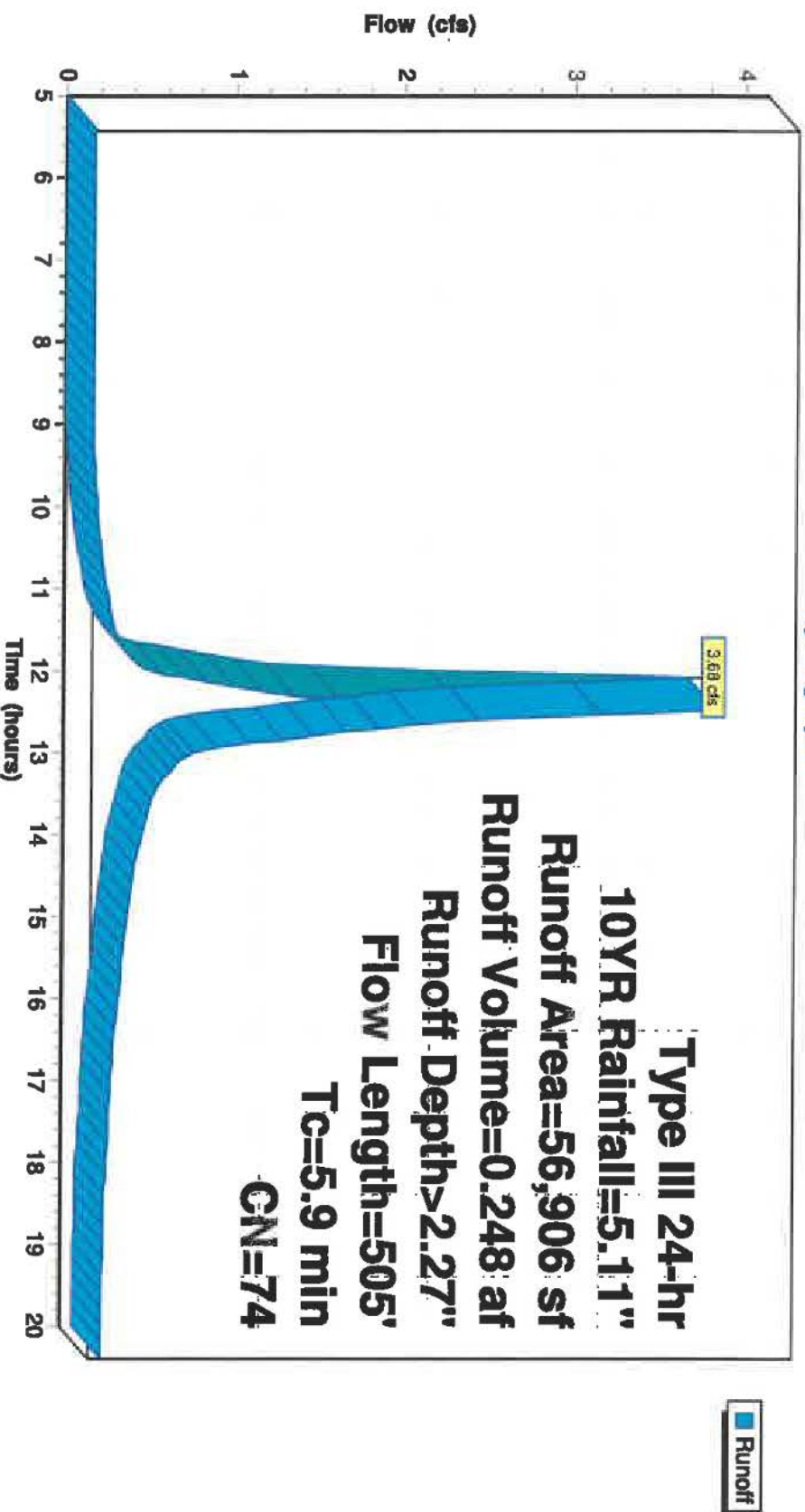
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)	CN	Description
4,207	98	Paved parking, HSG C
22,144	74	>75% Grass cover, Good, HSG C
30,555	70	Woods, Good, HSG C
56,906	74	Weighted Average
52,699		92.61% Pervious Area
4,207		7.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0350	0.19		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
0.5	100	0.4500	3.35		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.1	25	0.0250	3.21		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	250	0.0250	10.82	13.28	Pipe Channel, TRAVEL PATH F TO E
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.010
5.9	505	Total			

Subcatchment P-4: P-4

Hydrograph



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Type III 24-hr 10YR Rainfall=5.11"

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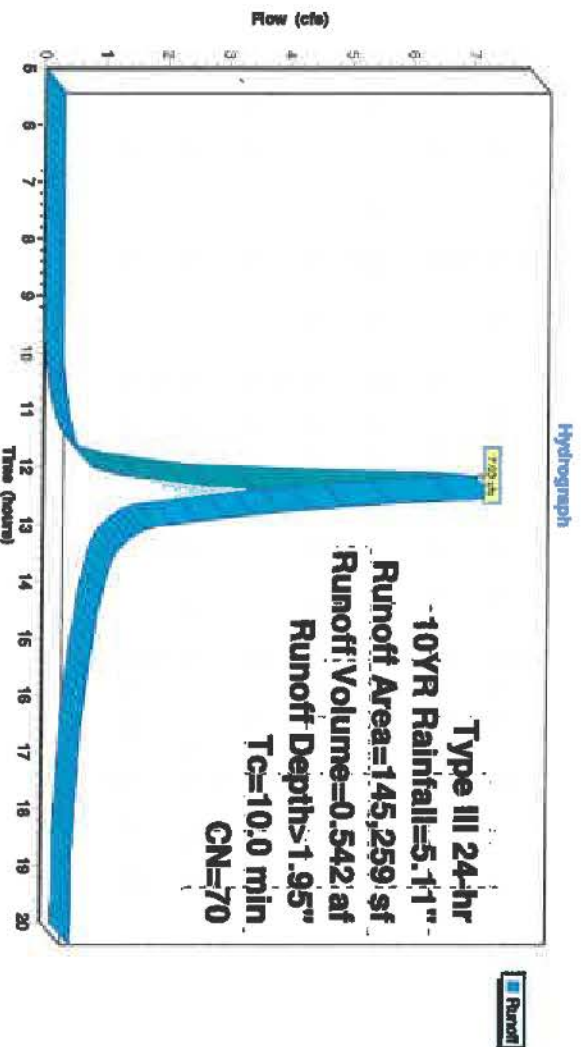
Summary for Subcatchment P-5: P-5

Runoff = 7.03 cfs @ 12.15 hrs, Volume= 0.542 af, Depth> 1.95"
Routed to Reach IP-P1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)		CN	Description		
145,259		70	Woods, Good, HSG C		
145,259			100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					Direct Entry, TRAVEL PATH

Subcatchment P-5: P-5



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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Subcatchment P-6: P-6

Runoff = 4.38 cfs @ 12.12 hrs, Volume= 0.317 af, Depth> 2.03"
Routed to Reach IP-P2 : SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)		CN	Description		
13,023	74	>75% Grass cover, Good, HSG C			
68,668	70	Woods, Good, HSG C			
81,691	71	Weighted Average			
81,691		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
0.9	184	0.0450	3.42		Grass: Short n= 0.150 P2=3.20"
3.1	195	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
					Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
8.1	429	Total			

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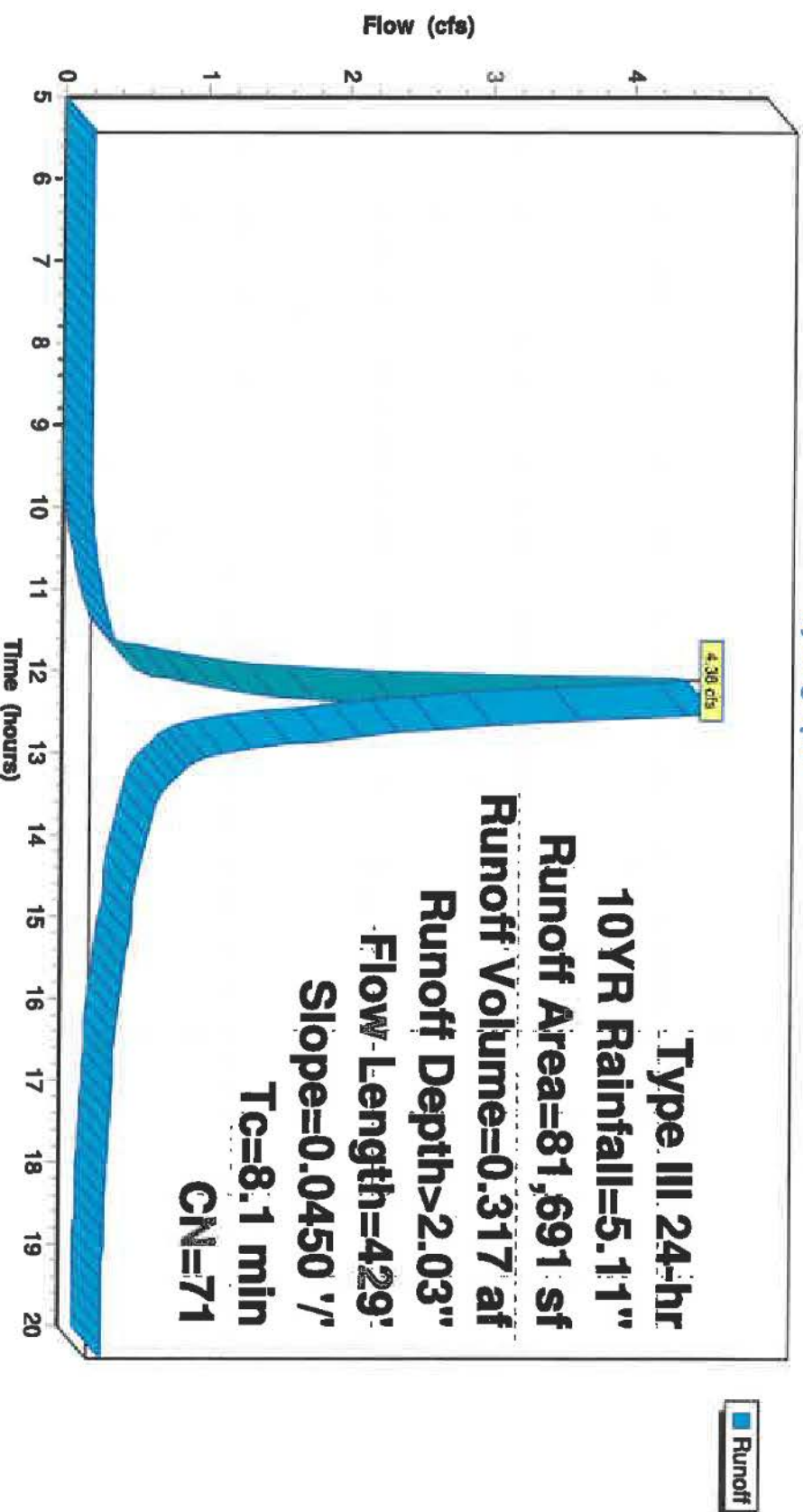
Type III 24-hr 10YR Rainfall=5.11"

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Subcatchment P-6: P-6

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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Subcatchment P-7: BUILDING

Runoff = 2.51 cfs @ 12.09 hrs, Volume= 0.194 af, Depth> 4.52"

Routed to Pond PND4 : U/G BASIN

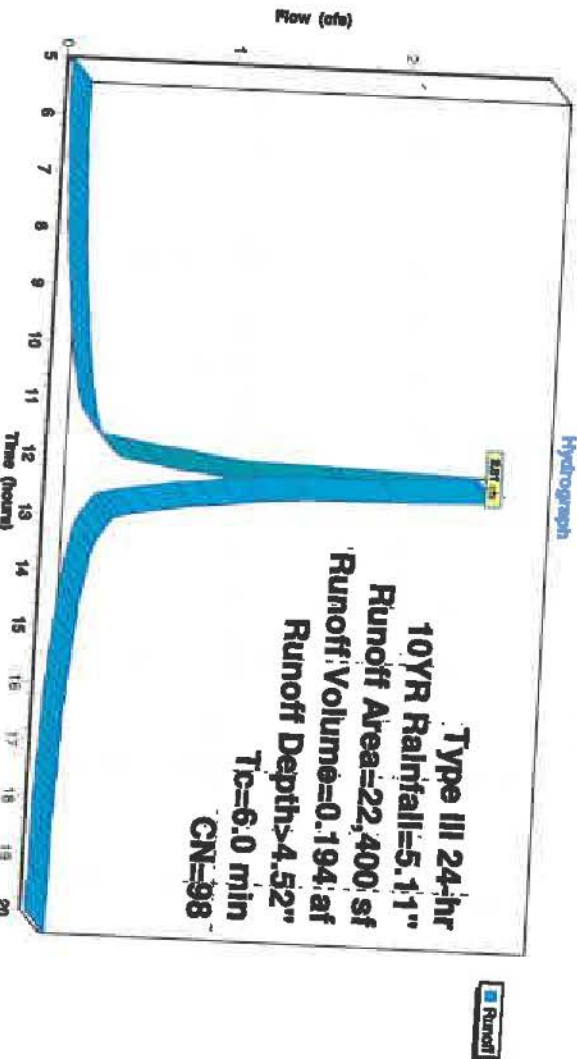
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 10YR Rainfall=5.11"

Area (sf)		CN	Description			
22,400		98	Roofs, HSG C			
22,400			100.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
6.0						

Direct Entry, TRAVEL PATH

Subcatchment P-7: BUILDING

Hydrograph



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Summary for Reach IP-P1: 45 MAIN ST

Inflow Area = 4.641 ac, 2.08% Impervious, Inflow Depth > 1.74" for 10YR event

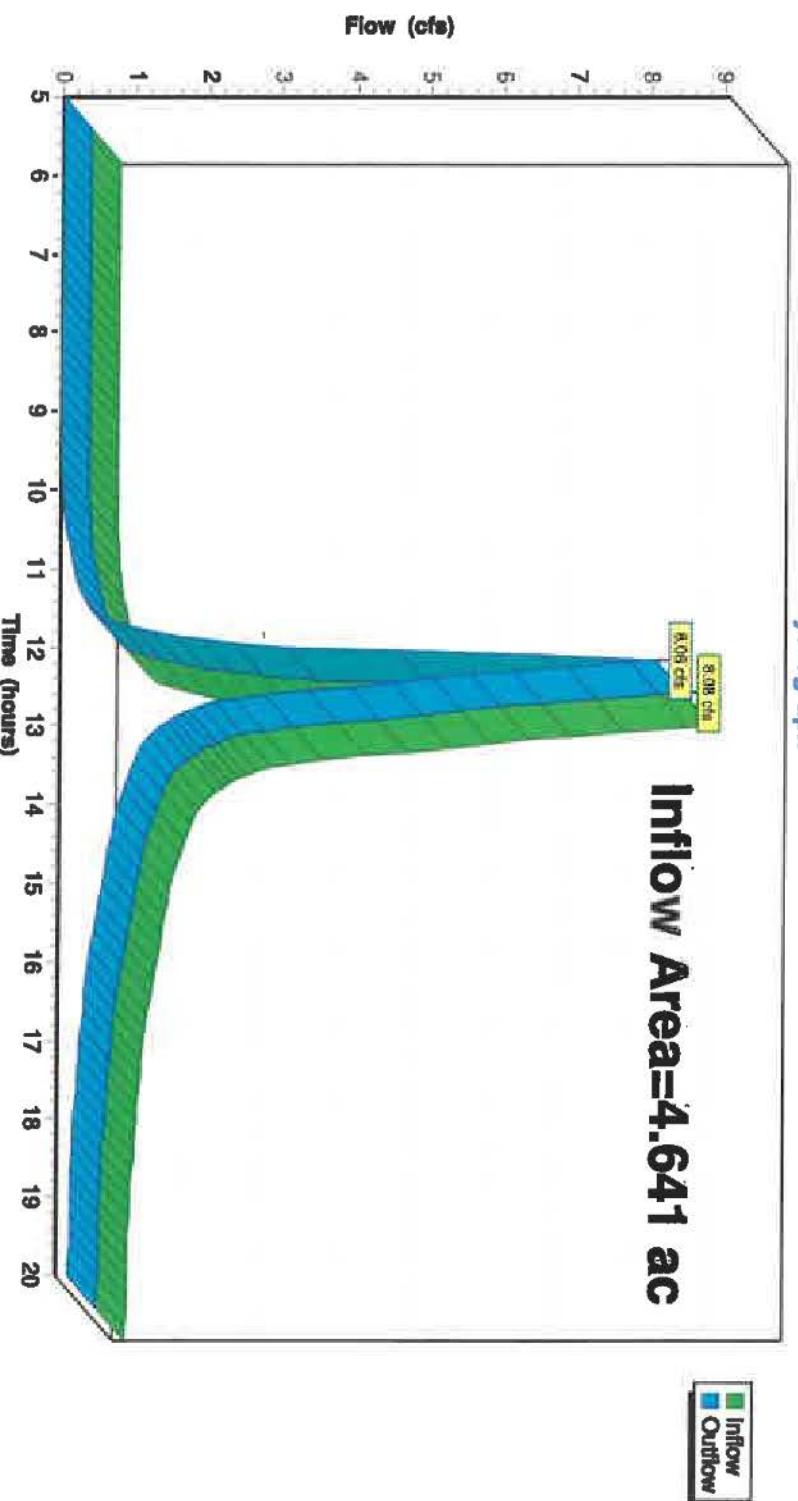
Inflow = 8.08 cfs @ 12.17 hrs, Volume= 0.671 af

Outflow = 8.08 cfs @ 12.17 hrs, Volume= 0.671 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P1: 45 MAIN ST

Hydrograph



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Summary for Reach IP-P2: SCHOOL ST

Inflow Area = 6.147 ac, 30.84% Impervious, Inflow Depth > 1.36" for 10YR event

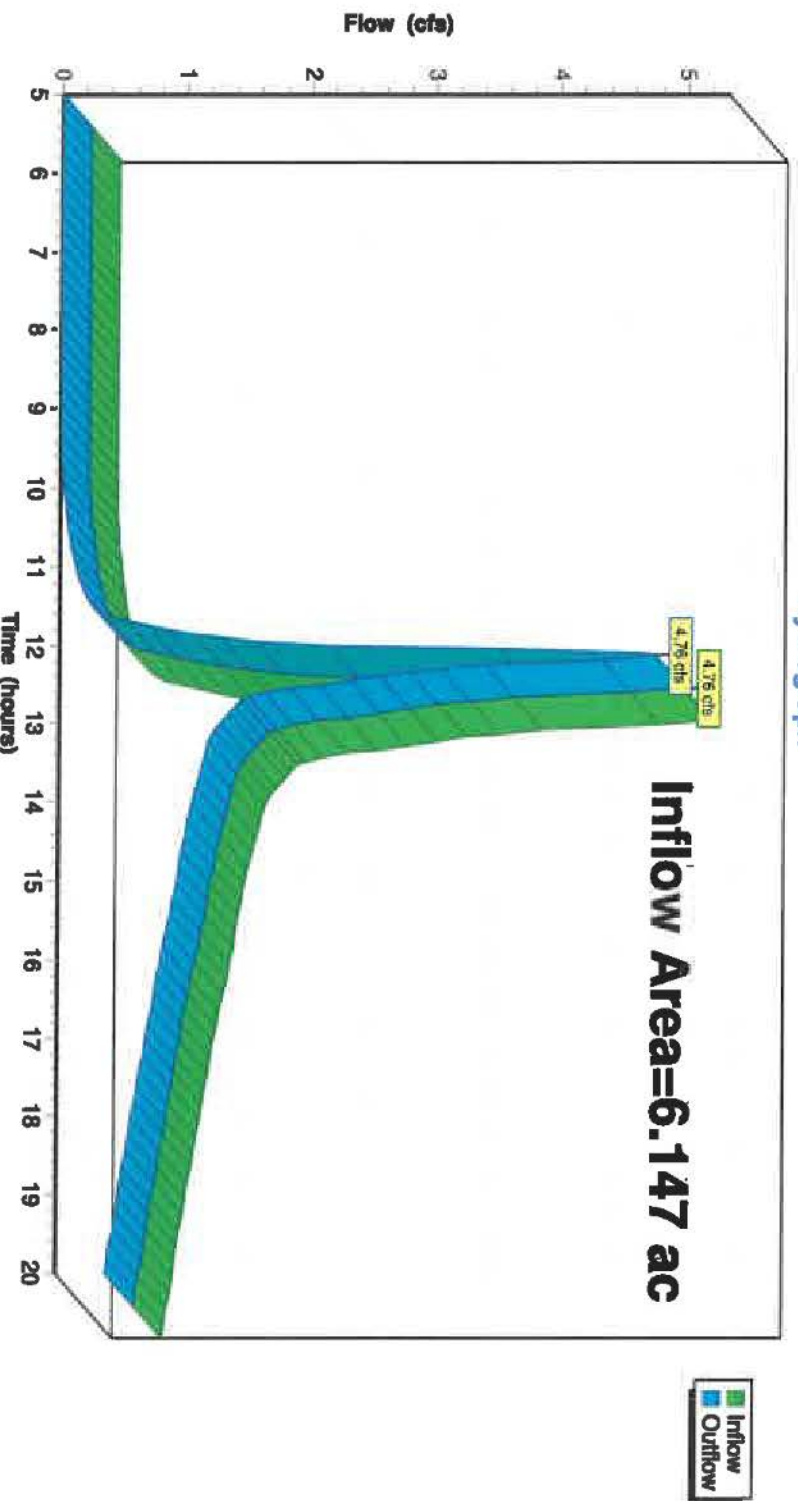
Inflow = 4.76 cfs @ 12.13 hrs, Volume= 0.696 af

Outflow = 4.76 cfs @ 12.13 hrs, Volume= 0.696 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P2: SCHOOL ST

Hydrograph



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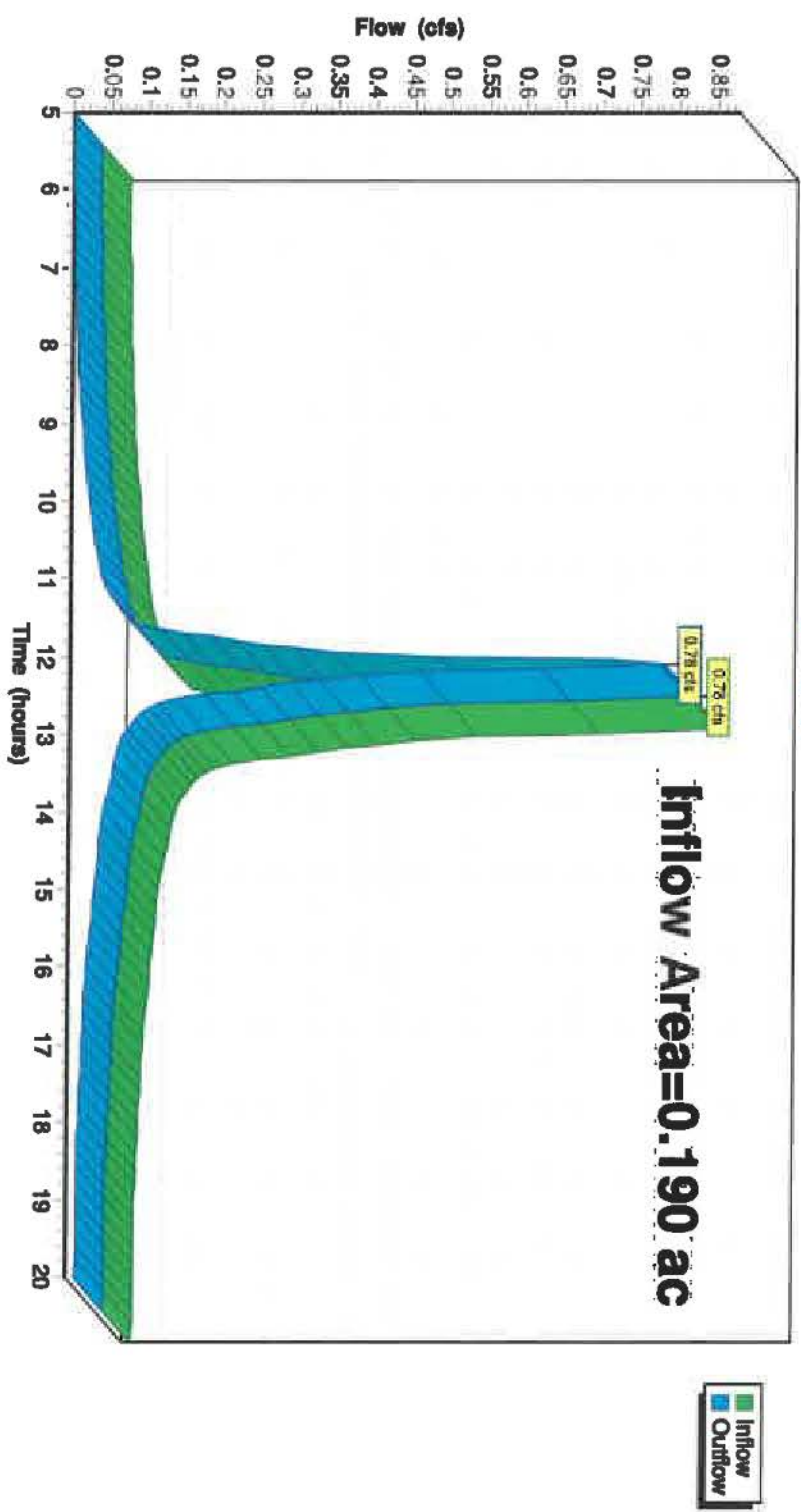
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Summary for Reach IP-P3: EX. CULVERT

Inflow Area = 0.190 ac, 54.38% Impervious, Inflow Depth > 3.46" for 10YR event
Inflow = 0.78 cfs @ 12.09 hrs, Volume= 0.055 af
Outflow = 0.78 cfs @ 12.09 hrs, Volume= 0.055 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P3: EX. CULVERT

Hydrograph



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Type III 24-hr 10YR Rainfall=5.11"
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Summary for Pond PND1: PND 1

Inflow Area = 2.380 ac, 52.89% Impervious, Inflow Depth > 2.33" for 10YR event
Inflow = 6.59 cfs @ 12.10 hrs, Volume= 0.463 af
Outflow = 0.69 cfs @ 12.97 hrs, Volume= 0.346 af, Atten= 89%, Lag= 52.1 min
Discarded = 0.10 cfs @ 12.97 hrs, Volume= 0.065 af
Primary = 0.60 cfs @ 12.97 hrs, Volume= 0.281 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 326.58' @ 12.97 hrs Surf.Area= 4,090 sf Storage= 11,097 cf

Plug-Flow detention time= 194.4 min calculated for 0.345 af (74% of inflow)
Center-of-Mass det. time= 134.1 min (916.7 - 782.6)

Volume Invert Avail.Storage Storage Description

#1	322.00'	23,430 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
322.00	1,030	0	0
324.00	2,125	3,155	3,155
326.00	3,600	5,725	8,880
328.00	5,300	8,900	17,780
329.00	6,000	5,650	23,430

Device Routing Invert Outlet Devices

#1	Discarded	322.00'	1.020 In/hr Exfiltration over Surface area
#2	Primary	324.50'	4.0" Vert. Office/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.50'	4.0" Vert. Office/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	327.50'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00			
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32			

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Type III 24-hr 10YR Rainfall=5.11"

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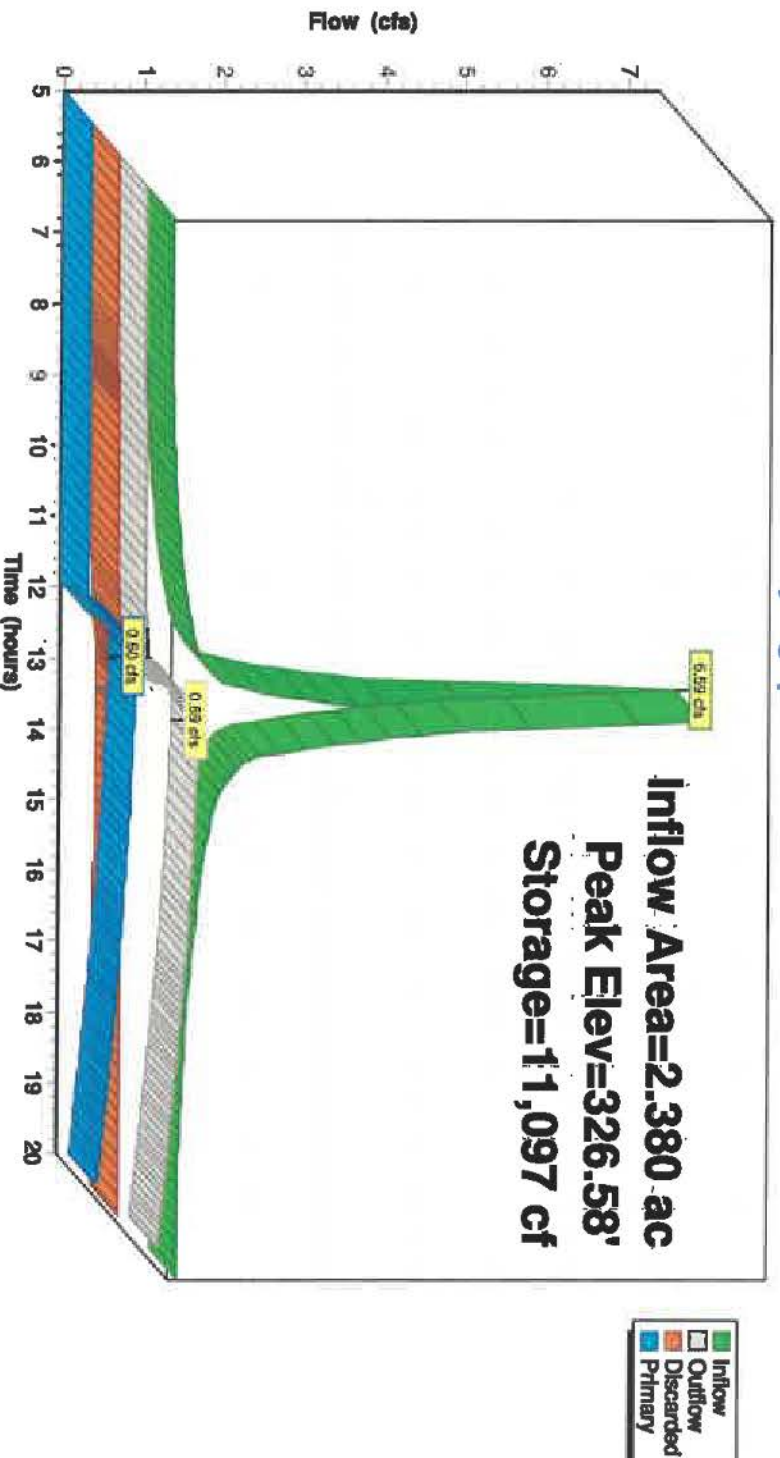
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Discarded OutFlow Max=0.10 cfs @ 12.97 hrs HW=326.58' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.59 cfs @ 12.97 hrs HW=326.58' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.58 cfs @ 6.65 fps)
3=Orifice/Grate (Orifice Controls 0.01 cfs @ 0.94 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND1: PND 1

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Type III 24-hr 10YR Rainfall=5.11"

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Summary for Pond PND2: BASIN#2

Inflow Area = 1.891 ac, 33.69% Impervious, Inflow Depth > 2.88" for 10YR event
Inflow = 6.49 cfs @ 12.10 hrs, Volume= 0.455 af
Outflow = 0.26 cfs @ 15.66 hrs, Volume= 0.188 af, Atten= 96%, Lag= 213.2 min
Discarded = 0.11 cfs @ 15.66 hrs, Volume= 0.091 af
Primary = 0.16 cfs @ 15.66 hrs, Volume= 0.098 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 327.28' @ 15.66 hrs Surf.Area= 4,523 sf Storage= 13,096 cf

Plug-Flow detention time= 230.5 min calculated for 0.188 af (41% of inflow)
Center-of-Mass det. time= 141.8 min (926.6 - 784.9)

Volume Invert Avail.Storage Storage Description

#1	324.00'	20,173 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
			21,436 cf Overall - 1,263 cf Embedded = 20,173 cf	
#2	324.00'	1,263 cf	Cultec R-330XLHD x 24 Inside #1	
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf	
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap	
			Row Length Adjustment= +1.50' x 7.45 sf x 1 rows	
		21,436 cf	Total Available Storage	

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
324.00	3,312	0	0
326.00	4,212	7,524	7,524
328.00	4,700	8,912	16,436
329.00	5,300	5,000	21,436

Device	Routing	Invert	Outlet Devices
#1	Discarded	324.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	325.00'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	327.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	328.00'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir

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Type III 24-hr 10YR Rainfall=5.11"

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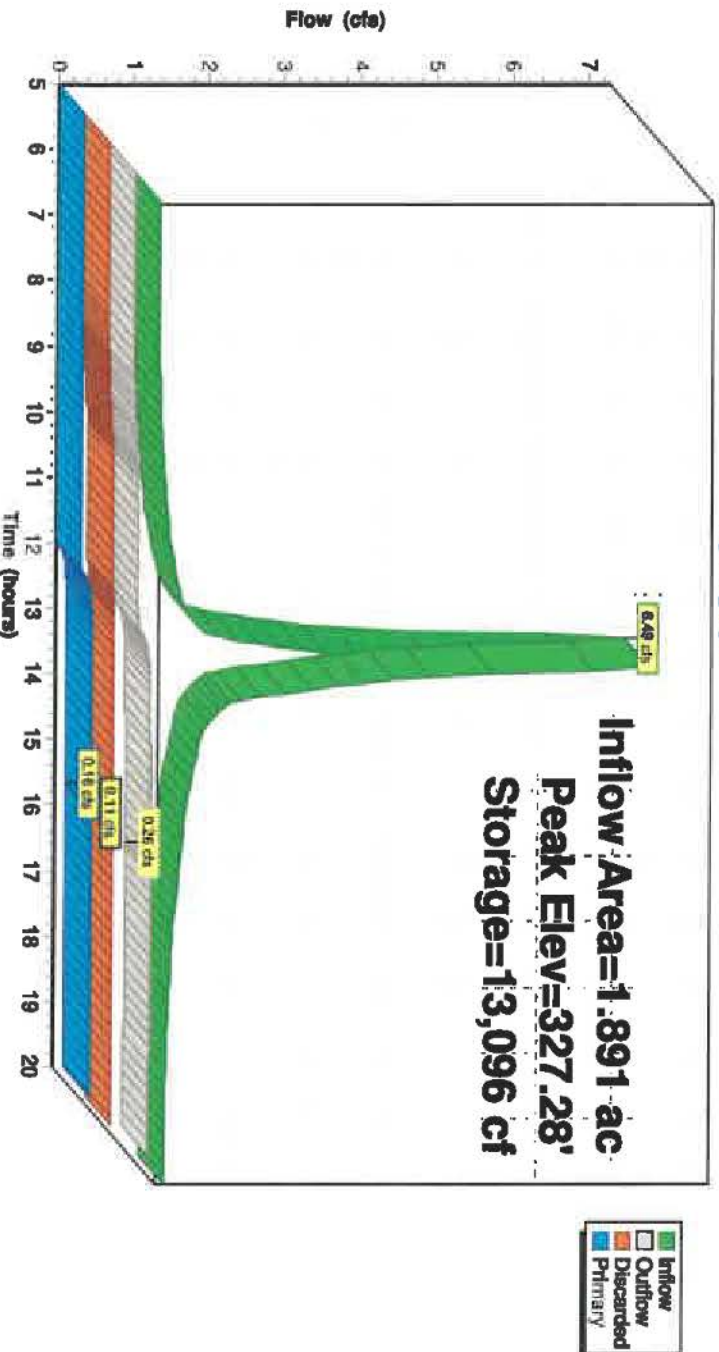
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.11 cfs @ 15.66 hrs HW=327.28' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.16 cfs @ 15.66 hrs HW=327.28' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.13 fps)
3=Orifice/Grate (Controls 0.00 cfs)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND2: BASIN#2

Hydrograph



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Summary for Pond PND3: BASIN3

Inflow Area = 1.306 ac, 7.39% Impervious, Inflow Depth > 2.27' for 10YR event
Inflow = 3.68 cfs @ 12.09 hrs, Volume= 0.248 af
Outflow = 1.67 cfs @ 12.31 hrs, Volume= 0.191 af, Atten= 55%, Lag= 12.9 min
Discarded = 0.09 cfs @ 12.31 hrs, Volume= 0.061 af
Primary = 1.58 cfs @ 12.31 hrs, Volume= 0.130 af
Routed to Reach IP-P1 : 45 MAIN ST

Routing by Stor-Ind method, Time Span=5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 332.97' @ 12.31 hrs Surf.Area= 1,649 sf Storage= 3,587 cf

Plug-Flow detention time= 97.6 min calculated for 0.190 af (77% of inflow)
Center-of-Mass det. time= 40.0 min (838.6 - 798.6)

Volume Invert Avail.Storage Storage Description

#1	330.00'	7,937 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
330.00	925	0	0
332.00	1,250	2,175	2,175
334.00	2,070	3,320	5,495
335.00	2,814	2,442	7,937

Device Routing Invert Outlet Devices

#1	Discarded	330.00'	2,400 In/hr Exfiltration over Surface area
#2	Primary	332.20'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	334.00'	20.0" long x 0.5' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00			
Coef. (English) 2.80 2.92 3.08 3.30 3.32			

POST DEVELOPMENT 1-8-24

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Type III 24-hr 10YR Rainfall=5.11"

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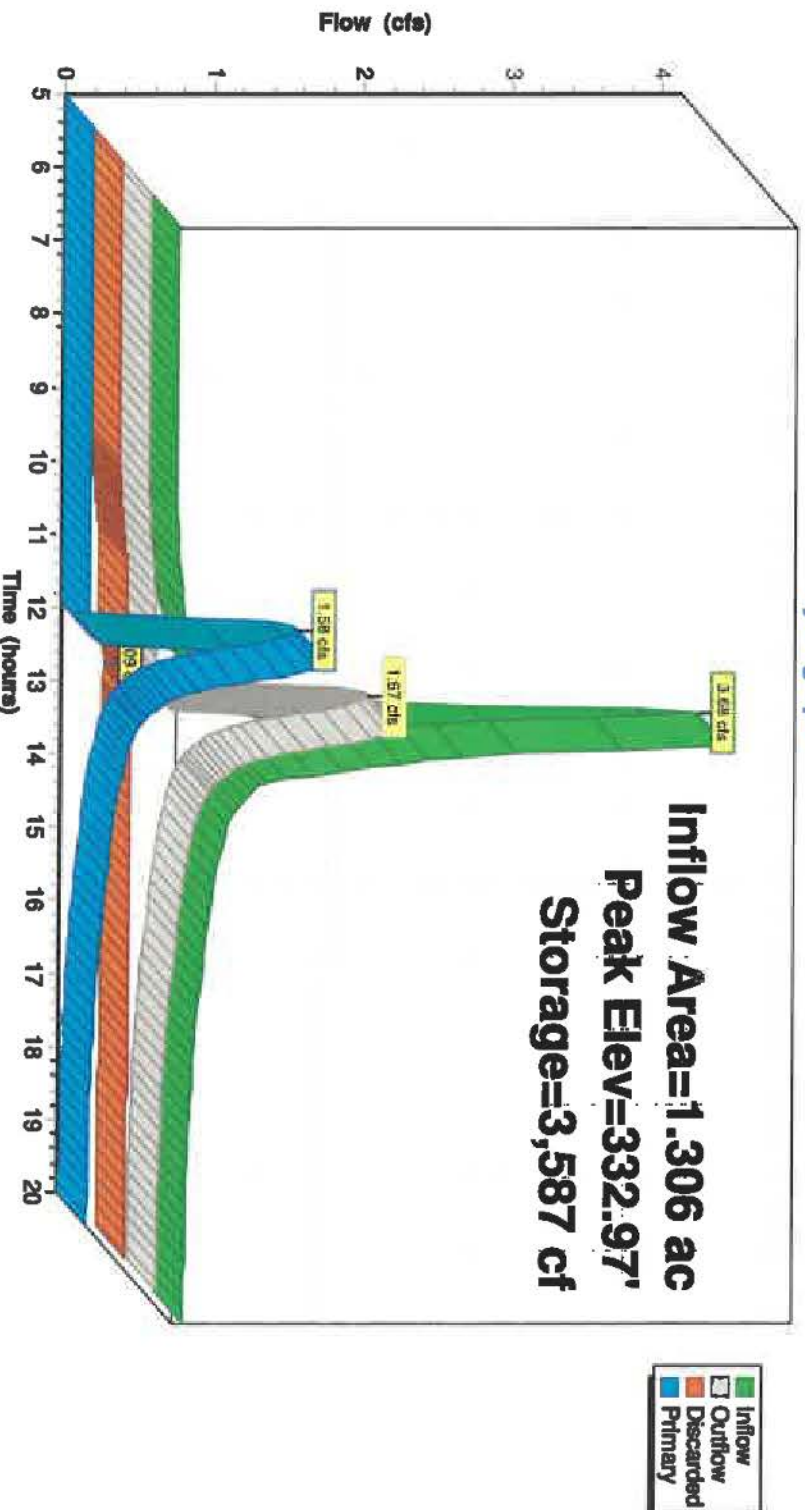
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Discarded OutFlow Max=0.09 cfs @ 12.31 hrs HW=332.97' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=1.58 cfs @ 12.31 hrs HW=332.97' (Free Discharge)
2=Orifice/Grate (Orifice Controls 1.58 cfs @ 2.99 fps)
3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND3: BASIN3

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Type III 24-hr 10YR Rainfall=5.11"
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Summary for Pond PND4: U/G BASIN

Inflow Area = 0.514 ac, 100.00% Impervious, Inflow Depth > 4.52" for 10YR event
Inflow = 2.51 cfs @ 12.09 hrs, Volume= 0.194 af
Outflow = 0.09 cfs @ 9.25 hrs, Volume= 0.093 af, Atten= 97%, Lag= 0.0 min
Discarded = 0.09 cfs @ 9.25 hrs, Volume= 0.093 af
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Routed to Pond PND1 : PND 1

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs,
Peak Elev= 326.50' @ 15.53 hrs Surf.Area= 3,637 sf Storage= 4,956 cf

Plug-Flow detention time= 168.5 min calculated for 0.092 af (48% of inflow)
Center-of-Mass det. time= 69.4 min (804.4 - 735.0)

Volume	Invert	Avail.Storage	Storage Description
#1	325.00'	2,587 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 12,739 cf Overall - 6,270 cf Embedded = 6,468 cf x 40.0% Voids Cultec R-330XLHD x 120 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
#2	325.00'	6,270 cf	
		8,857 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
325.00	3,673	0	0
325.50	3,637	1,828	1,828
328.00	3,637	9,093	10,920
328.50	3,637	1,819	12,739

Device	Routing	Invert	Outlet Devices
#1	Discarded	325.00'	1.020 In/hr Exfiltration over Surface area
#2	Primary	326.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 10YR Rainfall=5.11"

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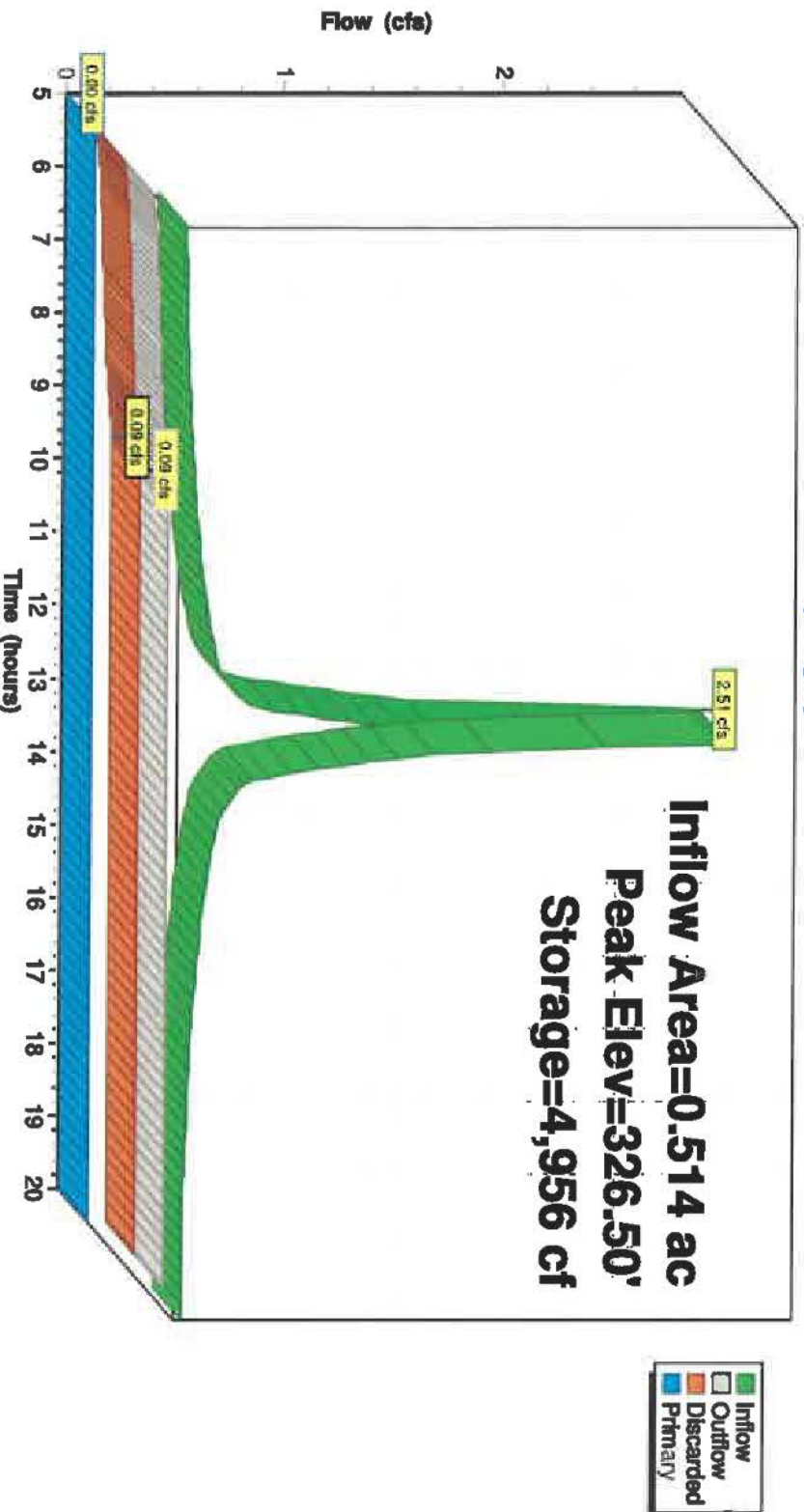
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Discarded OutFlow Max=0.09 cfs @ 9.25 hrs HW=325.04' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=325.00' (Free Discharge)
2=Orifice/Grate (Controls 0.00 cfs)

Pond PND4: U/G BASIN

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Type III 24-hr 25YR Rainfall=6.19"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: P-1

Runoff Area=81,268 sf 39.90% Impervious Runoff Depth>3.91"
Flow Length=618' Tc=7.1 min CN=82 Runoff=8.57 cfs 0.608 af

Subcatchment P-2: P-2

Runoff Area=82,391 sf 33.69% Impervious Runoff Depth>3.81"
Flow Length=330' Tc=7.1 min CN=81 Runoff=8.49 cfs 0.600 af

Subcatchment P-3: P-3

Runoff Area=8,272 sf 54.38% Impervious Runoff Depth>4.44"
Tc=6.0 min CN=87 Runoff=0.99 cfs 0.070 af

Subcatchment P-4: P-4

Runoff Area=56,906 sf 7.39% Impervious Runoff Depth>3.12"
Flow Length=505' Tc=5.9 min CN=74 Runoff=5.04 cfs 0.339 af

Subcatchment P-5: P-5

Runoff Area=145,259 sf 0.00% Impervious Runoff Depth>2.74"
Tc=10.0 min CN=70 Runoff=9.93 cfs 0.760 af

Subcatchment P-6: P-6

Flow Length=429'
Runoff Area=81,691 sf 0.00% Impervious Runoff Depth>2.83"
Slope=0.0450 ' Tc=8.1 min CN=71 Runoff=6.14 cfs 0.442 af

Subcatchment P-7: BUILDING

Runoff Area=22,400 sf 100.00% Impervious Runoff Depth>5.50"
Tc=6.0 min CN=98 Runoff=3.05 cfs 0.236 af

Reach IP-P1: 45 MAIN ST

Inflow=12.20 cfs 0.976 af
Outflow=12.20 cfs 0.976 af

Reach IP-P2: SCHOOL ST

Inflow=6.76 cfs 1.069 af
Outflow=6.76 cfs 1.069 af

Reach IP-P3: EX. CULVERT

Inflow=0.99 cfs 0.070 af
Outflow=0.99 cfs 0.070 af

Pond PND1: PND 1

Peak Elev=327.32' Storage=14,379 cf Inflow=8.57 cfs 0.637 af
Discarded=0.11 cfs 0.075 af Primary=1.02 cfs 0.422 af Outflow=1.14 cfs 0.497 af

Pond PND2: BASIN#2

Peak Elev=327.90' Storage=15,948 cf Inflow=8.49 cfs 0.600 af
Discarded=0.11 cfs 0.098 af Primary=0.45 cfs 0.204 af Outflow=0.56 cfs 0.302 af

Pond PND3: BASIN3

Peak Elev=333.50' Storage=4,505 cf Inflow=5.04 cfs 0.339 af
Discarded=0.10 cfs 0.066 af Primary=2.46 cfs 0.215 af Outflow=2.57 cfs 0.281 af

Pond PND4: U/G BASIN

Peak Elev=326.75' Storage=5,688 cf Inflow=3.05 cfs 0.236 af
Discarded=0.09 cfs 0.096 af Primary=0.12 cfs 0.029 af Outflow=0.20 cfs 0.125 af

Total Runoff Area = 10.978 ac Runoff Volume = 3.056 af Average Runoff Depth = 3.34"
80.91% Pervious = 8.882 ac 19.09% Impervious = 2.096 ac

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Summary for Subcatchment P-1: P-1

Runoff = 8.57 cfs @ 12.10 hrs, Volume= 0.608 af, Depth> 3.91"
Routed to Pond PND1 : PND 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)	CN	Description
32,428	98	Paved parking, HSG C
26,882	74	>75% Grass cover, Good, HSG C
21,958	70	Woods, Good, HSG C
81,268	82	Weighted Average
48,840		60.10% Pervious Area
32,428		39.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.6	100	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.6	100	0.0200	2.87		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	288	0.0250	12.22	21.59	Pipe Channel, TRAVEL PATH E TO F
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.010
7.1	618	Total			

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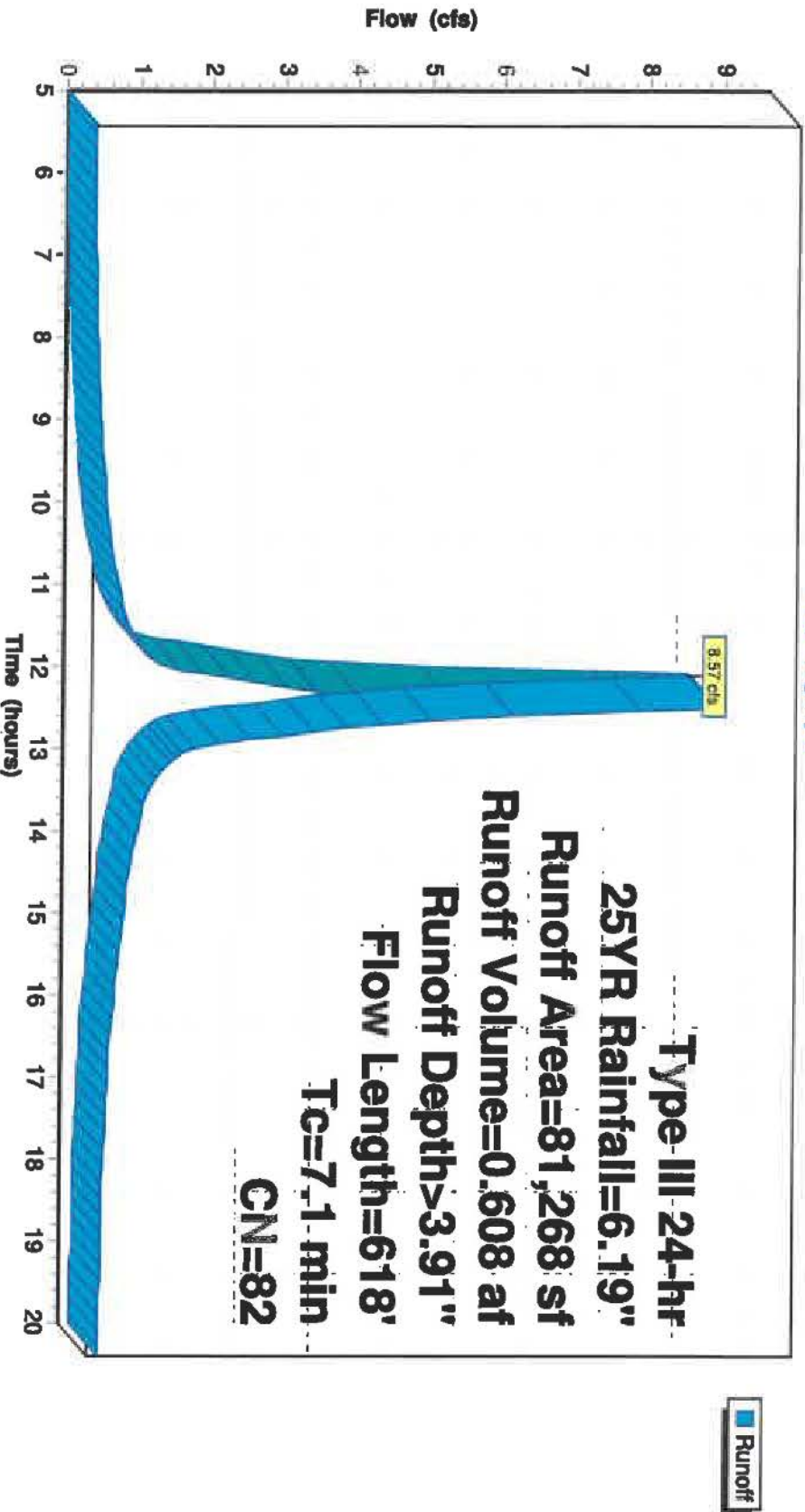
Type III 24-hr 25YR Rainfall=6.19"

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Subcatchment P-1: P-1

Hydrograph



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Summary for Subcatchment P-2: P-2

Runoff = 8.49 cfs @ 12.10 hrs, Volume= 0.600 af, Depth> 3.81"
Routed to Pond PND2 : BASIN#2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)	CN	Description
27,755	98	Paved parking, HSG C
34,256	74	>75% Grass cover, Good, HSG C
20,380	70	Woods, Good, HSG C
82,391	81	Weighted Average
54,636		66.31% Pervious Area
27,755		33.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0450	0.19		Sheet Flow, TRAVEL PATH A TO B
					Cultivated: Residue>20% n=0.170 P2=3.20"
0.7	135	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.9	145	0.0650	1.27		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
7.1	330	Total			

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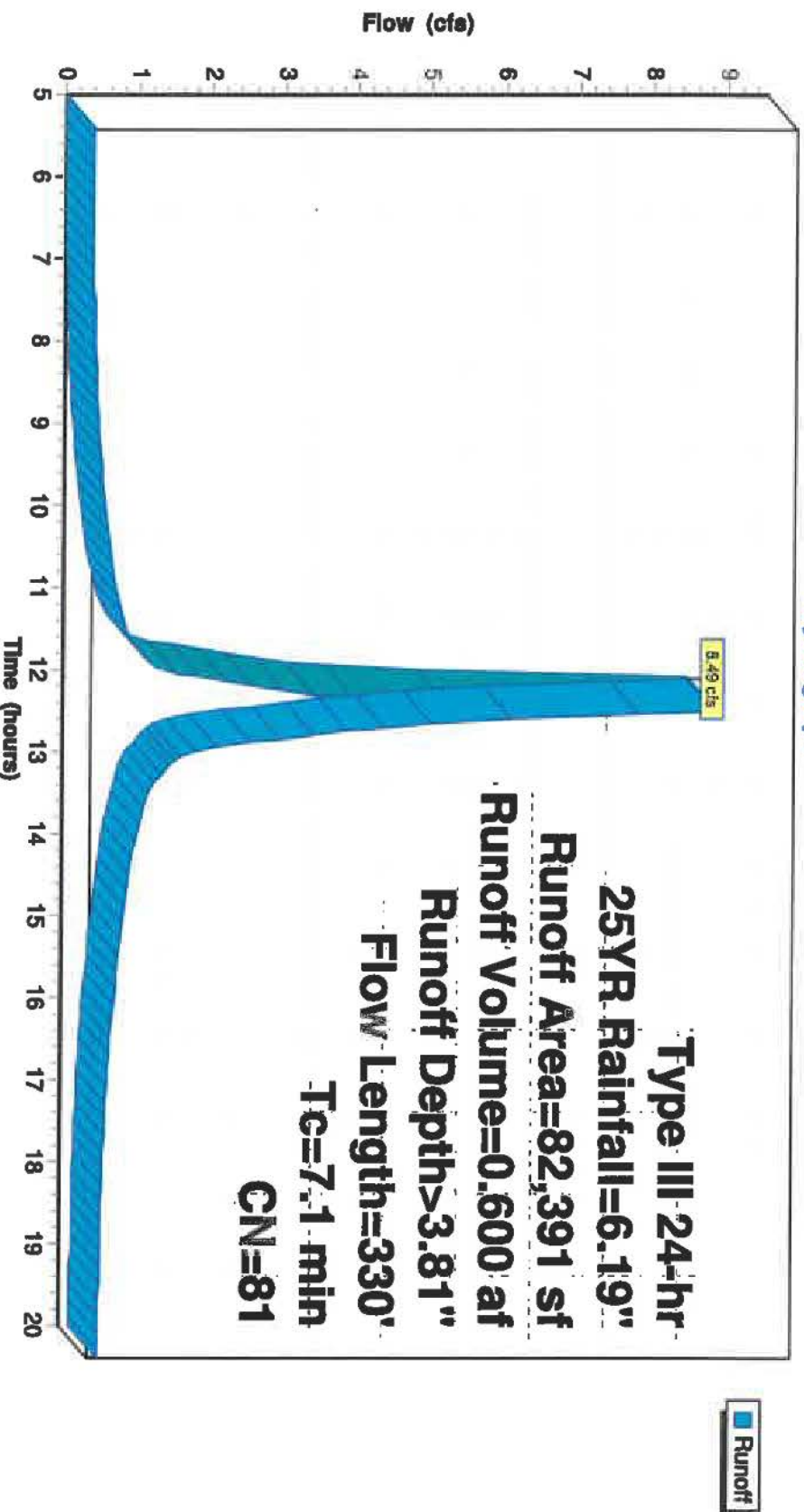
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Subcatchment P-2: P-2

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Summary for Subcatchment P-3: P-3

Runoff = 0.99 cfs @ 12.09 hrs, Volume= 0.070 af, Depth> 4.44"
Routed to Reach IP-P3 : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)		CN	Description		
4,498	98	Paved parking, HSG C			
3,774	74	>75% Grass cover, Good, HSG C			
8,272	87	Weighted Average			
3,774		45.62% Pervious Area			
4,498		54.38% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, TRAVEL PATH

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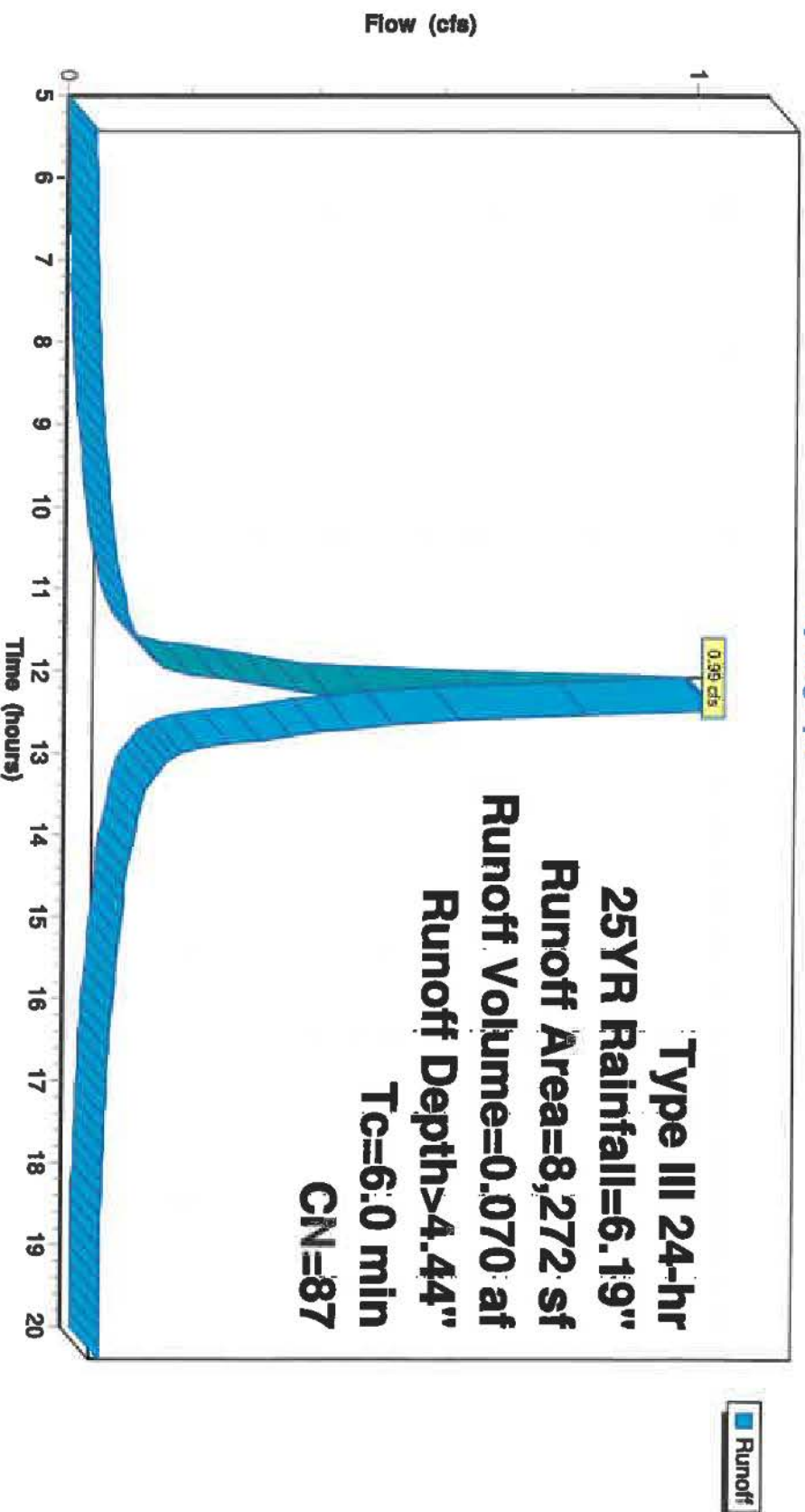
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Subcatchment P-3: P-3

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Summary for Subcatchment P-4: P-4

Runoff = 5.04 cfs @ 12.09 hrs, Volume= 0.339 af, Depth> 3.12"
Routed to Pond PND3 : BASIN3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)	CN	Description
4,207	98	Paved parking, HSG C
22,144	74	>75% Grass cover, Good, HSG C
30,555	70	Woods, Good, HSG C
56,906	74	Weighted Average
52,699		92.61% Pervious Area
4,207		7.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0350	0.19		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
0.5	100	0.4500	3.35		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.1	25	0.0250	3.21		Shallow Concentrated Flow, TARVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	250	0.0250	10.82	13.28	Pipe Channel, TRAVEL PATH F TO E
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.010
5.9	505	Total			

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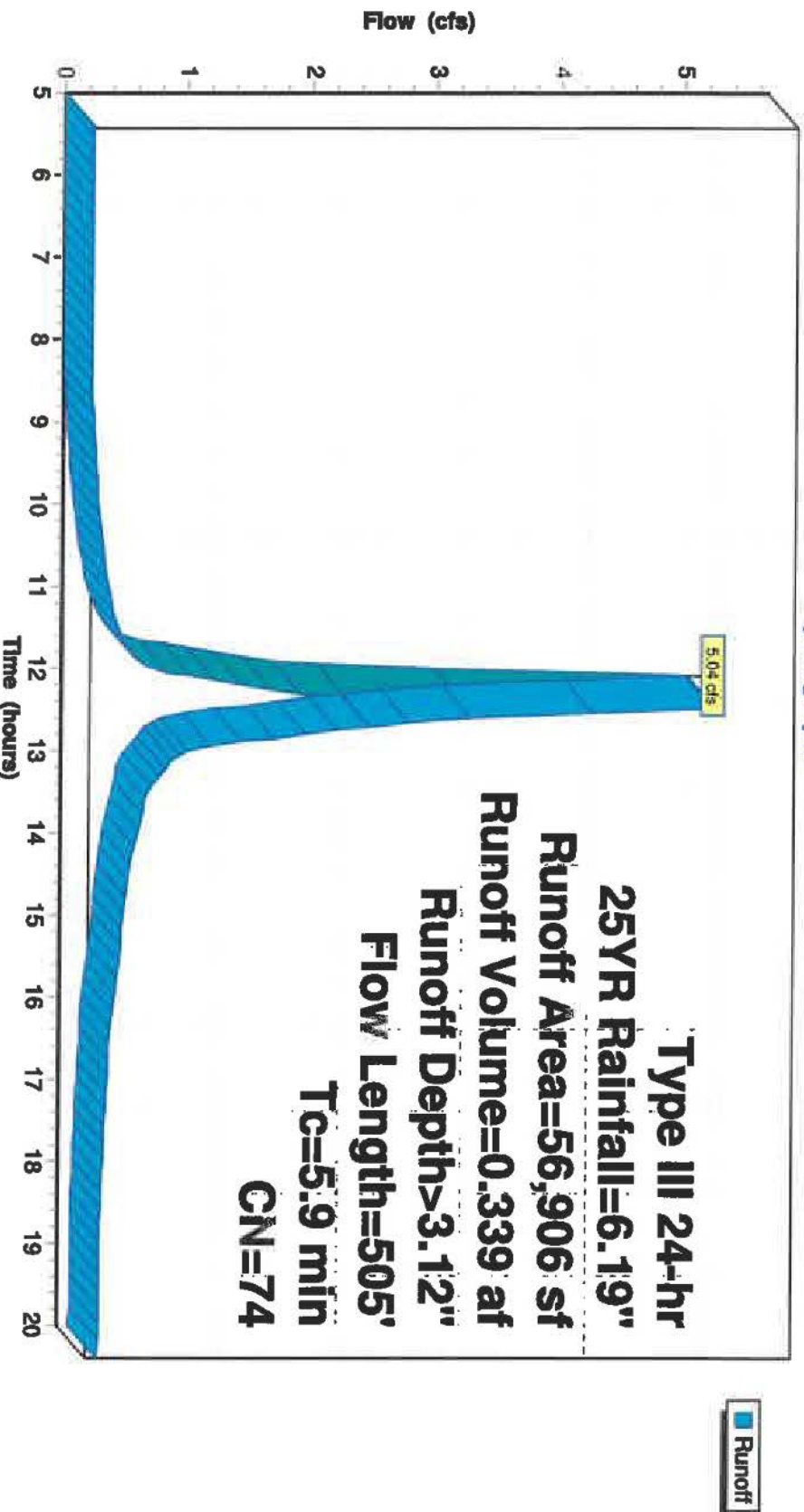
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Subcatchment P-4: P-4

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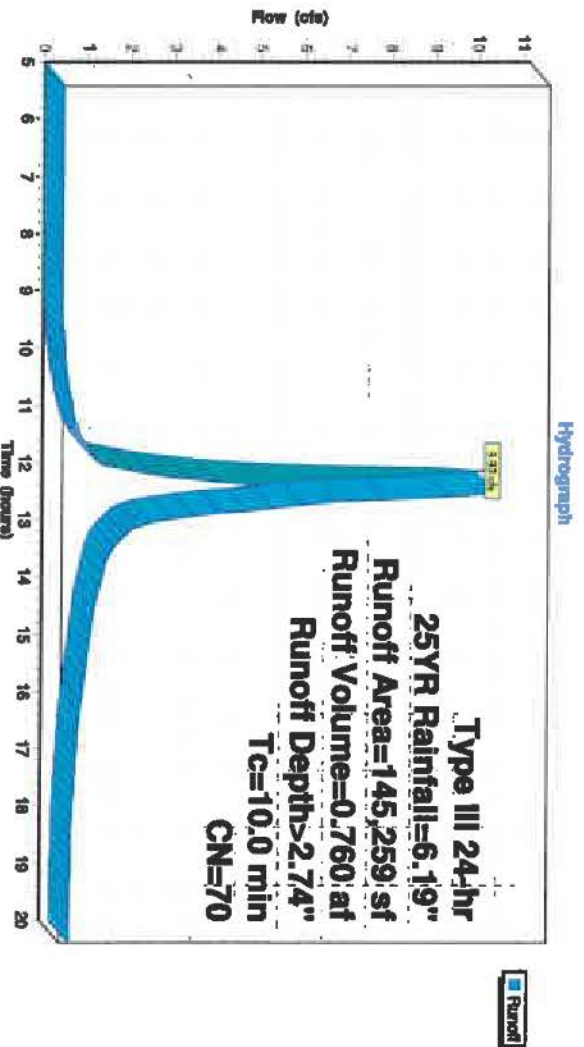
Summary for Subcatchment P-5: P-5

Runoff = 9.93 cfs @ 12.15 hrs, Volume= 0.760 af, Depth> 2.74"
Routed to Reach IP-P1 : 45 MAIN ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)	CN	Description			
145,259	70	Woods, Good, HSG C			
145,259		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					Direct Entry, TRAVEL PATH

Subcatchment P-5: P-5



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Summary for Subcatchment P-6: P-6

Runoff = 6.14 cfs @ 12.12 hrs, Volume= 0.442 af, Depth> 2.83"
Routed to Reach IP-P2 : SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)						CN	Description
13,023						74	>75% Grass cover, Good, HSG C
68,668						70	Woods, Good, HSG C
81,691						71	Weighted Average
81,691							100.00% Pervious Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B		
					Grass: Short n=0.150 P2=3.20"		
0.9	184	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C		
					Unpaved Kv= 16.1 fps		
3.1	195	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D		
					Woodland Kv= 5.0 fps		
8.1	429	Total					

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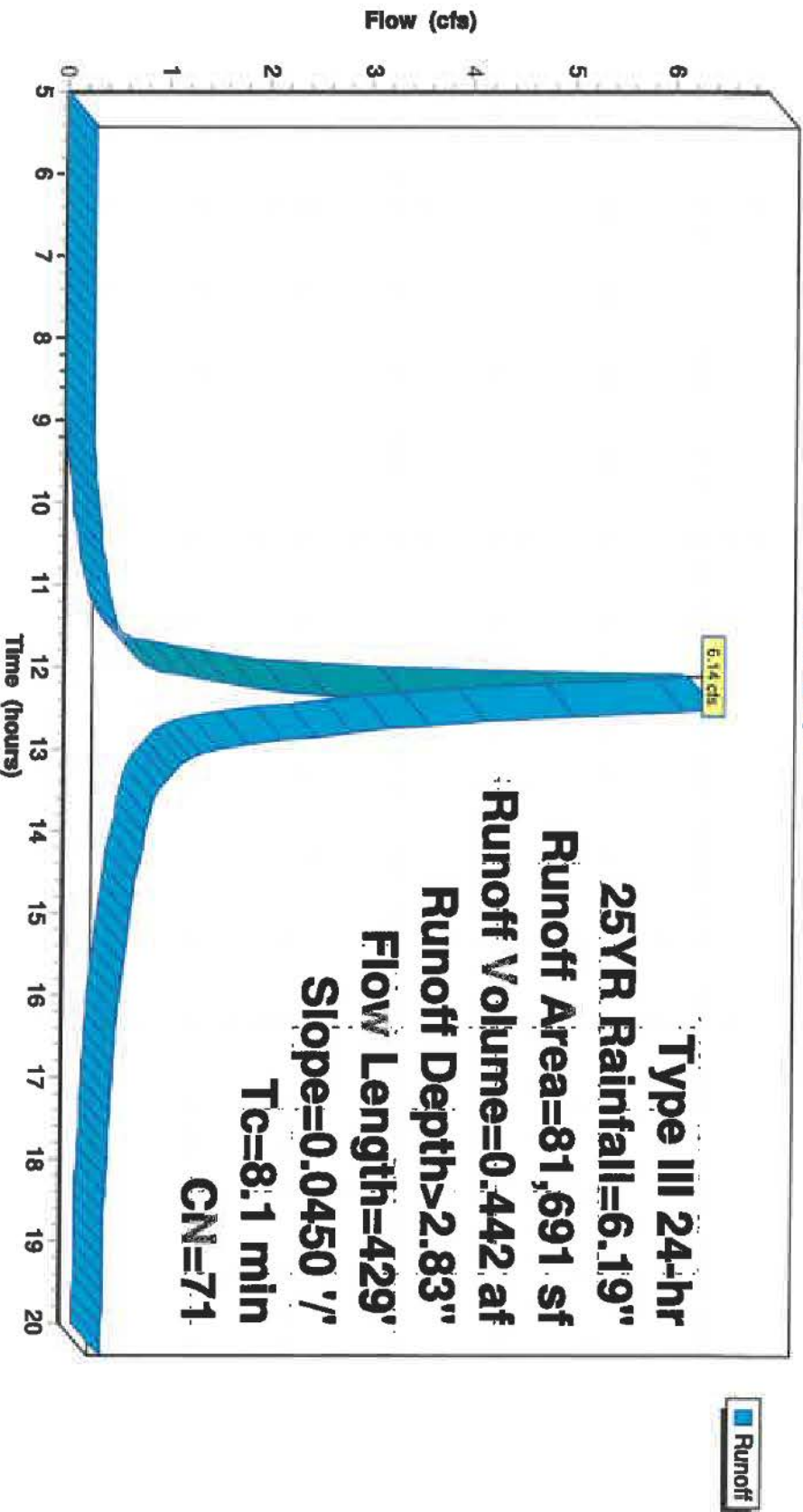
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Subcatchment P-6: P-6

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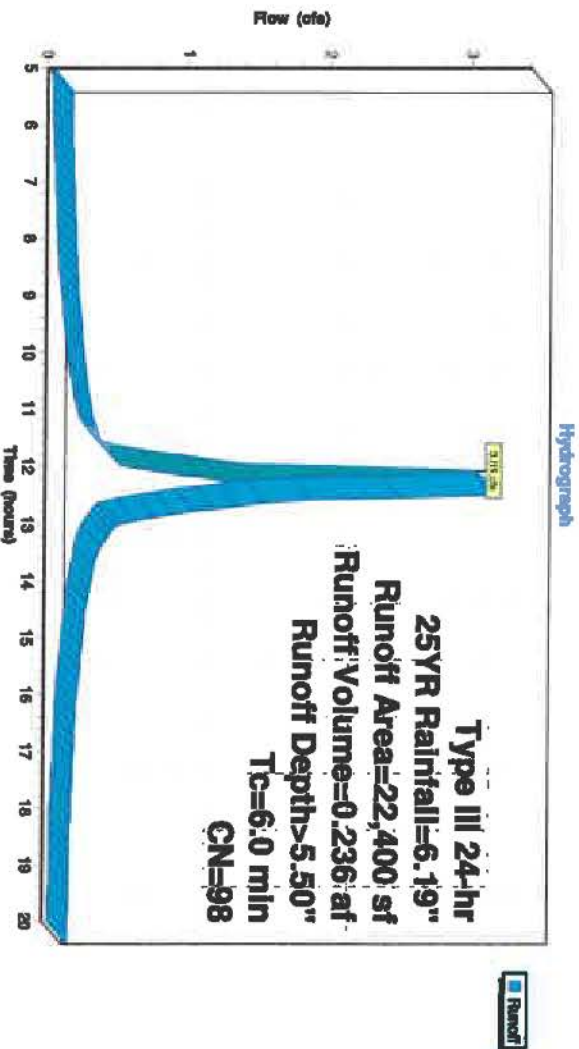
Summary for Subcatchment P-7: BUILDING

Runoff = 3.05 cfs @ 12.09 hrs, Volume= 0.236 af, Depth> 5.50"
Routed to Pond PND4 : U/G BASIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 25YR Rainfall=6.19"

Area (sf)		CN	Description		
22,400		98	Roofs, HSG C		
22,400			100.00% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, TRAVEL PATH

Subcatchment P-7: BUILDING



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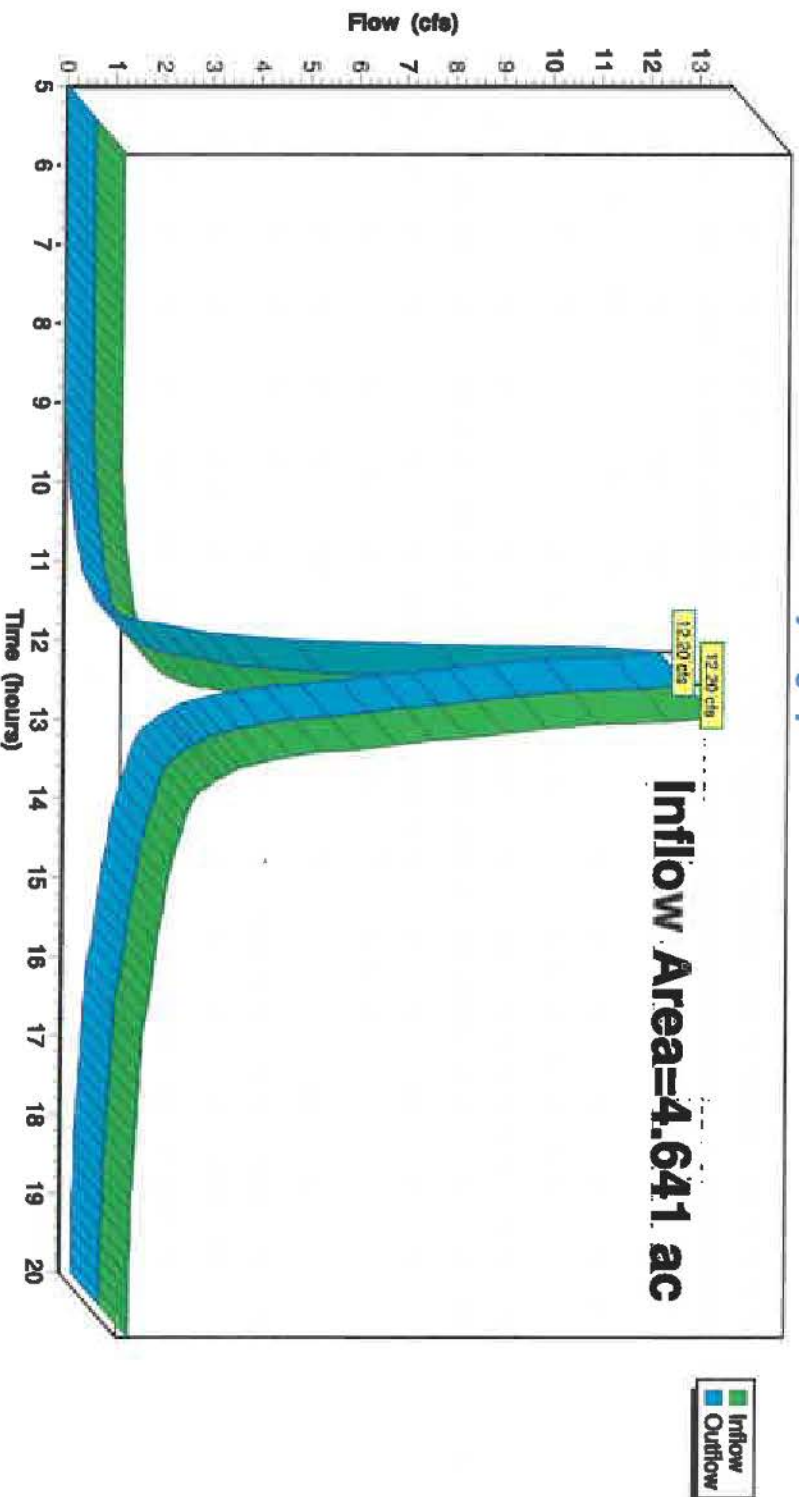
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Summary for Reach IP-P1: 45 MAIN ST

Inflow Area = 4.641 ac, 2.08% Impervious, Inflow Depth > 2.52" for 25YR event
Inflow = 12.20 cfs @ 12.15 hrs, Volume= 0.976 af
Outflow = 12.20 cfs @ 12.15 hrs, Volume= 0.976 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P1: 45 MAIN ST

Hydrograph



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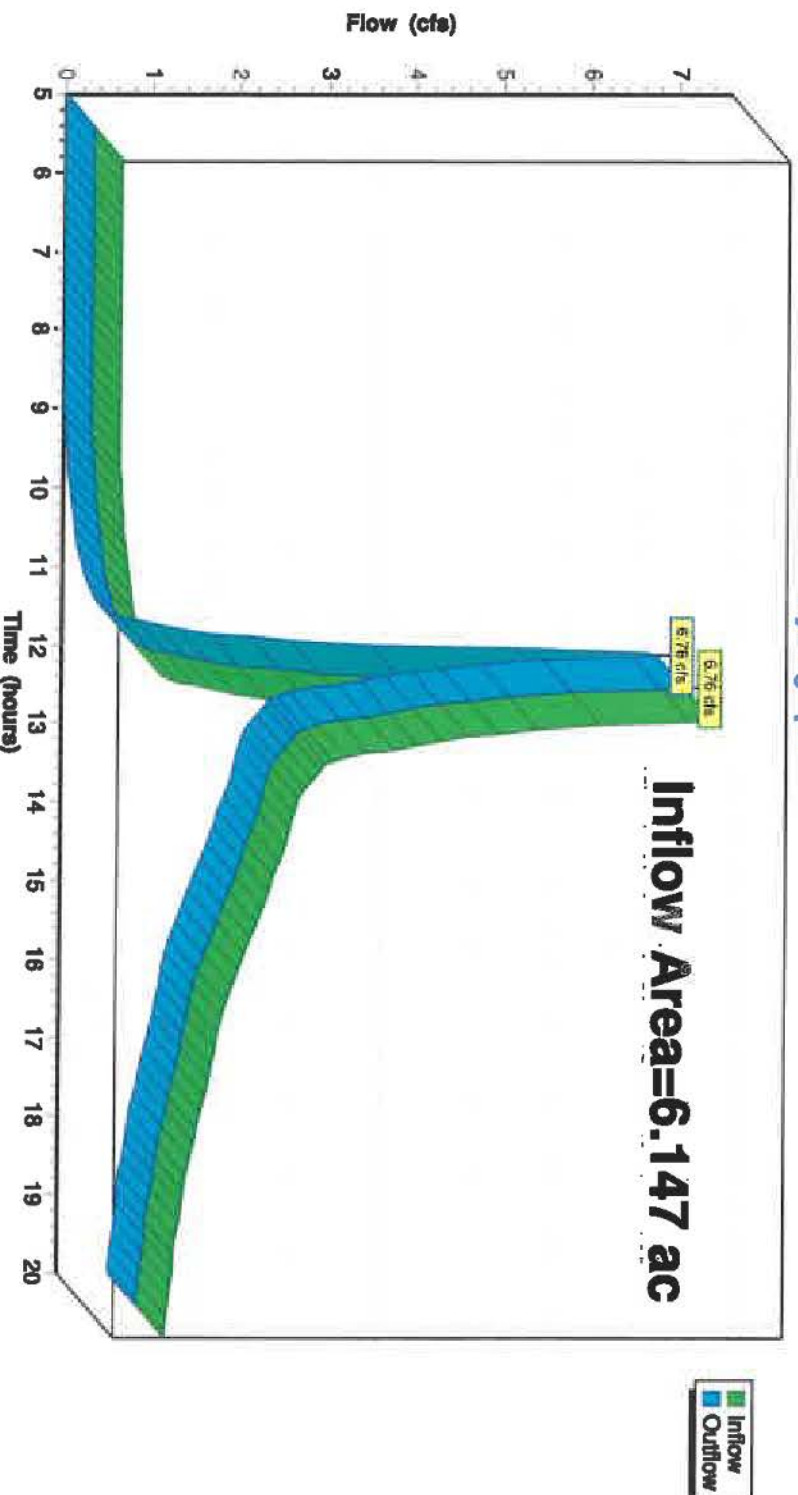
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Summary for Reach IP-P2: SCHOOL ST

Inflow Area = 6.147 ac, 30.84% Impervious, Inflow Depth > 2.09" for 25YR event
Inflow = 6.76 cfs @ 12.12 hrs, Volume= 1.069 af
Outflow = 6.76 cfs @ 12.12 hrs, Volume= 1.069 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P2: SCHOOL ST

Hydrograph



Type III 24-hr 25YR Rainfall=6.19"

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Summary for Pond PND1: PND 1

Inflow Area = 2.380 ac, 52.89% Impervious, Inflow Depth > 3.21" for 25YR event
Inflow = 8.57 cfs @ 12.10 hrs, Volume= 0.637 af
Outflow = 1.14 cfs @ 12.79 hrs, Volume= 0.497 af, Atten= 87%, Lag= 40.9 min
Discarded = 0.11 cfs @ 12.79 hrs, Volume= 0.075 af
Primary = 1.02 cfs @ 12.79 hrs, Volume= 0.422 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 327.32' @ 12.79 hrs Surf.Area= 4,723 sf Storage= 14,379 cf

Plug-Flow detention time= 183.8 min calculated for 0.497 af (78% of inflow)
Center-of-Mass det. time= 128.2 min (908.5 - 780.2)

Volume Invert Avail.Storage Storage Description

#1	322.00'	23,430 cf	Custom Stage Data (Prismatic) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
322.00	1,030	0	0		
324.00	2,125	3,155	3,155		
326.00	3,600	5,725	8,880		
328.00	5,300	8,900	17,780		
329.00	6,000	5,650	23,430		

Device Routing Invert Outlet Devices

#1	Discarded	322.00'	1.020 In/hr Exfiltration over Surface area														
#2	Primary	324.50'	4.0" Vert. Orifice/Grate	C= 0.600	Limited to weir flow at low heads												
#3	Primary	326.50'	4.0" Vert. Orifice/Grate	C= 0.600	Limited to weir flow at low heads												
#4	Primary	327.50'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir														
			Head (feet)	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00	2.50	3.00		
			Coef. (English)	2.69	2.72	2.75	2.85	2.98	3.08	3.20	3.28	3.31	3.30	3.31	3.32		

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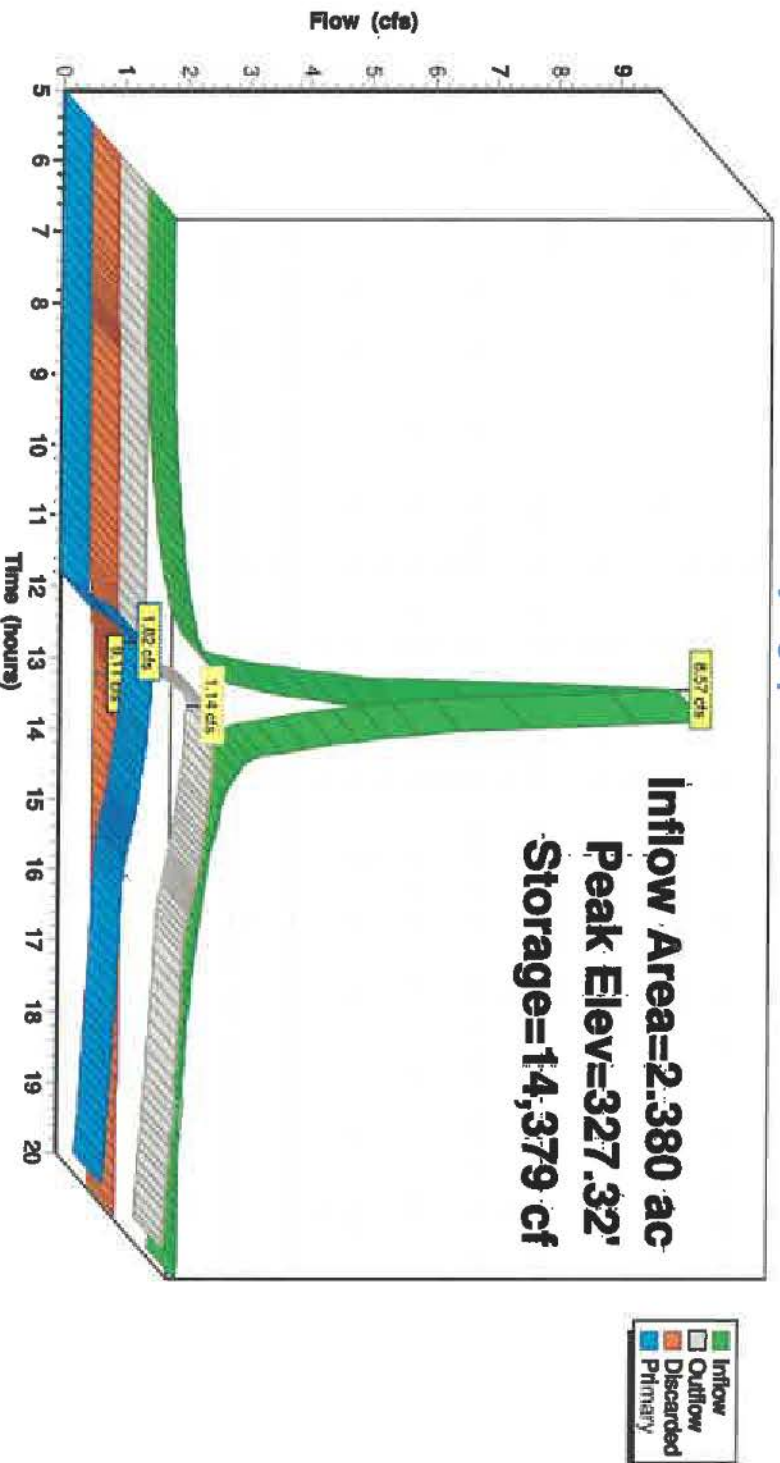
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Discarded OutFlow Max=0.11 cfs @ 12.79 hrs HW=327.32' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=1.02 cfs @ 12.79 hrs HW=327.32' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.68 cfs @ 7.84 fps)
3=Orifice/Grate (Orifice Controls 0.34 cfs @ 3.90 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND1: PND 1

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Summary for Pond PND2: BASIN#2

Inflow Area = 1.891 ac, 33.69% Impervious, Inflow Depth > 3.81" for 25YR event
Inflow = 8.49 cfs @ 12.10 hrs, Volume= 0.600 af
Outflow = 0.56 cfs @ 13.90 hrs, Volume= 0.302 af, Atten= 93%, Lag= 108.0 min
Discarded = 0.11 cfs @ 13.90 hrs, Volume= 0.098 af
Primary = 0.45 cfs @ 13.90 hrs, Volume= 0.204 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 327.90' @ 13.90 hrs Surf.Area= 4,675 sf Storage= 15,948 cf

Plug-Flow detention time= 211.2 min calculated for 0.302 af (50% of inflow)
Center-of-Mass det. time= 128.9 min (907.3 - 778.4)

Volume	Invert	Avail.Storage	Storage Description
#1	324.00'	20,173 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 21,436 cf Overall - 1,263 cf Embedded = 20,173 cf
#2	324.00'	1,263 cf	Cultec R-330XLHD x 24 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
21,436 cf Total Available Storage			

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
324.00	3,312	0	0
326.00	4,212	7,524	7,524
328.00	4,700	8,912	16,436
329.00	5,300	5,000	21,436

Device	Routing	Invert	Outlet Devices
#1	Discarded	324.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	325.00'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	327.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	328.00'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir

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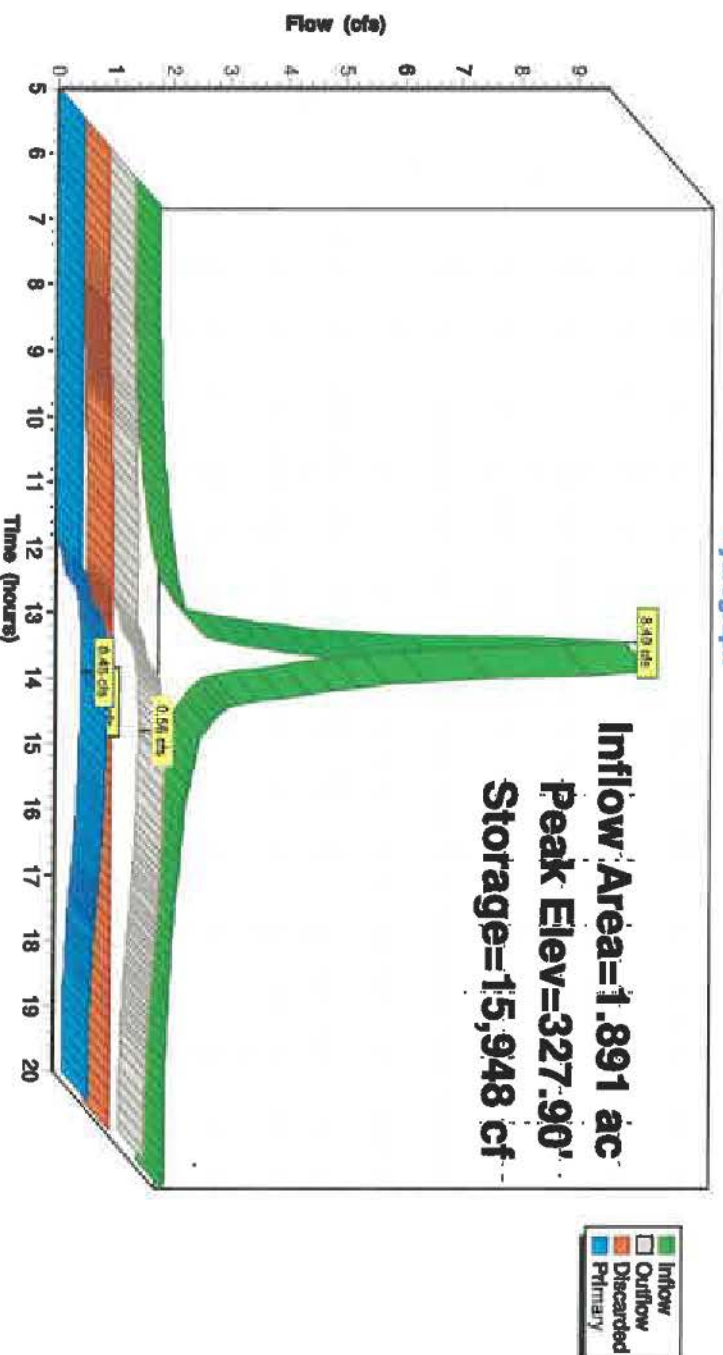
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.11 cfs @ 13.90 hrs HW=327.90' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.45 cfs @ 13.90 hrs HW=327.90' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.18 cfs @ 8.08 fps)
3=Orifice/Grate (Orifice Controls 0.28 cfs @ 3.15 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND2: BASIN#2

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Summary for Pond PND3: BASIN3

Inflow Area = 1.306 ac, 7.39% Impervious, Inflow Depth > 3.12" for 25YR event
Inflow = 5.04 cfs @ 12.09 hrs, Volume= 0.339 af
Outflow = 2.57 cfs @ 12.26 hrs, Volume= 0.281 af, Atten= 49%, Lag= 9.9 min
Discarded = 0.10 cfs @ 12.26 hrs, Volume= 0.066 af
Primary = 2.46 cfs @ 12.26 hrs, Volume= 0.215 af
Routed to Reach IP-P1 : 45 MAIN ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 333.50' @ 12.26 hrs Surf.Area= 1,864 sf Storage= 4,505 cf

Plug-Flow detention time= 79.6 min calculated for 0.281 af (83% of inflow)
Center-of-Mass det. time= 31.6 min (823.0 - 791.5)

Volume Invert Avail.Storage Storage Description

#1 330.00' 7,937 cf Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
330.00	925	0	0
332.00	1,250	2,175	2,175
334.00	2,070	3,320	5,495
335.00	2,814	2,442	7,937

Device Routing Invert Outlet Devices

#1	Discarded	330.00'	2,400 in/hr Exfiltration over Surface area
#2	Primary	332.20'	10.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	334.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00			
Coef. (English) 2.80 2.92 3.08 3.30 3.32			

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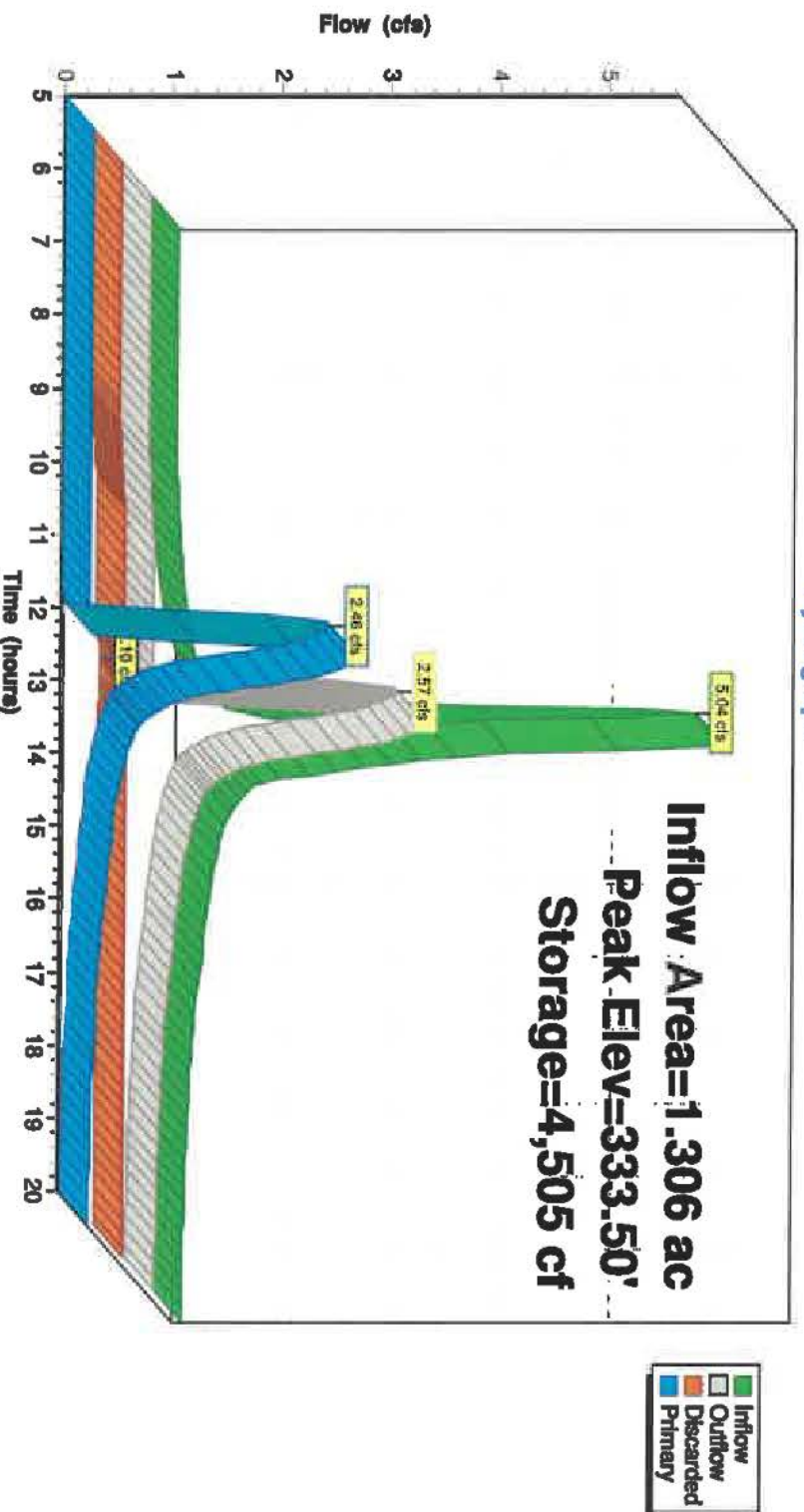
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Discarded OutFlow Max=0.10 cfs @ 12.26 hrs HW=333.49' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=2.46 cfs @ 12.26 hrs HW=333.49' (Free Discharge)
2=Orifice/Grate (Orifice Controls 2.46 cfs @ 4.51 fps)
3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond PND3: BASIN3

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Summary for Pond PND4: U/G BASIN

Inflow Area = 0.514 ac, 100.00% Impervious, Inflow Depth > 5.50" for 25YR event
Inflow = 3.05 cfs @ 12.09 hrs, Volume= 0.236 af
Outflow = 0.20 cfs @ 13.50 hrs, Volume= 0.125 af, Atten= 93%, Lag= 84.7 min
Discarded = 0.09 cfs @ 8.65 hrs, Volume= 0.096 af
Primary = 0.12 cfs @ 13.50 hrs, Volume= 0.029 af
Routed to Pond PND1 : PND 1

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 326.75' @ 13.50 hrs Surf.Area= 3,637 sf Storage= 5,688 cf

Plug-Flow detention time= 165.1 min calculated for 0.125 af (53% of Inflow)
Center-of-Mass det. time= 74.6 min (808.6 - 734.0)

Volume	Invert	Avail.Storage	Storage Description
#1	325.00'	2,587 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 12,739 cf Overall - 6,270 cf Embedded = 6,468 cf x 40.0% Voids Cuttec R-330XLHD x 120 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
#2	325.00'	6,270 cf	
		8,857 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
325.00	3,673	0	0
325.50	3,637	1,828	1,828
328.00	3,637	9,093	10,920
328.50	3,637	1,819	12,739

Device	Routing	Invert	Outlet Devices
#1	Discarded	325.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	326.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Type III 24-hr 25YR Rainfall=6.19"

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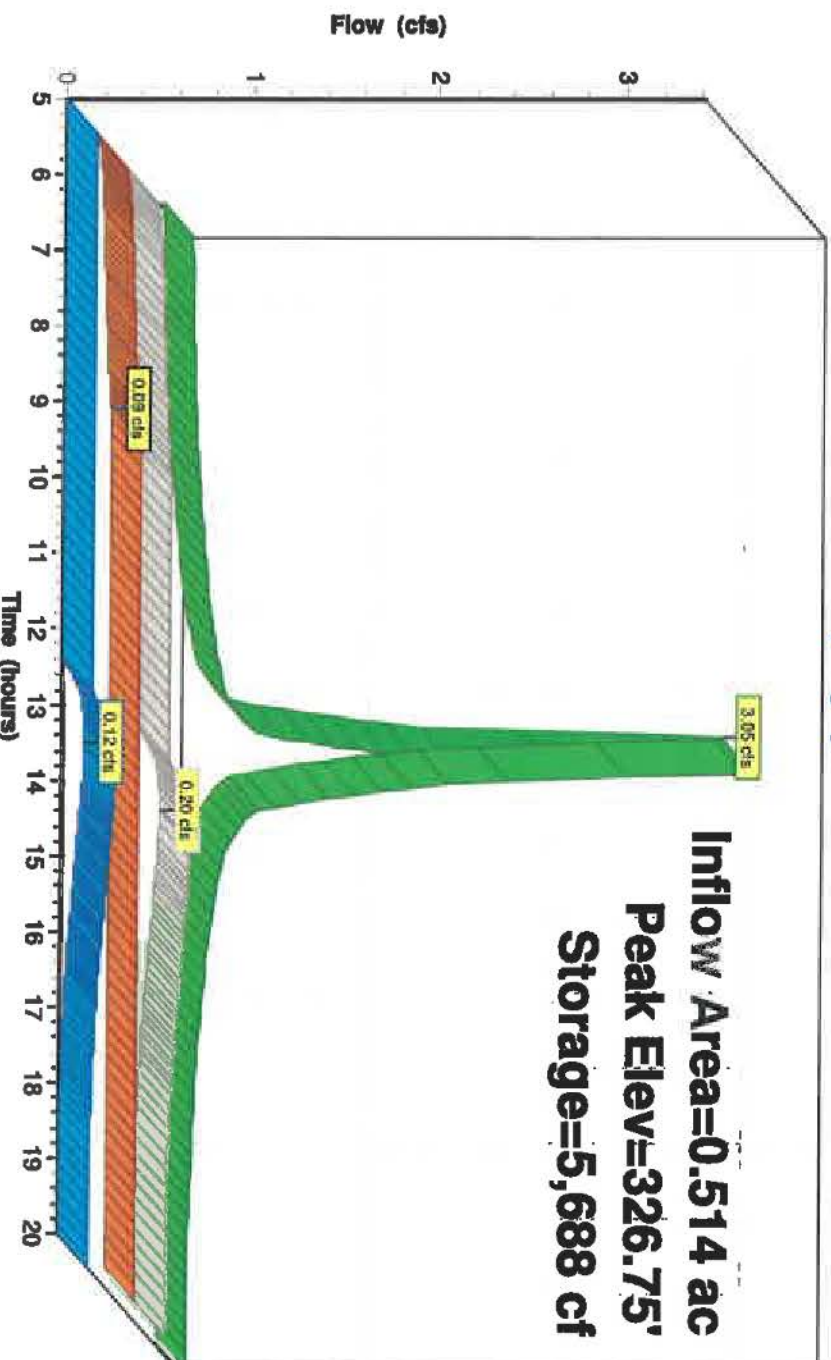
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Discarded OutFlow Max=0.09 cfs @ 8.65 hrs HW=325.04' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.12 cfs @ 13.50 hrs HW=326.75' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.12 cfs @ 1.69 fps)

Pond PND4: U/G BASIN

Hydrograph



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Type III 24-hr 100YR Rainfall=7.85"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: P-1

Runoff Area=81,268 sf 39.90% Impervious Runoff Depth>5.39"
Flow Length=618' Tc=7.1 min CN=82 Runoff=11.63 cfs 0.838 af

Subcatchment P-2: P-2

Runoff Area=82,391 sf 33.69% Impervious Runoff Depth>5.28"
Flow Length=330' Tc=7.1 min CN=81 Runoff=11.59 cfs 0.832 af

Subcatchment P-3: P-3

Runoff Area=8,272 sf 54.38% Impervious Runoff Depth>5.96"
Tc=6.0 min CN=87 Runoff=1.31 cfs 0.094 af

Subcatchment P-4: P-4

Runoff Area=56,906 sf 7.39% Impervious Runoff Depth>4.49"
Flow Length=505' Tc=5.9 min CN=74 Runoff=7.19 cfs 0.488 af

Subcatchment P-5: P-5

Runoff Area=145,259 sf 0.00% Impervious Runoff Depth>4.04"
Tc=10.0 min CN=70 Runoff=14.64 cfs 1.122 af

Subcatchment P-6: P-6

Flow Length=429' Runoff Area=81,691 sf 0.00% Impervious Runoff Depth>4.15"
Slope=0.0450 1' Tc=8.1 min CN=71 Runoff=8.98 cfs 0.649 af

Subcatchment P-7: BUILDING

Runoff Area=22,400 sf 100.00% Impervious Runoff Depth>7.01"
Tc=6.0 min CN=98 Runoff=3.87 cfs 0.300 af

Reach IP-P1: 45 MAIN ST

Inflow=19.08 cfs 1.428 af
Outflow=19.08 cfs 1.428 af

Reach IP-P2: SCHOOL ST

Inflow=11.532 af
Outflow=14.33 cfs 1.532 af

Reach IP-P3: EX. CULVERT

Inflow=1.31 cfs 0.094 af
Outflow=1.31 cfs 0.094 af

Pond PND1: PND 1

Peak Elev=327.73' Storage=16,374 cf Inflow=11.63 cfs 0.923 af
Discarded=0.12 cfs 0.085 af Primary=5.62 cfs 0.669 af Outflow=5.74 cfs 0.755 af

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Type III 24-hr 100YR Rainfall=7.85"

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Pond PND2: BASIN#2

Peak Elev=328.21' Storage=17,456 cf Inflow=11.59 cfs 0.832 af
Discarded=0.11 cfs 0.106 af Primary=4.56 cfs 0.414 af Outflow=4.68 cfs 0.520 af

Pond PND3: BASIN3

Peak Elev=334.10' Storage=5,702 cf Inflow=7.19 cfs 0.488 af
Discarded=0.12 cfs 0.073 af Primary=4.92 cfs 0.356 af Outflow=5.04 cfs 0.429 af

Pond PND4: U/G BASIN

Peak Elev=327.25' Storage=6,951 cf Inflow=3.87 cfs 0.300 af
Discarded=0.09 cfs 0.100 af Primary=0.32 cfs 0.085 af Outflow=0.41 cfs 0.186 af

Total Runoff Area = 10.978 ac Runoff Volume = 4,323 af Average Runoff Depth = 4.73"
80.91% Pervious = 8.882 ac 19.09% Impervious = 2.096 ac

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Type III 24-hr 100YR Rainfall=7.85"
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Summary for Subcatchment P-1: P-1

Runoff = 11.63 cfs @ 12.10 hrs, Volume= 0.838 af, Depth> 5.39"
Routed to Pond PND1 : PND 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)	CN	Description
32,428	98	Paved parking, HSG C
26,862	74	>75% Grass cover, Good, HSG C
21,958	70	Woods, Good, HSG C
81,268	82	Weighted Average
48,840		60.10% Pervious Area
32,428		39.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n= 0.150 P2= 3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
1.6	100	0.0450	1.06		Shallow Concentrated Flow, TARVEL PATH C TO D
					Woodland Kv= 5.0 fps
0.6	100	0.0200	2.87		Shallow Concentrated Flow, TARVEL PATH D TO E
					Paved Kv= 20.3 fps
0.4	288	0.0250	12.22	21.59	Pipe Channel, TARVEL PATH E TO F
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.010
7.1	618	Total			

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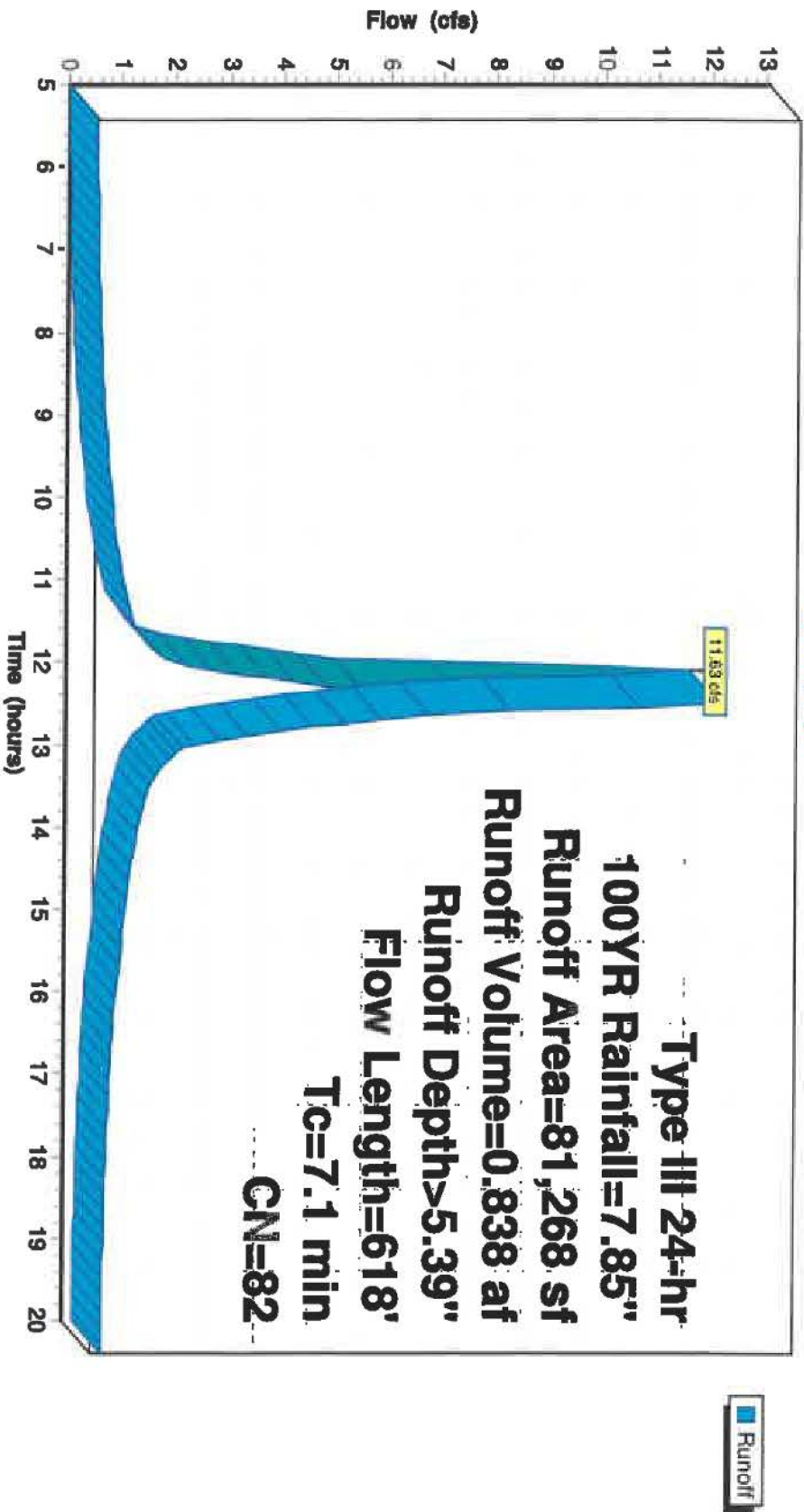
Type III 24-hr 100YR Rainfall=7.85"

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Subcatchment P-1: P-1

Hydrograph



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Type III 24-hr 100YR Rainfall=7.85"

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Summary for Subcatchment P-2: P-2

Runoff = 11.59 cfs @ 12.10 hrs, Volume= 0.832 af, Depth> 5.28"
Routed to Pond PND2 : BASIN#2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)	CN	Description
27,755	98	Paved parking, HSG C
34,256	74	>75% Grass cover, Good, HSG C
20,380	70	Woods, Good, HSG C
82,391	81	Weighted Average
54,636		66.31% Pervious Area
27,755		33.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0450	0.19		Sheet Flow, TRAVEL PATH A TO B
0.7	135	0.0450	3.42		Cultivated: Residue>20% n= 0.170 P2= 3.20" Shallow Concentrated Flow, TRAVEL PATH B TO C
1.9	145	0.0650	1.27		Unpaved Kv= 16.1 fps Shallow Concentrated Flow, TRAVEL PATH C TO D
7.1	330	Total			Woodland Kv= 5.0 fps

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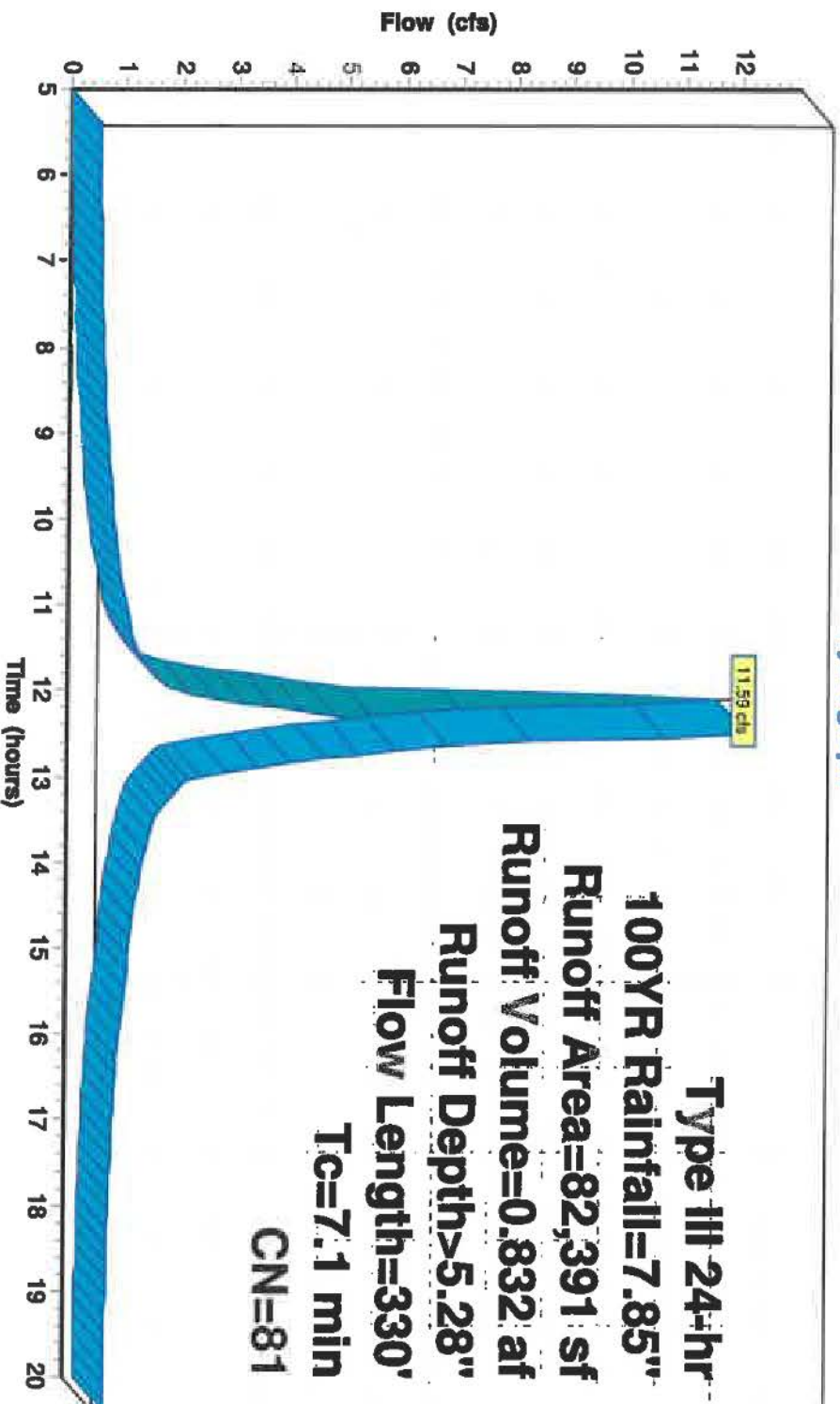
Type III 24-hr 100YR Rainfall=7.85"

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Subcatchment P-2: P-2

Hydrograph



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Summary for Subcatchment P-3: P-3

Runoff = 1.31 cfs @ 12.09 hrs, Volume= 0.094 af, Depth> 5.96"
Routed to Reach IP-P3 : EX. CULVERT

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)		CN	Description		
4,498	98	Paved parking, HSG C			
3,774	74	>75% Grass cover, Good, HSG C			
8,272	87	Weighted Average			
3,774		45.62% Pervious Area			
4,498		54.38% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, TRAVEL PATH

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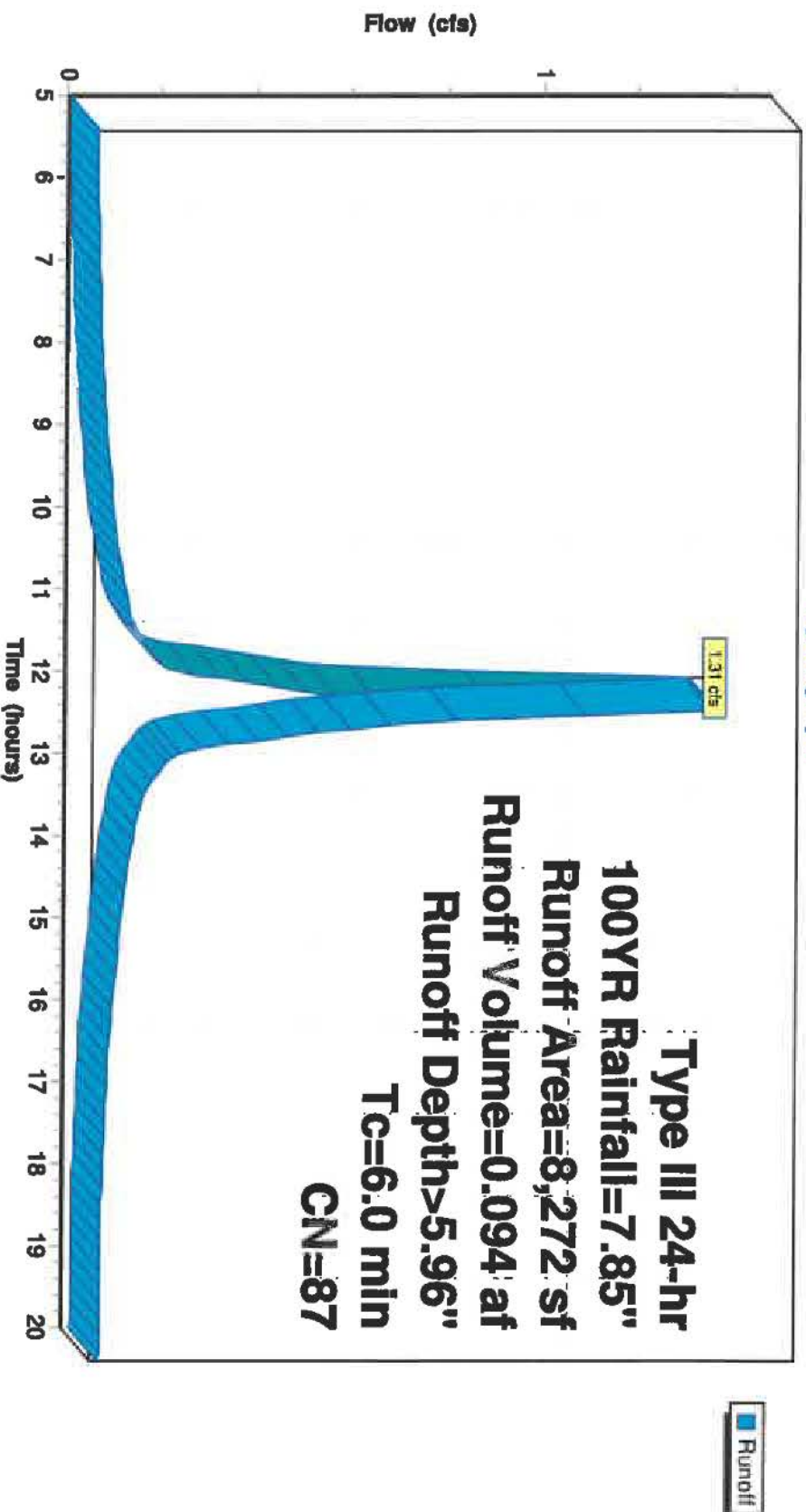
Type III 24-hr 100YR Rainfall=7.85"

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Subcatchment P-3: P-3

Hydrograph



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Summary for Subcatchment P-4: P-4

Runoff = 7.19 cfs @ 12.09 hrs, Volume= 0.488 af, Depth= 4.49"
Routed to Pond PND3 : BASIN3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)		CN	Description		
4,207	98	Paved parking, HSG C			
22,144	74	>75% Grass cover, Good, HSG C			
30,555	70	Woods, Good, HSG C			
56,906	74	Weighted Average			
52,699		92.61% Pervious Area			
4,207		7.39% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.5	50	0.0350	0.19		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n=0.150 P2=3.20"
0.4	80	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv=16.1 fps
0.5	100	0.4500	3.35		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv=5.0 fps
0.1	25	0.0250	3.21		Shallow Concentrated Flow, TRAVEL PATH D TO E
					Paved Kv=20.3 fps
0.4	250	0.0250	10.82	13.28	Pipe Channel, TRAVEL PATH F TO E
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.010
5.9	505	Total			

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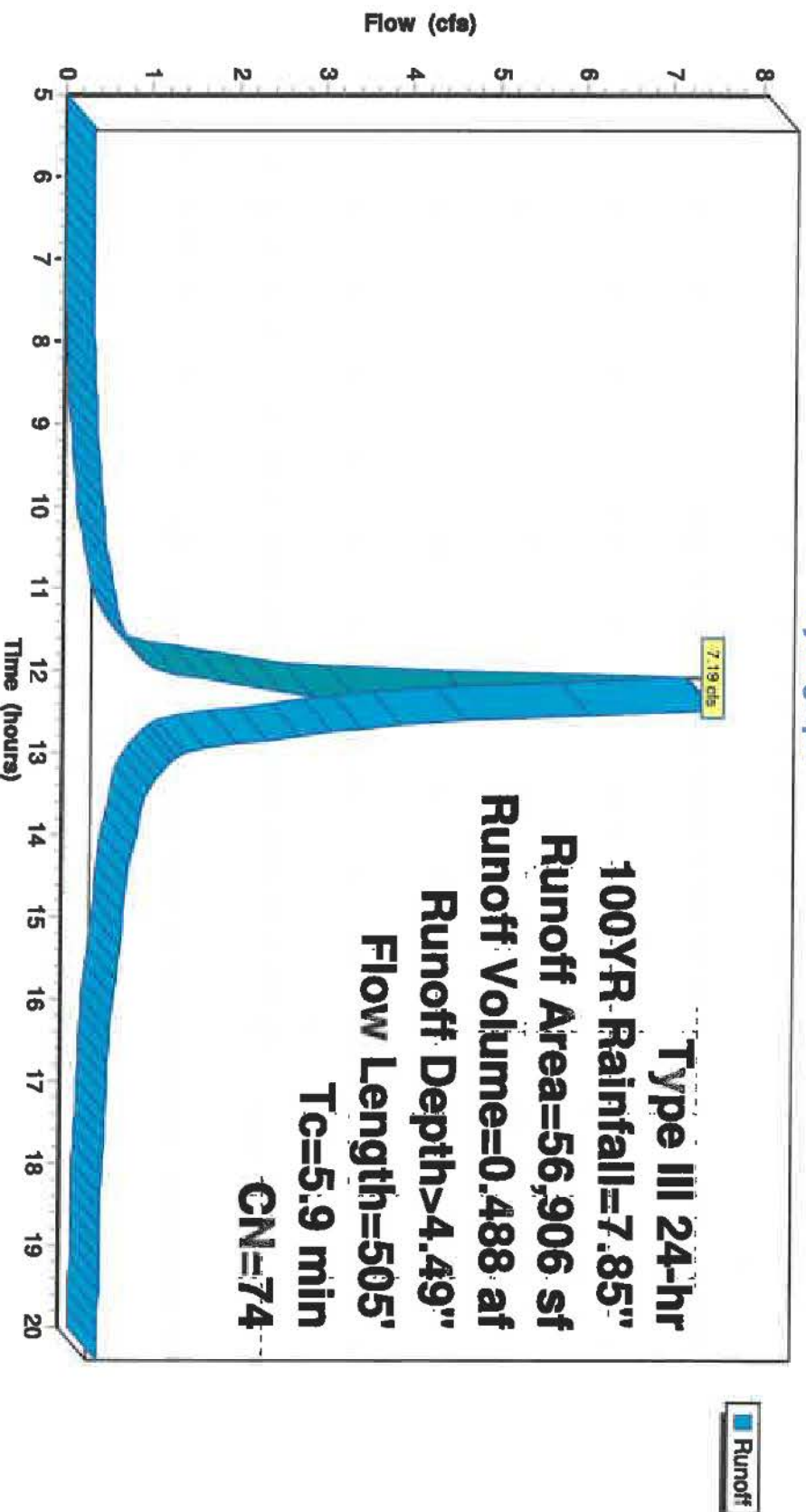
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Subcatchment P-4: P-4

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Summary for Subcatchment P-5: P-5

Runoff = 14.64 cfs @ 12.14 hrs, Volume= 1.122 af, Depth> 4.04"
Routed to Reach IP-P1 : 45 MAIN ST

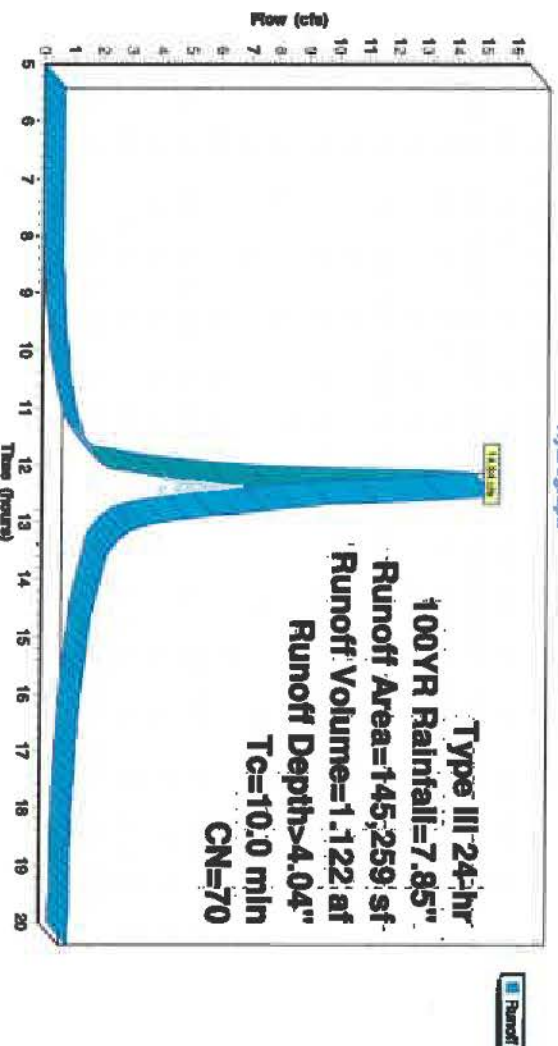
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)	CN	Description
145,259	70	Woods, Good, HSG C
145,259	100.00%	Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.0					Direct Entry, TRAVEL PATH

Subcatchment P-5: P-5

Hydrograph



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Type III 24-hr 100YR Rainfall=7.85"
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Summary for Subcatchment P-6: P-6

Runoff = 8.98 cfs @ 12.12 hrs, Volume= 0.649 af, Depth> 4.15"
Routed to Reach IP-P2 : SCHOOL ST

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)		CN	Description		
13,023	74	>75%	Grass cover, Good, HSG C		
68,668	70	Woods, Good,	HSG C		
81,691	71	Weighted Average			
81,691		100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.1	50	0.0450	0.20		Sheet Flow, TRAVEL PATH A TO B
					Grass: Short n=0.150 P2=3.20"
0.9	184	0.0450	3.42		Shallow Concentrated Flow, TRAVEL PATH B TO C
					Unpaved Kv= 16.1 fps
3.1	195	0.0450	1.06		Shallow Concentrated Flow, TRAVEL PATH C TO D
					Woodland Kv= 5.0 fps
8.1	429	Total			

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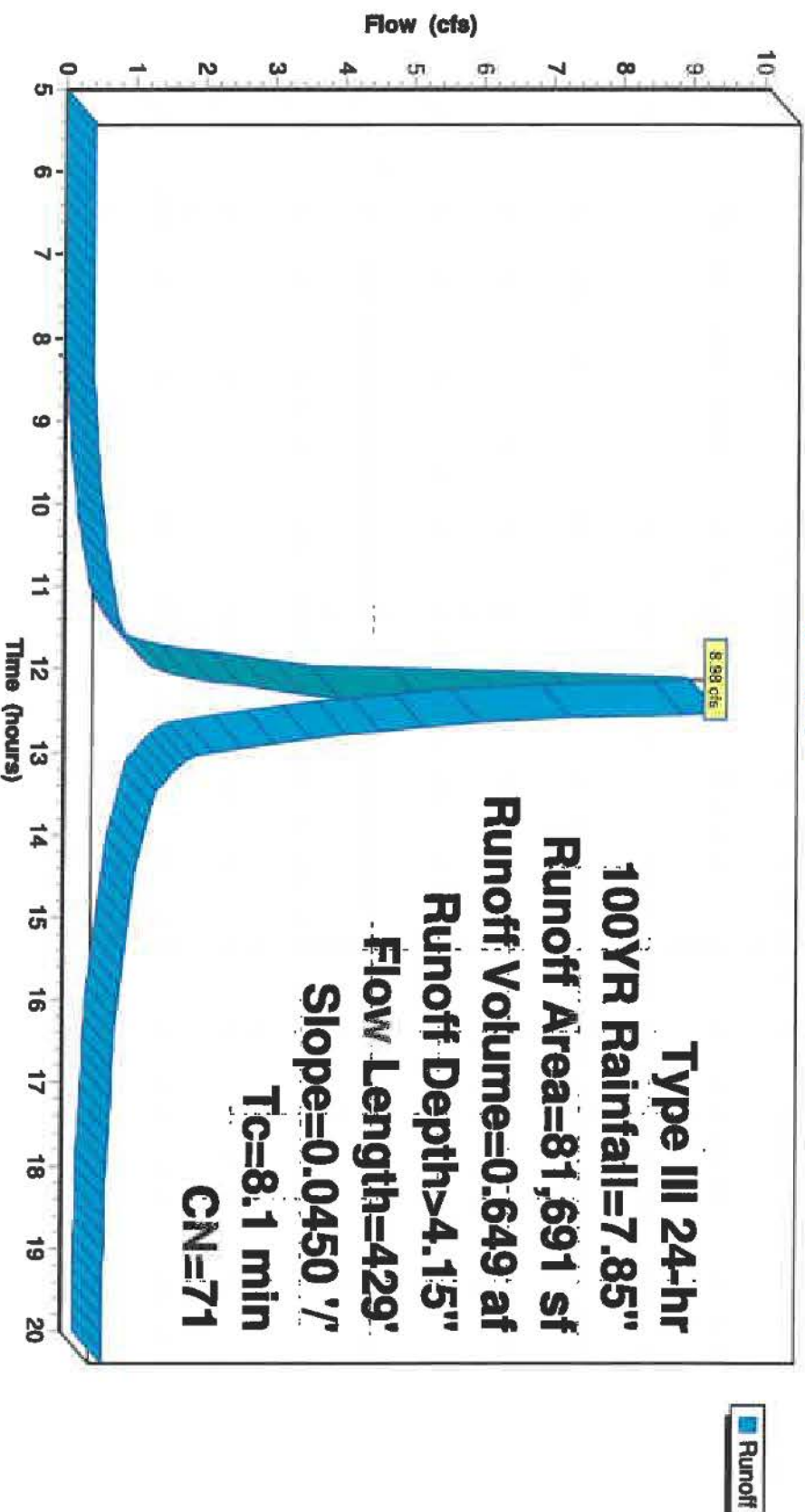
Type III 24-hr 100YR Rainfall=7.85"

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Subcatchment P-6: P-6

Hydrograph



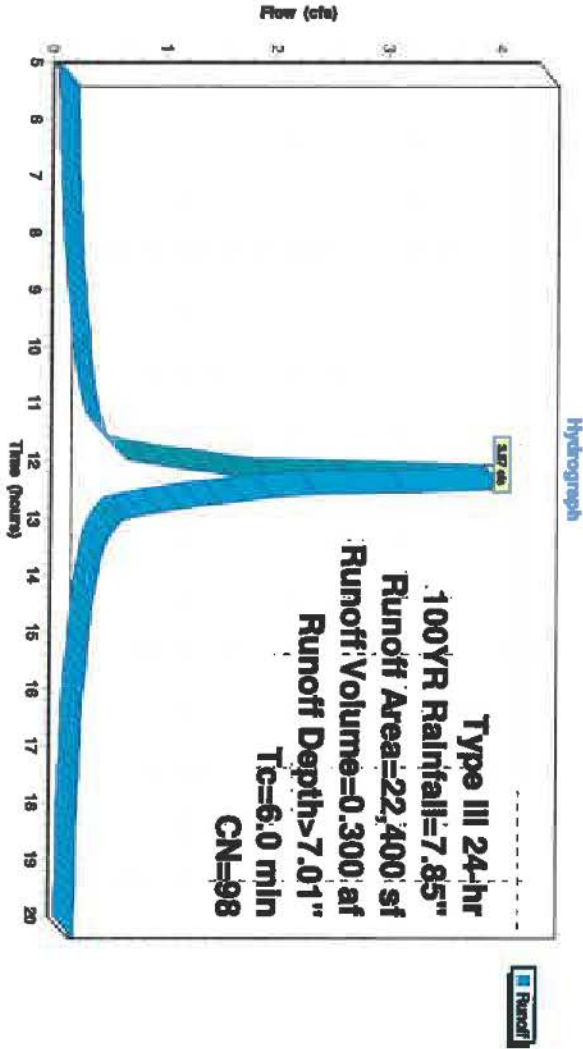
Summary for Subcatchment P-7: BUILDING

Runoff = 3.87 cfs @ 12.09 hrs, Volume= 0.300 af, Depth> 7.01"
Routed to Pond PND4 : U/G BASIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100YR Rainfall=7.85"

Area (sf)		CN	Description		
22,400		98	Roofs, HSG C		
22,400			100.00% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, TRAVEL PATH

Subcatchment P-7: BUILDING



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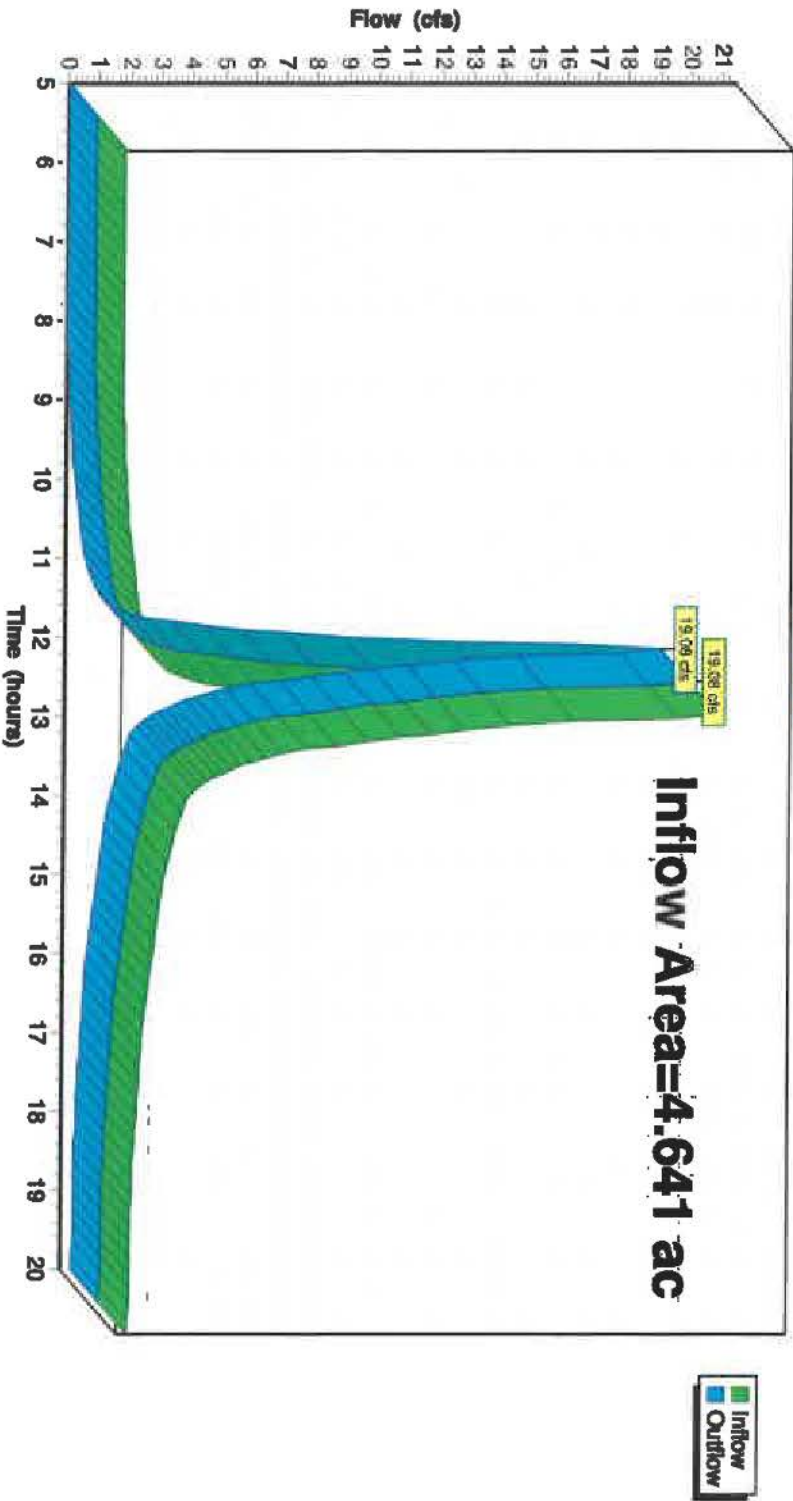
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Summary for Reach IP-P1: 45 MAIN ST

Inflow Area = 4.641 ac, 2.08% Impervious, Inflow Depth > 3.82" for 100YR event
Inflow = 19.08 cfs @ 12.16 hrs, Volume= 1.428 af
Outflow = 19.08 cfs @ 12.16 hrs, Volume= 1.428 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P1: 45 MAIN ST

Hydrograph



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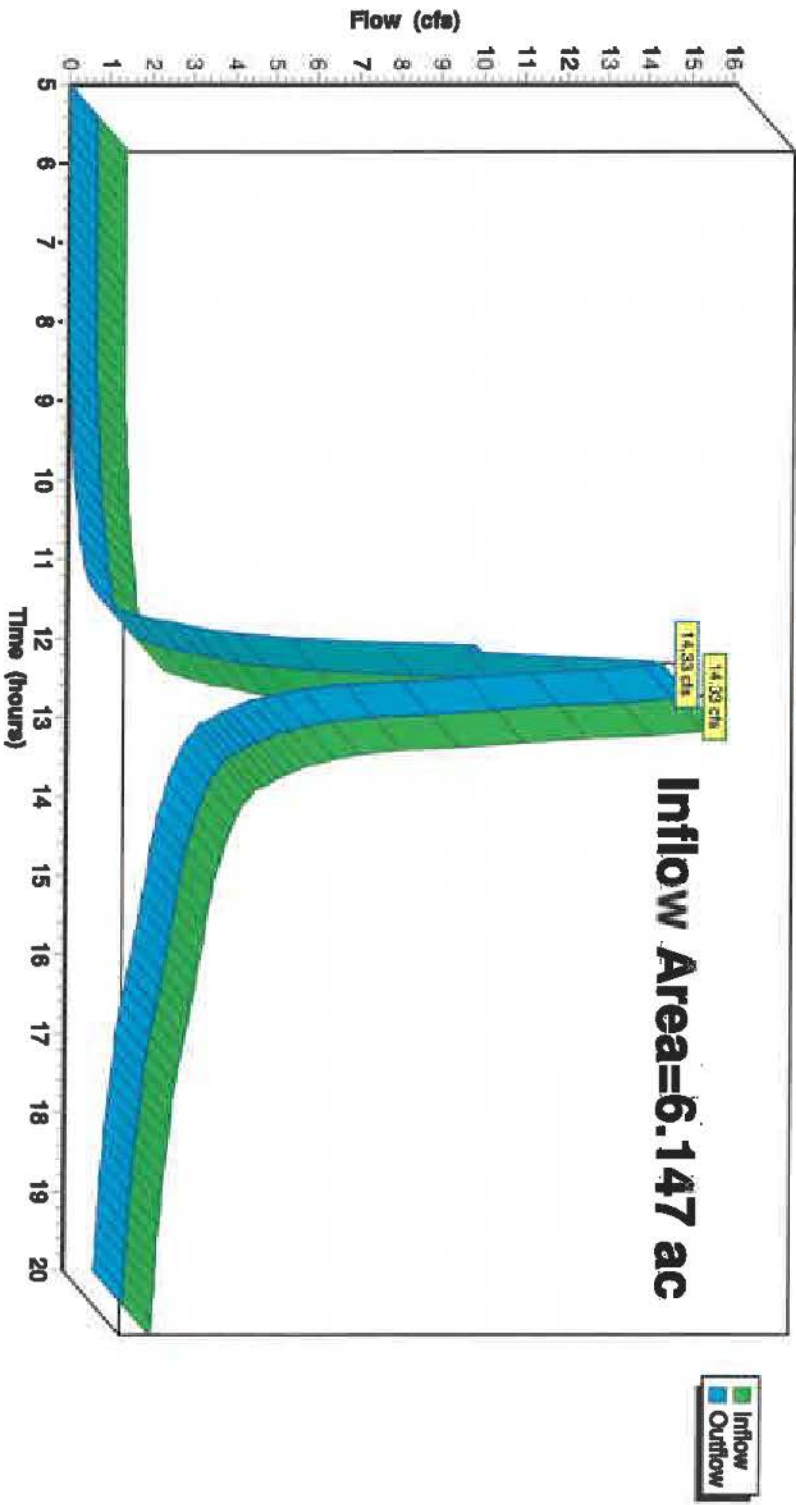
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Summary for Reach IP-P2: SCHOOL ST

Inflow Area = 6.147 ac, 30.84% Impervious, Inflow Depth > 3.38" for 100YR event
Inflow = 14.33 cfs @ 12.32 hrs, Volume= 1.532 af
Outflow = 14.33 cfs @ 12.32 hrs, Volume= 1.532 af, Atten= 0%, Lag= 0.0 min
Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P2: SCHOOL ST

Hydrograph



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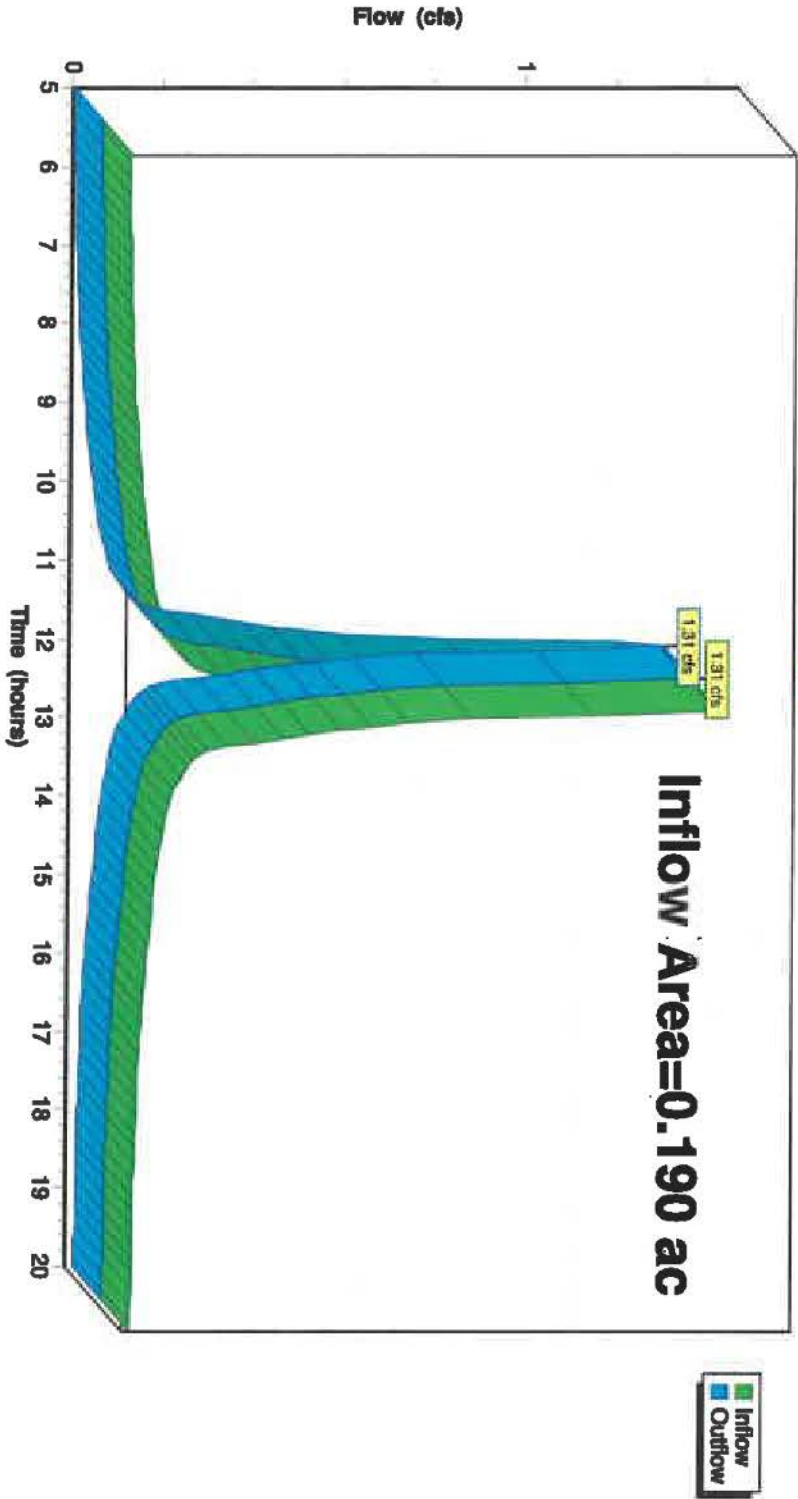
Summary for Reach IP-P3: EX. CULVERT

Inflow Area = 0.190 ac, 54.38% Impervious, Inflow Depth > 5.96" for 100YR event
Inflow = 1.31 cfs @ 12.09 hrs, Volume= 0.094 af
Outflow = 1.31 cfs @ 12.09 hrs, Volume= 0.094 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Reach IP-P3: EX. CULVERT

Hydrograph



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Summary for Pond PND1: PND 1

Inflow Area = 2.380 ac, 52.89% Impervious, Inflow Depth > 4.66" for 100YR event
 Inflow = 11.63 cfs @ 12.10 hrs, Volume= 0.923 af
 Outflow = 5.74 cfs @ 12.31 hrs, Volume= 0.755 af, Atten= 51%, Lag= 12.2 min
 Discarded = 0.12 cfs @ 12.31 hrs, Volume= 0.085 af
 Primary = 5.62 cfs @ 12.31 hrs, Volume= 0.669 af
 Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 327.73' @ 12.31 hrs Surf.Area= 5,070 sf Storage= 16,374 cf

Plug-Flow detention time= 149.1 min calculated for 0.752 af (81% of Inflow)
 Center-of-Mass det. time= 100.6 min (877.0 - 776.4)

Volume Invert Avail.Storage Storage Description

#1 322.00' 23,430 cf Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
322.00	1,030	0	0
324.00	2,125	3,155	3,155
326.00	3,600	5,725	8,880
328.00	5,300	8,900	17,780
329.00	6,000	5,650	23,430

Device Routing Invert Outlet Devices

#1	Discarded	322.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	324.50'	4.0" Vert. OrificeGrate C= 0.600 Limited to weir flow at low heads
#3	Primary	326.50'	4.0" Vert. OrificeGrate C= 0.600 Limited to weir flow at low heads
#4	Primary	327.50'	15.0" long x 1.0" breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00			
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32			

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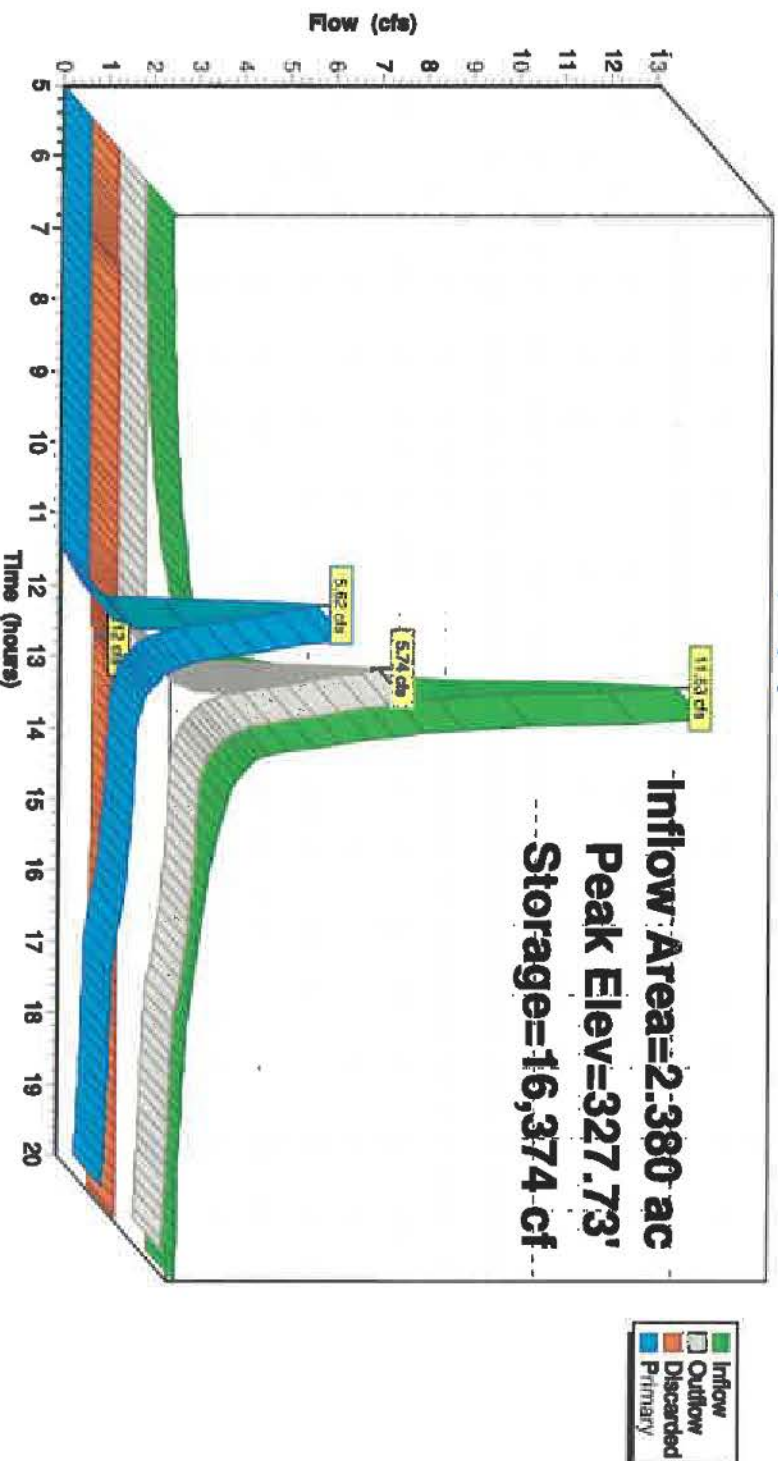
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Discarded OutFlow Max=0.12 cfs @ 12.31 hrs HW=327.73' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=5.55 cfs @ 12.31 hrs HW=327.73' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.74 cfs @ 8.42 fps)
3=Orifice/Grate (Orifice Controls 0.43 cfs @ 4.96 fps)
4=Broad-Crested Rectangular Weir (Weir Controls 4.38 cfs @ 1.28 fps)

Pond PND1: PND 1

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Summary for Pond PND2: BASIN#2

Inflow Area = 1.891 ac, 33.69% Impervious, Inflow Depth > 5.28" for 100YR event
Inflow = 11.59 cfs @ 12.10 hrs, Volume= 0.832 af
Outflow = 4.68 cfs @ 12.37 hrs, Volume= 0.520 af, Atten= 60%, Lag= 15.8 min
Discarded = 0.11 cfs @ 12.37 hrs, Volume= 0.106 af
Primary = 4.56 cfs @ 12.37 hrs, Volume= 0.414 af
Routed to Reach IP-P2 : SCHOOL ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 328.21' @ 12.37 hrs Surf.Area= 4,828 sf Storage= 17,456 cf

Plug-Flow detention time= 162.2 min calculated for 0.518 af (62% of inflow)
Center-of-Mass det. time= 90.2 min (860.8 - 770.6)

Volume	Invert	Avail.Storage	Storage Description
#1	324.00'	20,173 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 21,436 cf Overall - 1,263 cf Embedded = 20,173 cf
#2	324.00'	1,263 cf	Cuttec R-330XLHD x 24 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
21,436 cf Total Available Storage			

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
324.00	3,312	0	0
326.00	4,212	7,524	7,524
328.00	4,700	8,912	16,436
329.00	5,300	5,000	21,436

Device	Routing	Invert	Outlet Devices
#1	Discarded	324.00'	1.020 In/hr Exfiltration over Surface area
#2	Primary	325.00'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	327.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Primary	328.00'	15.0' long x 1.0' breadth Broad-Crested Rectangular Weir

POST DEVELOPMENT1-8-24

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Type III 24-hr 100YR Rainfall=7.85"

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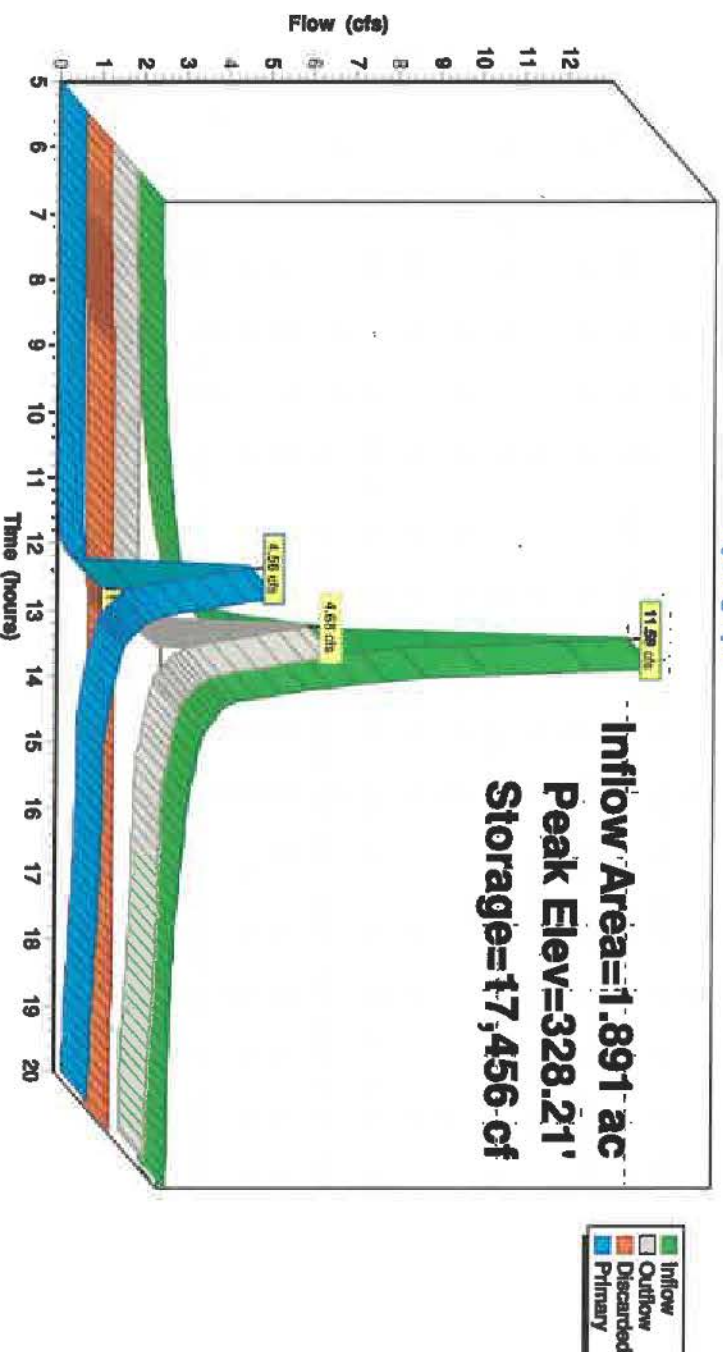
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Discarded OutFlow Max=0.11 cfs @ 12.37 hrs HW=328.21' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=4.45 cfs @ 12.37 hrs HW=328.21' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.51 fps)
3=Orifice/Grate (Orifice Controls 0.36 cfs @ 4.15 fps)
4=Broad-Crested Rectangular Weir (Weir Controls 3.90 cfs @ 1.23 fps)

Pond PND2: BASIN#2

hydrograph



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Summary for Pond PND3: BASIN3

Inflow Area = 1.306 ac, 7.39% Impervious, Inflow Depth > 4.49" for 100YR event
Inflow = 7.19 cfs @ 12.09 hrs, Volume= 0.488 af
Outflow = 5.04 cfs @ 12.19 hrs, Volume= 0.429 af, Atten= 30%, Lag= 6.0 min
Discarded = 0.12 cfs @ 12.19 hrs, Volume= 0.073 af
Primary = 4.92 cfs @ 12.19 hrs, Volume= 0.356 af
Routed to Reach IP-P1 : 45 MAIN ST

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 334.10' @ 12.19 hrs Surf.Area= 2,143 sf Storage= 5,702 cf

Plug-Flow detention time= 64.9 min calculated for 0.429 af (88% of inflow)
Center-of-Mass det. time= 27.2 min (810.2 - 783.1)

Volume Invert Avail.Storage Storage Description

#1 330.00' 7,937 cf Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
330.00	925	0	0
332.00	1,250	2,175	2,175
334.00	2,070	3,320	5,495
335.00	2,814	2,442	7,937

Device Routing Invert Outlet Devices

#1	Discarded	330.00'	2,400 in/hr Exfiltration over Surface area
#2	Primary	332.20'	10.0' Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	334.00'	20.0' long x 0.5' breadth Broad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00
Coef. (English) 2.80 2.92 3.08 3.30 3.32

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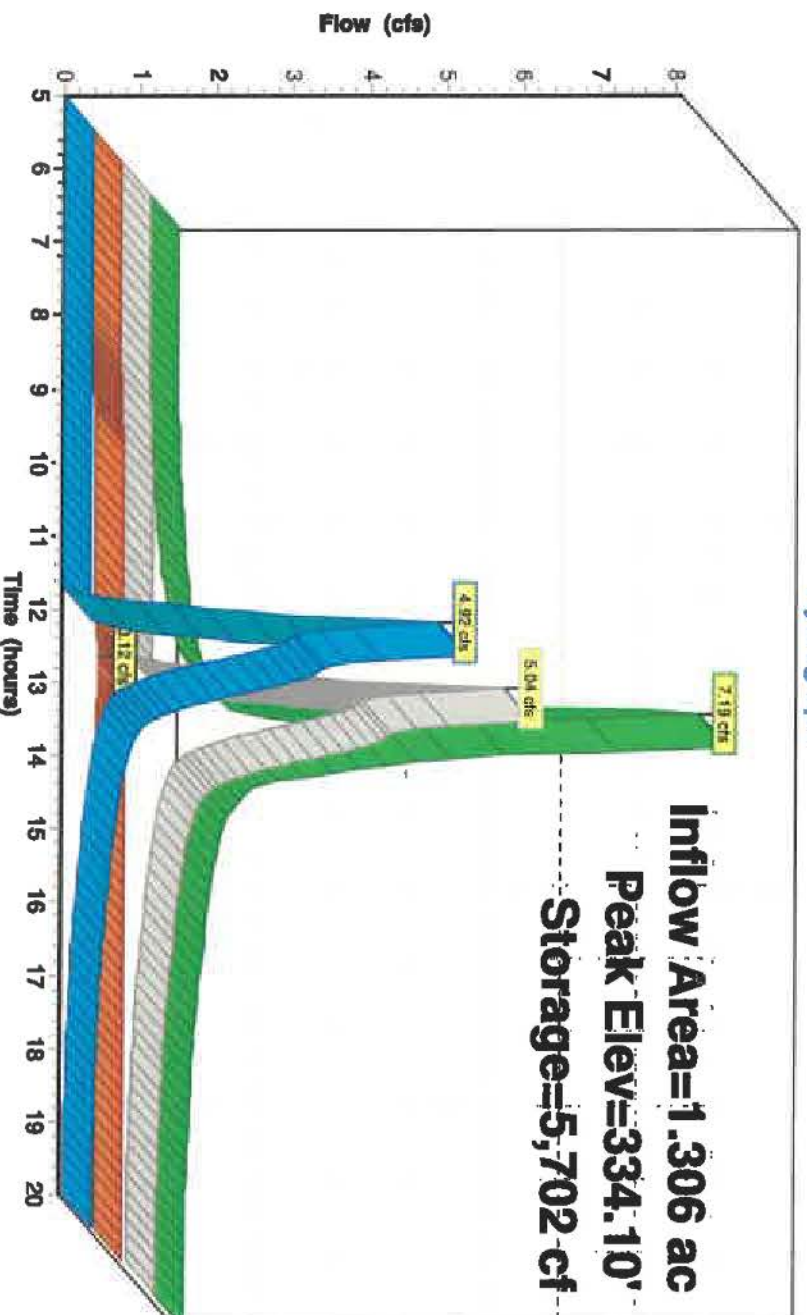
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Discarded OutFlow Max=0.12 cfs @ 12.19 hrs HW=334.09' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=4.78 cfs @ 12.19 hrs HW=334.09' (Free Discharge)
2=Orifice/Grate (Orifice Controls 3.19 cfs @ 5.85 fps)
3=Broad-Created Rectangular Weir (Weir Controls 1.59 cfs @ 0.85 fps)

Pond PND3: BASIN3

Hydrograph



POST DEVELOPMENT1-8-24

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Summary for Pond PND4: U/G BASIN

Inflow Area = 0.514 ac, 100.00% Impervious, Inflow Depth > 7.01" for 100YR event
Inflow = 3.87 cfs @ 12.09 hrs, Volume= 0.300 af
Outflow = 0.41 cfs @ 12.80 hrs, Volume= 0.186 af, Atten= 90%, Lag= 43.0 min
Discarded = 0.09 cfs @ 7.80 hrs, Volume= 0.100 af
Primary = 0.32 cfs @ 12.80 hrs, Volume= 0.085 af
Routed to Pond PND1 : PND 1

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Peak Elev= 327.25' @ 12.80 hrs Surf.Area= 3,637 sf Storage= 6,951 cf

Plug-Flow detention time= 157.8 min calculated for 0.185 af (62% of inflow)
Center-of-Mass det. time= 79.4 min (812.5 - 733.1)

Volume	Invert	Avail.Storage	Storage Description
#1	325.00'	2,587 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 12,739 cf Overall - 6,270 cf Embedded = 6,468 cf x 40.0% Voids #2 325.00' 6,270 cf Cultec R-330XLHD x 120 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 1 rows
		8,857 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
325.00	3,673	0	0
325.50	3,637	1,828	1,828
328.00	3,637	9,093	10,920
328.50	3,637	1,819	12,739

Device	Routing	Invert	Outlet Devices
#1	Discarded	325.00'	1.020 in/hr Exfiltration over Surface area
#2	Primary	326.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

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Discarded OutFlow Max=0.09 cfs @ 7.80 hrs HW=325.04' (Free Discharge)
1=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.32 cfs @ 12.80 hrs HW=327.25' (Free Discharge)
2=Orifice/Grate (Orifice Controls 0.32 cfs @ 3.66 fps)

Pond PND4: U/G BASIN

Hydrograph

